Interoffice Memorandum
October 29, 2018

| TO: | Mayor Teresa Jacobs <br> and Board of County Commissioners <br> FROM: <br> SUBJECT: <br>  <br> Raymond E. Hanson, P. E., Director <br> Utilities Department |
| :--- | :--- |
|  | BCC AGENDA ITEM - Consent Agenda <br> November 13, 2018 BCC Meeting <br> First Amendment to Reedy Creek Improvement District/Orange County <br> Amended and Restated Water, Wastewater, and Reclaimed Water Service |
| Territorial Agreement; Interlocal Agreement between Reedy Creek <br> Improvement District and Orange County for Delivery of Wholesale Water |  |
| Services to the Flamingo Crossings Development <br> Contact Person: Andres Salcedo, P.E., Assistant Director <br> Atilities Department |  |
|  | 407-254-9719 |

Reedy Creek Improvement District (RCID) has requested Orange County (the "County") to amend the Reedy Creek Improvement District/Orange County Amended and Restated Water, Wastewater, and Reclaimed Water Service Territorial Agreement (the "Territorial Agreement"), dated September 30, 2008. This first amendment would revise the potable water, wastewater, and reclaimed water service territorial boundary between RCID and the County to remove parcels FC-1 and FC-2, which are areas inside of FC Ultimate, from the water, wastewater, and reclaimed water territory of RCID and to include those parcels within the territorial jurisdiction of the County.

Contemporaneously with the first amendment to the Territorial Agreement, RCID has requested an interlocal agreement for RCID to provide wholesale water, wastewater, and reclaimed water services ("Water Services") to the County to serve FC Ulitimate until the County initiates Water Services from its utility systems. The interlocal agreement also provides that if, after the County initiates water service to FC Ulimate and the County determines that it needs additional water to address a hydraulic constraint, RCID will provide wholesale water to the County for use by FC Ultimate until such time as the County eliminates the hydraulic constraint, which shall be on or before the $10^{\text {th }}$ anniversary of the effective date of the agreement.

The backup documentation for this item has been delivered under separate cover. It may also be accessed online as part of the eAgenda by clicking here.

The County Attorney's Office staff reviewed this agreement and finds it acceptable. Utilities Department staff recommends approval.

Action Requested: Approval and execution of (1) First Amendment to Reedy Creek Improvement District/Orange County Amended and Restated Water, Wastewater, and Reclaimed Water Service Territorial Agreement and (2) Interlocal Agreement between Reedy Creek Improvement District and Orange County for delivery of wholesale water services to the Flamingo Crossings Development.

District 1.

# FIRST AMENDMENT 

TO

# REEDY CREEK IMPROVEMENT DISTRICT/ORANGE COUNTY AMENDED AND RESTATED WATER, WASTEWATER, AND RECLAIMED WATER SERVICE TERRITORIAL AGREEMENT 


#### Abstract

THIS FIRST AMENDMENT TO THE REEDY CREEK IMPROVEMENT DISTRICT/ORANGE COUNTY AMENDED AND RESTATED WATER, WASTEWATER, AND RECLAIMED WATER SERVICE TERRITORIAL AGREEMENT (this "First Amendment"), is made and entered into on the date of later execution below, by ańd between REEDY CREEK IMPROVEMENT DISTRICT, a public corporation and public body corporate and politic of the State of Florida, whose address is P.O. Box 10170, Lake Buena Vista, Florida 32830 (hereinafter called "RCID"), and ORANGE COUNTY, a charter county and political subdivision of the State of Florida (hereinafter called the "County"), whose address is 201 South Rosalind Avenue, Orlando, Florida 32801.


## RECITALS

WHEREAS, RCID and the County entered into that agreement entitled "Reedy Creek Improvement District/Orange County Amended and Restated Water, Wastewater, and Reclaimed Water Service Territorial Agreement" (the "Agreement"), dated September 30, 2008; and

WHEREAS, the Agreement defines the potable water, wastewater and reclaimed water service territorial boundary between RCID and the County and describes the areas referred to therein as "RCID's Territorial Area" and the "Adjacent Territorial Area;" and

WHEREAS, RCID and the County desire to modify and alter RCID's Territorial Area, as that term is defined in the Agreement, in accordance with the provisions set forth herein; and

WHEREAS, Section 5 of the Agreement provides that RCID and the County may alter the RCID Territorial Area by mutual consent by the preparation of a document fully describing such alteration, which document is approved by the governing boards of each party and provides the legal description and map of the proposed new RCID Territorial Area; and

WHEREAS, this First Amendment fulfills the requirements set forth in Section

5 of the Agreement.
NOW, THEREFORE, in consideration of the foregoing premises, and for other good and valuable consideration, the parties agree as follows:

1. The recitals set forth above are true and correct and by this reference are incorporated into this First Amendment.
2. Exhibit "A" to the Agreement is hereby replaced with Exhibit "A1," which is attached hereto and incorporated by this reference into the Agreement.
3. Except as modified by this First Amendment, the terms and provisions of the Agreement shall remain unchanged and in full force and effect.
[SIGNATURES APPEAR ON THE FOLLOWING PAGES]

IN WITNESS WHEREOF, RCID and the County have caused this First Amendment to be executed by their duly designated representatives as of the date and year indicated below.
"RCID"
REEDY CREEK IMPROVEMENT DISTRICT


John H. Classe, Jr.
District Administrator
Date: $\qquad$


## "COUNTY"

ORANGE COUNTY
By: Board of County Commissioners
By: Hin dalcaan fa


Date: $\qquad$

ATTEST: Phil Diamond, CPA, Orange County Comptroller as Clerk to the Board of County Commissioners


# DESCRIPTION OF <br> REEDY CREEK IMPROVEMENT DISTRICT WATER AND WASTE WATER TERRITORIAL AREA IN ORANGE COUNTY 

Begin at the Southwest corner of the Northwest $1 / 4$ of the Southwest $1 / 4$ of Section 6, Township 24 South, Range 28 Eost run $N 00^{\circ} 00^{\prime} 22^{\prime \prime} E, 1327.43$ feet along the West line of Section 5 to the West $1 / 4$ corner thereaf; thence $N$ $89^{\circ} 27^{\prime} 45^{\prime \prime} \mathrm{E}, 1997.50$ feet along the North line of the South half of Section 6 , to the Southwest corner of the East $1 / 2$ of the Southeast $1 / 4$ of the Northwest $1 / 4$ of Section 6, thence $N 00^{\circ} 20^{\prime} 35^{\prime \prime} \mathrm{W}, 1154.75$ feet along the West line of the East $1 / 2$ of the Southeast $1 / 4$ of the Northwest $1 / 4$ of Section 6; thence $N 89.38^{\prime} 50^{\prime \prime}$ E, 663.64 feet along a line that is 165.00 feet South of and parallel to the North line of the Southeast $1 / 4$ of the Northwest $1 / 4$ of Section 6 ; thence $N 8911^{\prime} 34^{\prime \prime}$ E, 148.62 feet $+/-$ along a line parallel to and 165.00 feet South of the North line of the Southwest $1 / 4$ of the Northeast $1 / 4$ of Section 6 to a point on the Westerly shore line of Lake Mable; thence meander the shore line of Lake Mable in a Southerly direction, to a point on the South line of Section 6 and the North line of Section 7, Township 24 South, Range 28 East, said point being $S 16^{\circ} 20^{\prime \prime} 10^{\prime \prime} W^{\prime} 3981.97$ feet more or less from the previously described point, and also lying $N 89^{\circ} 31^{\prime} 17^{\prime \prime} \mathrm{E}, 1683.05$ feet from the Southwest corner of Section 6 ; thence continue along the share line of Lake Mable in a Southeasterly and Northeosterly direction across the North $1 / 4$ of Section 7, to the North line of Section 7 and the South line of Section 6, Township 24 South, Range 28 East, said point being $N 89^{\circ} 31^{\prime} 17^{\prime \prime} \mathrm{E}$, along the North section line of Section $7,1381.64$ feet from the previously described point and lying $S 89^{\circ} 31^{\prime \prime} 17^{\prime \prime} \mathrm{W}, 2304.35$ feet from the Northeast corner of Section 7; thence continue to meander the shore line of Lake Mable in a Northeasterly direction across the Southeast $1 / 4$ of Section 6, Township 24 South, Range 28 East to a point on soid shoreline which is intersected by the North line of the South half of the Southeast $1 / 4$ of Section 6, said point being $N 25^{\circ} 14^{\prime} 10^{\prime \prime} \mathrm{E}, 1475.82$ feet from the previously described point; thence $N 89^{\circ} 29^{\prime} 30^{\prime \prime} \mathrm{E}$, along said North line of the South half of the Southeast $1 / 4$ of Section 6, 1679.89 feet to the East section line thereof; thence $S 00^{\circ} 12^{\prime} 20^{\prime \prime}$ W. 1330.62 feet along the East line of Section 6 to the Southeast comer of Section 6 and the Northwest corner of Section 8, Township 24 South, Range 28 East; thence $N 89^{\circ} 21^{\prime} 03^{\prime \prime}$ E along the Narth line of Section 8, 191.58 feet more or less to a point on the West shore line of South Lake; thence meander the shore line of South Lake in a Southwesterly, Southeasterly and Northeasterly direction to a point where the shore line of South Lake intersects the East line of the West half of the West half of Section B; said point being $525^{\circ} 17^{\prime} 13^{\prime \prime}$ E, 2679.01 feet more or less from the previously described point; thence $500^{\circ} 13^{\prime} 59^{\prime \prime} \mathrm{W}, 221.07$ feet to the Northeast corner of the Northwest $1 / 4$ of the Southwest $1 / 4$ of Section 8 ; thence $S 0006^{\prime} 21^{\prime \prime}$ E along the East line of the West half of the Southwest $1 / 4$ of Section $8,1334.85$ feet to the Southeast corner of the Northwest $1 / 4$ of the Southwest $1 / 4$ of Section 8; thence S $88^{\circ} 48^{\prime} 04^{\prime \prime}$ W, 1111.09 feet to a point of curvature of a curve concave Southeasterly having a radius of 545.08 feet, and a central angle of $81{ }^{\prime} 15^{\prime} 08^{\prime \prime}$; thence run Southwesterly along the arc of said curve. 772.99 feet; to a point of reverse curvature of a curve concave Northerly having a radius of 80.00 feet, and a central angle of $128^{\circ} 43^{\prime} 50^{\prime \prime}$; thence run Westerly along the arc of said curve, 179.74 feet; thence $S 43^{\circ} 40^{\prime} 59^{\prime \prime} E, 16.92$ feet; thence $S 34^{\circ} 38^{\prime} 41^{\prime \prime} \mathrm{E}, 8.13$ feet; thence $S 25^{\circ} 16^{\prime} 40^{\prime \prime} \mathrm{E}, 86.79$ feet; thence $S 28^{\circ} 57^{\prime} 56^{\prime \prime} \mathrm{E}, 106.03$ feet; thence $S 58^{\circ} 01^{\circ} 53^{\prime \prime} \mathrm{E}$, 87.73 feet; thence $N 85^{\circ} 59^{\prime} 29^{\prime \prime} \mathrm{E}, 134.58$ feet to a point of curvature of a curve concove Southerly having a radius of 425.00 feet, and a central angle of $23^{\circ} 29^{\prime} 59^{\prime \prime}$; thence run Easterly along the arc of said curve, 174.31 feet; to a point of compound curvature of a curve concave Southwesterly having a radius of 15.00 feet, and a central angle of $46^{\circ} 20^{\prime} 48^{\prime \prime}$; thence run Southeasterly along the arc of said curve, 12.13 feet; to a point of compound curvature of a curve concave Westerly having a radius of 425.00 feet, and a central angle of $16^{\circ} 33^{\prime} 54^{\prime \prime}$; thence run Southerly along the arc of said curve, 122.87 feet; to a point of compound curvature of a curve concave Westerly having a radius of 25.00 feet, and a central angle of $51^{\prime} 32^{\prime} 25^{\prime \prime}$; thence run Southerly along the arc of said curve, 22.49 feet; thence $S$ $43^{\circ} 56^{\prime} 36^{\prime \prime} \mathrm{W}, 91.06$ feet; thence $S 64^{\circ} 40^{\prime} 37^{\prime \prime} \mathrm{W}, 105.25$ feet; thence $S 40^{\circ} 45^{\prime} 32^{\prime \prime} \mathrm{W}, 117.42$ feet; thence $S 13.26^{\prime} 04^{\prime \prime} \mathrm{W}$, 97.39 feet; thence $S^{\prime} 42^{\circ} 14^{\prime \prime} 20^{\prime \prime} \mathrm{W}$, 133.97 feet; thence $S 68^{\circ} 59^{\prime} 11^{\prime \prime} \mathrm{W}, 89.71$ feet; thence $S 28^{\circ} 50^{\prime \prime} 44^{\prime \prime} \mathrm{W}$, 77.77 feet; thence $S 14^{\circ} 52^{\prime} 47^{\prime \prime} \mathrm{W}, 88.32$ feet; thence $S ~ 01^{\circ} 59^{\prime} 29^{\prime \prime} \mathrm{E}, 106.28$ feet; thence $S 24^{\circ} 42^{\prime} 46^{\prime \prime} \mathrm{W}, 241.59$ feet; thence $S$ $36^{\circ} 55^{\circ} 50^{\prime \prime} \mathrm{W}, 126.64$ feet; thence $524^{\circ} 03^{\prime} 44^{\prime \prime} \mathrm{W}, 71.01$ feet to a point of curvature of a curve concove Northwesterly having a radius of 25.00 feet, and a central angle of $40^{\circ} 55^{\prime} 45^{\prime \prime}$; thence run Southwesterly along the arc of said curve, 17.86 feet; thence $S 64^{\circ} 59^{\prime} 30^{\prime \prime \prime} \mathrm{W}, 91.68$ feet to a point of curvature of a curve concave Northerly having a radius of 25.00 feet, and a central angle of $46^{\prime} 29^{\prime} 32^{\prime \prime}$; thence run Westerly along the arc of said curve, 20.29 feet; thence $N$ $68^{\circ} 30^{\prime} 58^{\prime \prime} \mathrm{W}, 131.37$ feet; thence $N 34^{\circ} 57^{\prime} 28^{n} \mathrm{~W}, 145.43$ feet; thence $N 10^{\circ} 44^{\circ} 04^{\prime \prime} \mathrm{W}, 144.09$ feet; thence $N 10.34^{\prime} 18^{\prime \prime} \mathrm{E}$, 129.55 feet; thence $N .44^{\circ} 03^{\prime} 35^{\prime \prime} \mathrm{E}, 129.67$ feet; thence $N 86^{\circ} 35^{\prime} 32^{\prime \prime}$ E, 100.03 feet; thence $N .62^{\circ} 48^{\prime} 18^{\circ}$ E, 100.08 feet; CONTINUED ON SHEET 2

SURVEYNE AND MAPPING DEPARTMENT P.O.B. 10000 LAKE BUENA VSTA F. 32830-1000 PHONE (407)560-7118 FAX (407)560-7869

| WLING AREA $D I S N E Y$ OVERALL | $\begin{aligned} & \text { DATE } \\ & 12 / 07 / 17 \\ & \hline \end{aligned}$ |
| :---: | :---: |
| $\stackrel{\text { Proget NaME }}{\text { RCID }}$ WATER/WASTE WATER TERRITORY | SCAIE |
| SURVEY TTPE <br> SKETCH OF DESCRIPTION | $\begin{aligned} & \text { DRAMN BY: } \\ & \text { JLG } \end{aligned}$ |
| ${ }_{\text {COMMENTS }}^{\text {EXHIBIT }}$ A1, SHEET 1 OF 31 SHEETS | $\begin{aligned} & \text { FILDAME: } \\ & 10 J G 096 R 2 \end{aligned}$ |

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thence $N 58^{\prime} 16^{\prime} 14^{\prime \prime} \mathrm{E}, 95.99$ feet; thence $N 15^{\circ} 01^{\prime} 47^{\prime \prime} \mathrm{E}, 86.03$ feet; thence $N .14^{\circ} 30^{\prime} 32^{\prime \prime} \mathrm{W}, 104.94$ feet; thence $N$ $03^{\circ} 06^{\prime} 23^{\prime \prime} \mathrm{W}, 111.09$ feet; thence $N 07.32^{\prime} 42^{\prime \prime} \mathrm{E}, 68.01$ feet; thence $N 15^{\prime} 14^{\prime} 13^{\prime \prime} \mathrm{W}, 80.67$ feet; thence $N$ 87.12'48" W , 40.11 feet; thence $S 77^{\circ} 42^{\prime} 57^{\prime \prime} \mathrm{W}$, 84.88 feet; thence $574^{\circ} 44^{\prime} 47^{\prime \prime} \mathrm{W}, 66.79$ feet; thence $S 35^{\circ} 20^{\prime} 27^{\prime \prime} \mathrm{W}, 90.33$ feet; thence $S 22^{\circ} 58^{\prime} 13^{\prime \prime} \mathrm{W}, 87.94$ feet; thence $S 20^{\circ} 05^{\prime} 22^{\prime \prime} \mathrm{W}, 168.18$ feet; thence $S 65^{\circ} 39^{\prime} 23^{\prime \prime} \mathrm{W}$, 108.46 feet; thence $N$ $79^{\circ} 02^{\prime} 16^{\prime \prime} \mathrm{W}, 146.86$ feet; thence $S^{\prime} 44^{\circ} 41^{\prime} 24^{\prime \prime} \mathrm{W}, 85.24$ feet; thence $S 66^{\circ} 58^{\prime} 59^{\prime \prime} \mathrm{W}, 80.82$ feet; thence $\mathrm{N} 89^{\circ} 03^{\prime} 00^{\prime \prime} \mathrm{W}$, 96.88 feet; thence $S 8478^{\prime} 13^{\prime \prime} W, 51.79$ feet; thence $S 77.56^{\circ} 53^{\prime \prime} \mathrm{W}, 116.91$ feet; thence $S 70^{\circ} 14^{\prime} 00^{\prime \prime} \mathrm{W}$, 84.26 feet; thence $N 63^{\circ} 52^{\prime} 48^{\prime \prime} \mathrm{W}, 163.26$ feet; thence $N 71^{\circ} 49^{\prime} 57^{\prime \prime} \mathrm{W}, 91.32$ feet; thence $N 560^{\circ} 38^{\prime} 48^{\prime \prime} \mathrm{W}, 106.72$ feet; thence. $N$ $37.38^{\prime} 37^{\prime \prime} \mathrm{W}, 96.72$ feet; thence $N 69^{\circ} 48^{\prime} 38^{\prime \prime} \mathrm{W}, 85.22$ feet; thence $N 8515^{\prime} 14^{\prime \prime} \mathrm{W}, 95.72$ feet; thence $N 76.56^{\prime} 11^{\prime \prime} \mathrm{W}$, 104.56 feet; thence $S 28^{\circ} 55^{\prime} 14^{\prime \prime} \mathrm{W}, 152.43$ feet; thence $S 13^{\circ} 45^{\prime} 44^{\prime \prime} \mathrm{E}, 47.73$ feet to a point of curvature of a curve concave Westerly having a radius of 75.00 feet, and a central angle of $30.06^{\prime} 13^{\prime \prime}$; thence run Southerly along the arc of said curve, 39.41 feet; to a point of reverse curvature of a curve concave Northeasterly having a radius of 45.00 feet, and a central angle of $99^{\circ} 54^{\prime \prime} 55^{\prime \prime}$; thence run Southeasterly along the are of said curve, 78.47 feet; to a point of reverse curvature of a curve concave Southwesterly having a radius of 250.00 feet, and a central angle of $55^{\circ} 31^{\prime \prime} 16^{\prime \prime}$; thence run Southeasterly along the arc of said curve, 242.26 feet; thence $S 28^{\circ} 03^{1} 11^{\prime \prime} \mathrm{E}, 95.35$ feet to a point of curvature of a curve concave Westerly having a radius of 125.00 feet, and a central angle of $59^{\circ} 41^{\circ} 01^{\circ}$; thence run Southerly alang the arc of said curve, 130.21 feet; thence $S 31^{\circ} 37^{\prime} 50^{\prime \prime} \mathrm{W}, 165.37$ feet; thence $S 51^{\circ} 01^{\prime \prime} 41^{\prime \prime} \mathrm{E}$, 83.54 feet to a point on a non-tangent curve concave Southeasterly having a radius of 675.49 feet, and a central angle of $29^{\circ} 43^{\prime} 07^{\prime \prime}$; thence from a tangent bearing of $N 50^{\circ} 17^{\prime} 44^{\prime \prime} \mathrm{E}$ run Northeasterly along the arc of said curve, 350.89 feet; thence $S 35^{\circ} 59^{\prime} 30^{\prime \prime} \mathrm{E}, 246.14$ feet; thence $S 55^{\circ} 37^{\prime} 13^{\prime \prime} \mathrm{E}, 316.45$ feet; thence $S 68^{\circ} 44^{\circ} 46^{\prime \prime} \mathrm{E}, 336.44$ feet to a point on a non-tangent curve concave Southerly having a radius of 399.38 feet, and a central angle of $090^{\circ} 53^{\prime} 41^{\prime \prime}$; thence from a tangent bearing of $N 79^{\prime} 13^{\prime} 56^{\prime \prime} \mathrm{E}$ run Easterly along the arc of said curve, 68.97 feet; to a paint of reverse curvature of a curve concave Northerly having a radius of 137.63 feet, and a central angle of $14^{\circ} 21^{\prime \prime} 49^{\prime \prime}$; thence run Easterly along the arc of said curve, 34.50 feet; thence $S 0357^{\prime} 40^{\prime \prime} \mathrm{W}, 60.74$ feet to a point on a non-tangent curve concave Southerly having a radius of 344.38 feet, and a central angle of $04{ }^{\prime} 15^{\prime \prime} 11^{\prime \prime}$; thence from a tangent bearing of $S 86^{\circ} 02^{\prime} 20^{\prime \prime}$ E run Easterly along the arc of said curve, 25.56 feet; to a point of compound curvature of a curve concove Southerly hoving a radius of 132.00 feet, and a central angle of $26.04^{\prime \prime} 01^{\prime \prime}$; thence run Easterly along the arc of said curve, 60.05 feet; to a point on a non-tangent curve concave Southwesterly having a radius of 184.37 feet, and a central angle of $31^{\circ} 44^{\prime} 00^{\prime \prime}$; thence from a tangent bearing of $549^{\circ} 44^{\prime} 21^{\prime \prime} \mathrm{E}$ run Southeasterly along the are of said curve, 102.11 feet; to a point of compound curvature of a curve concave Westerly having a radius of 679.36 feet, and a central angle of $08^{\circ} 51^{\prime \prime} 48^{\prime \prime}$; thence run Southerly alang the arc of said curve, 105.09 feet; to a point of reverse curvature of a curve concave Easterly having a radius of 437.18 feet, and a central angle of $18^{\circ} 37^{\circ} 07^{\prime \prime}$; thence run Southerly along the arc of said curve, 142.06 feet; to a point of compound curvature of a curve concave Northeasterly having a radius of 395.25 feet, and a central angle of $1873^{\prime} 39^{\prime \prime}$; thence run Southeasterly along the are of said curve, 125.74 feet; to a point of reverse curvature of a curve concave Southwesterly having a radius of 645.09 feet, and a central angle of $03^{\prime} 21^{\prime} 33^{\prime \prime}$; thence run Southeasterly along the arc of said curve, 37.82 feet; thence $\mathrm{N} 82^{\prime} 18^{\prime} 14^{\prime \prime} \mathrm{W}$, 71.09 feet; thence $N 51^{\prime} 44^{\prime} 44^{\prime \prime} \mathrm{W}, 65.78$ feet; thence $N 80^{\circ} 24^{\prime} 25^{\prime \prime} \mathrm{W}, 90.39$ feet; thence $548^{\prime} 32^{\prime} 46^{\prime \prime} \mathrm{W}$. 80.93 feet: thence $S 22.55^{\prime} 38^{\prime \prime} \mathrm{W}, 113.12$ feet; thence $S 27{ }^{\prime} 19^{\prime} 16^{\prime \prime} \mathrm{E}, 55.45$ feet; thence $S 18.40^{\prime} 56^{\prime \prime} W_{3}{ }^{\prime} 159.75$ feet; thence $S$ $10.48^{\prime} 30^{\prime \prime} \mathrm{W}, 160.42$ feet to a point of curvature of a curve concave Easterly having a radius of 223.65 feet, and a central angle of $59^{\circ} 02^{\prime} 33^{\prime \prime}$; thence run Southerly along the orc of said curve, 230.47 feet; to a point on the Northerly and Easterly boundary of Tract R, Golden Oak Phase $1 B$ according to the Plat thereof recorded in Plat Book 75, Pages 3 through 15 of the Public Records of Orange County, a non-tangent curve concave Northerly having a radius of 25.00 feet, and a central angle of $64.33^{\circ} 48^{\prime \prime}$; thence from a tangent bearing of $S 49^{\circ} 58^{\prime} 05^{\circ}$ E run Easterly along the are of said curve, 28.17 feet; thence $N 65^{\circ} 28^{\prime} 07^{\prime \prime} \mathrm{E}, 122.36$ feet; thence $N 76.27^{\prime} 23^{\prime \prime} \mathrm{E}, 76.59$ feet to a point of curvature of a curve concave Northwesterly having a radius of 25.00 feet, and a central angle of $25^{\circ} 14^{\prime} 16^{\prime \prime}$; thence run Northeasterly along the arc of said curve, 11.01 feet; thence $S 78.11^{\prime} 38^{\prime \prime} \mathrm{E}, 85.68$ feet to a point on a non-tangent curve concave Easterly having a radius of 1010.00 feet, and a central angle of $07.58^{\prime} 42^{\prime \prime}$; thence from a 'tangent bearing of $S 11^{\circ} 48^{\prime} 22^{\prime \prime} \mathrm{W}$ run Southerly along the are of said curve, 140.64 feet; to a point on a non-tangent curve

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concave Southwesterly having a radius of 25.00 feet, and a central angle of $87 \cdot 13^{\prime} 52^{\prime \prime}$; thence from a tangent bearing of $N 0.49^{\prime} 41^{\prime \prime} \mathrm{E}$ run Northwesterly alang the arc of said curve, 3 B .06 feet; thence $\mathrm{N} 83{ }^{\prime} 24^{\prime} 11^{\prime \prime} \mathrm{W}$, 42.54 feet to a point of curvature of a curve concave Southerly having a radius of 221.37 feet, and a central angle af $29^{\circ} 07^{\prime} 38^{\prime \prime}$; thence run Westerly along the arc of said curve, 112.54 feet; to a point of reverse curvature of a curve concave Northerly having a radius of 132.76 feet, and a central angle of $4816^{\prime} 12^{\prime \prime}$; thence run Westerly along the arc of said curve, 111.85 feet; to a point on a non-tangent curve concave Northeasterly having a radius of 234.18 feet, and a central angle of $14^{\circ} 51^{\prime} 36^{\prime \prime}$; thence from a tangent bearing of $N 64.15^{\prime} 37^{\prime \prime} \mathrm{W}$ run Northwesterly along the arc of said curve, 60.74 feet; thence $S 24^{\circ} 23^{\prime} 32^{\prime \prime} \mathrm{E}, 34.06$ feet; thence $S 184^{\circ} 09^{\prime \prime} \mathrm{E}, 78.70$ feet to a point on a non-tangent curve concave Northwesterly having a radius of 25.00 feet, and a central angle of $115^{\circ} 40^{\circ} 49^{\prime \prime}$; thence from a tangent bearing of $S 170^{\circ} 50^{\prime 2} 29^{\prime \prime} \mathrm{E}$ run Southwesterly along the arc of said curve, 50.48 feet; thence $\mathrm{N} 82^{\circ} 09^{\prime} 40^{\prime \prime} \mathrm{W}, 26.47$ feet; thence $S 26^{\circ} 43^{\prime} 01^{\prime \prime} \mathrm{W}, 107.99$ feet; thence $S 13^{\circ} 53^{\prime} 13^{\prime \prime} \mathrm{W}, 84.71$ feet; thence $S 20^{\circ} 06^{\prime} 37^{\prime \prime} \mathrm{W}$, 86.21 feet; thence $S$ $22^{\circ} 42^{\prime} 17^{\prime \prime} \mathrm{W}, 90.27$ feet; thence $S 48^{\prime} 33^{\prime} 38^{\prime \prime} \mathrm{W}, 93.96$ feet; thence $S 51^{\circ} 48^{\prime} 05^{\prime \prime} \mathrm{W}, 58.47$ feet; thence $S 70^{\circ} 41^{\circ} 52^{\prime \prime} \mathrm{W}$, 98.39 feet; thence $S 75^{\circ} 48^{\prime} 30^{\prime \prime} \mathrm{W}, 82.70$ feet; thence $N 82^{\circ} 22^{\prime} 12^{\prime \prime} \mathrm{W}, 18.57$ feet; thence $S 59^{\circ} 48^{\prime} 12^{\prime \prime} \mathrm{W}, 61.99$ feet; thence $S 23^{\circ} 48^{\circ} 42^{\prime \prime} \mathrm{W}, 31.41$ feet; thence $S 21^{\circ} 34^{\prime \prime} 58^{\prime \prime} \mathrm{E}, 112.96$ feet; thence $S 25^{\circ} 04^{\circ} 56^{\prime \prime} \mathrm{E}$, 80.36 feet; thence $S$ $06.58^{\prime \prime} 19^{\prime \prime} \mathrm{E}, 51.79$ feet to a point of curvature of a curve concave Westerly having a radius of 25.00 feet, and a central angle of $54^{\prime \prime} 17^{\prime} 13^{\prime \prime}$; thence run Southerly along the arc of said curve, 23.69 feet; thence $S 47{ }^{\prime} 18^{\prime} 54^{\prime \prime} \mathrm{W}, 37.10$ feet: thence $S 03^{\prime} 48^{\prime} 45^{\prime \prime} \mathrm{E}, 24.29$ feet to a point of curvature of a curve concave Northwesterly having a radius of 25.00 feet, and a central angle of $79^{\prime \prime} 16^{\prime} 52^{\prime \prime}$; thence run Southwesterly alang the arc of said curve, 34.59 feet; thence $S 75^{\circ} 28^{\prime} 07^{\prime \prime} \mathrm{W}, 70.19$ feet to a point of curvature of a curve concave Northerly having a radius of 25.00 feet, and a central angle of $41^{\prime} 16^{\prime} 24^{\prime \prime}$; thence run Westerly along the arc of said curve, 18.01 feet; thence $N 63^{\circ} 15^{\prime} 30^{\prime \prime} \mathrm{W}, 63.09$ feet to a point on the Easterly right-of-way of RCID canal L-105 as described in Official Records Book 1896, Page 232 of the Public Records of this County, and a non-tangent curve concave Easterly having a radius of 1505.50 feet, and a central angle of $37^{\circ} 08^{\prime} 46^{\prime \prime}$; thence from a tangent bearing of $503^{\circ} 51^{\prime} 20^{\prime \prime} \mathrm{E}$ run Southerly along the arc of said curve and right-of-way, 976.05 feet; thence continue along said right-af-way $S 4^{\circ} 00^{\prime} 06^{\prime \prime} E, 193.39$ feet; thence $S$ $48^{\circ} 59^{\prime} 54^{\prime \prime}$ W, 100.00 feet to a point on the westerly right-af-way of said Canal; thence departing said Canal run, $N$ $87{ }^{\circ} 15^{\prime} 41^{\prime \prime} \mathrm{W}, 130.57$ feet; thence $N 63^{\circ} 21^{\prime} 34^{\circ} \mathrm{W}, 33.90$ feet; thence $\mathrm{N} 81^{\circ} 08^{\prime} 52^{\prime \prime} \mathrm{W}, 154.09$ feet; thence $N 39^{\circ} 33^{\prime} 00^{\prime \prime} \mathrm{W}$, 38.53 feet; thence $N 28^{\circ} 54^{\prime} 14^{\prime \prime} \mathrm{W}, 86.79$ feet; thence $N 28^{\circ} 30^{\circ} 43^{\prime \prime} \mathrm{W}, 101.63$ feet; thence $N .32^{\circ} 36^{\circ} 46^{*} \mathrm{~W}, 77.00$ feet; thence $N 39^{\circ} 30^{\prime} 36^{\prime \prime}$ W, 98.30 feet to a point of curvature of a curve concave Easterly having a radius of 25.00 feet, and a central angle of $3794^{\prime} 40^{\prime \prime}$; thence run Northerly along the arc of said curve, 16.25 feet; thence $\mathrm{N} 02.15^{\prime} 56^{\prime \prime} \mathrm{W}$, 56.50 feet; thence $N 39^{\circ} 36^{\prime} 59^{\prime \prime}$ W, 135.27 feet; thence $N 855^{\circ} 04^{\prime} 00^{\prime \prime} \mathrm{W}, 67.65$ feet to a point of curvature of a curve concave Northeasterly having a radius of 25.00 feet, and a central angle of $46^{\circ} 40^{\prime} 29^{\prime \prime}$; thence run Northwesterly along the arc of said curve, 20.37 feet; thence $N 38^{\circ} 23^{\prime} 30^{\prime \prime} \mathrm{W}, 64.62$ feet; thence $\mathrm{N} 64^{\circ} 16^{\prime} 04^{\prime \prime} \mathrm{W}, 16.33$ feet to a point of curvature of a curve concave Northeasterly having a radius of 25.00 feet, and a central angle of $58^{\circ} 38^{\prime} 45^{\prime \prime}$; thence run Northwesterly along the arc of said curve, 25.59 feet; thence $N 05.37^{\prime} 20^{\circ} \mathrm{W}, 20.54$ feet; thence $N 44^{\circ} 31^{\circ} 28^{\prime \prime} \mathrm{W}, 62.56$ feet; thence $523^{\circ} 42^{\prime} 54^{\prime \prime} \mathrm{W}, 95.95$ feet to a paint of curvature of a curve concave Northwesterly having a radius of 25.00 feet, and a central angle of $84^{\circ} 46^{\prime} 10^{\prime \prime}$; thence run Southwesterly along the arc of said curve, 36.99 feet; thence $N 71^{\circ} 30^{\prime} 56^{\prime \prime} \mathrm{W}, 65.59$ feet; thence $N 67^{\circ} 45^{\circ} 46^{\prime \prime} \mathrm{W}, 71.42$ feet; thence $\mathrm{N} 47^{\circ} 09^{\prime} 12^{\prime \prime} \mathrm{W}, 129.61$ feet; thence $N 28^{\circ} 09^{\prime} 10^{\prime \prime}$ W, 67.04 feet to a point of curvature of a curve concave Easterly having a radius of 25.00 feet, and a central angle of $58.17^{\prime} 03^{\prime \prime}$; thence run Northerly along the arc of said curve, 25.43 feet; thence $N 30^{\circ} 07^{\prime} 52^{\prime \prime} E, 66.18$ feet; thence $N$ $41^{\circ} 27^{\prime} 39^{\prime \prime} \mathrm{E}, 82.62$ feet; thence $N 28^{\circ} 03^{\prime} 16^{\prime \prime} \mathrm{E}, 61.53$ feet; thence $N 21^{\circ} 03^{\prime} 09^{\prime \prime} \mathrm{W}, 47.93$ feet; thence $N 17.13^{\prime} 11^{\prime \prime} \mathrm{W}$, 99.26 feet; thence $N 00^{\circ} 32^{\prime} 57^{\prime \prime} E, 48.45$ feet; thence $N 121^{\circ} 21^{\prime} 10^{\prime \prime} E, 151.79$ feet; thence $N 23^{\circ} 46^{\prime} 35^{\prime \prime} E$, 109.94 feet; thence $N 39^{\circ} 26^{\prime} 51^{\prime \prime} E, 91.52$ feet; thence $N 17^{\circ} 00^{\prime} 45^{\prime \prime} E, 45.16$ feet; thence $N 34^{\circ} 56^{\prime} 26^{\prime \prime} W$, 27.03 feet; thence $N$ $26^{\circ} 29^{\prime} 23^{\prime \prime}$ W, 104.81 feet; thence $S 48^{\circ} 40^{\prime} 54^{\prime \prime}$ W, 30.14 feet to a point on a non-tangent curve concave Southerly hoving a radius of 7.86 feet, and a central angle of $78^{\circ} 20^{\prime} 37^{\prime \prime}$; thence from a tangent bearing of $\mathrm{N} 28^{\circ} 56^{\prime} \mathrm{O} 3^{\prime \prime} \mathrm{W}$ run Westerly along the arc of said curve, 10.75 feet; to a point of compound curvature of a curve concave Southeasterly having a radius of 19.64 feet, and a central angle of $36.52^{\prime} 37^{\prime \prime}$; thence run Southwesterly along the arc of said curve, 12.64 feet; to a point of compound curvature of a curve concave Easterly having a radius of 3.95 feet, and a central angle of $74^{\prime 2} 25^{\prime} 35^{\prime \prime}$; thence run Southerly along the arc of said curve, 5.13 feet; thence $S^{\prime \prime} 38^{\prime} 34^{\circ} 51^{\prime \prime} E$, 13.88 feet; CONTINUED ON SHEET:4

|  | SURVEYING AND MAPPING DEPARTMENT P.O.B 10000 LAKE BUENA VISTA F. 32830-1000 PHONE (407)560-7118 FAX (407)560-7869 |  |  |  | $\begin{aligned} & \hline \text { DATE: } \\ & 12 / 07 / 17 \\ & \hline \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | PRGECT NAME <br> RCID WATER/WASTE WATER TERRITORY |  |  | SCALE |
|  |  | SURVEYTPE |  |  | $\begin{aligned} & \text { DRAWN BY: } \\ & \text { JLG } \end{aligned}$ |
|  |  | COMMENTS <br> EXHIAIT <br> A1 <br> SHEET 3 OF 31 SHEETS |  |  | $\begin{aligned} & \text { fLDVAME: } \\ & 10 \mathrm{JG09096R2} \end{aligned}$ |

## CONTINUED FROM SHEET 3

thence $S 51^{\circ} 58^{\prime} 30^{\prime \prime} \mathrm{W}, 145.54$ feet; thence $N 37.57^{\prime} 09^{\prime \prime} \mathrm{W}, 16.70$ feet to a point on a non-tangent curve concave Northeasterly having a radius of 1080.42 feet, and a central angle of $20.21^{\prime} 16^{\prime \prime}$; thence from a tangent bearing of $N$ $48^{\circ} 06^{\prime} 54^{\prime \prime} \mathrm{W}$ run Northwesterly along the arc of said curve, 383.82 feet; thence $N 37.56^{\prime} 18^{\prime \prime}$ W, 17.87 feet; thence $N$ $30{ }^{\circ} 54^{\prime} 21^{\prime \prime} \mathrm{W}, 193.79$ feet to a point on a non-tangent curve concove Southeasterly having a radius of 762.70 feet, and a central ongle of $08^{\circ} 52^{\prime} 54^{\prime \prime}$; thence from a tangent bearing of $S 63^{\circ} 58^{\prime} 49^{\prime \prime} \mathrm{W}$ run Southwesterly along the arc of said curve, 118.23 feet; thence $S 55^{\circ} 05^{\prime} 55^{\circ} \mathrm{W}, 58.77$. feet to a point of curvature of a curve concave Southeasterly having a rodius of 160.82 feet, and a central angle of $1916^{\prime} 01^{\prime \prime}$; thence run Southwesterly along the arc of said curve, 54.08 feet; to a point of reverse curvature of a curve concave Northwesterly having a radius of 159.35 feet, and a central angle of $36^{\circ} 15^{\prime} 00^{\prime \prime}$; thence run Southwesterly along the arc of said curve, 100.82 feet; thence $S 72^{\circ} 04^{\prime} 54^{\prime \prime} \mathrm{W}, 26.78$ feet to a paint of curvature of a curve concave Southeasterly having a radius af 158.03 feet , and a central angle of $21^{\circ} 54^{\prime} 44^{\prime \prime}$; thence run Southwesterly along the orc of said curve, 60.44 feet; to a point on a non-tangent curve concave Northeasterly having a radius of 52.89 feet, and a central angle of $104^{\circ} 26^{\prime} 29^{\prime \prime}$; thence from a tangent bearing of $575^{\circ} 27^{\prime} 00^{\prime \prime} \mathrm{W}$ run Northwesterly along the arc of said curve, 96.41 feet; thence $N 00^{\circ} 06^{\prime} 37^{\prime \prime} W, 54.31$ feet; thence $N 74^{\circ} 49^{\prime} 42^{\prime \prime} \mathrm{W}, 43.41$ feet; thence $S 44^{\circ} 47^{\prime} 41^{\prime \prime} \mathrm{W}, 145.43$ feet; thence $S 455^{\circ} 05^{\prime} 06^{\prime \prime} \mathrm{E}, 18.68$ feet; thence $S 03^{\circ} 14^{\prime} 02^{\prime \prime} \mathrm{W}$, 84.66 feet; thence $S 05^{\circ} 12^{\prime} 38^{\prime \prime} E, 58.35$ feet to a point of curvature of a curve concave Easterly having a radius of 1125.00 feet, and a central angle of $27.57^{\prime} 29^{\prime \prime}$; thence run Southerly along the arc of said curve, 548.95 feet; thence $S 33^{\prime} 10^{\prime} 07^{\prime \prime} E_{1} 163.59$ feet to a point of curvature of a curve concave Westerly having a radius of 492.00 feet, and a central angle of $26^{\circ} 59^{\prime} 13^{\prime \prime}$; thence run Southerly along the arc of soid curve, 231.74 feet; thence $\mathrm{N} 86^{\circ} 26^{\prime} 26^{\prime \prime} \mathrm{E}$, 126.87 feet; thence $N 76^{\prime} 15^{\prime} 46^{\prime \prime}$ E, 63.89 feet; thence $S 64.36^{\prime} 17^{\prime \prime} \mathrm{E}, 118.17$ feet; thence $S 52^{\circ} 36^{\prime} 40^{\prime \prime} \mathrm{E}$, 63.05 feet; thence $S 45^{\circ} 16^{\prime} 16^{\prime \prime}$ E, 127.88 feet to a point of curvature of a curve concave Southwesterly having a radius of 25.00 feet, and a central angle of $35^{\prime} 13^{\prime} 41^{\prime \prime}$; thence run Southeasterly along the arc of said curve, 15.37 feet; thence $S$ $10.02^{\prime} 35^{\prime \prime} \mathrm{E}, 93.01$ feet to a point of curvature of a curve concave Westerly having a radius of 25.00 feet, and a central angle of $46^{\circ} 18^{\prime} 35^{\prime \prime}$; thence run Southerly along the arc of said curve, 20.21 feet; thence $S 36^{\circ} 16^{\prime} 00^{\prime \prime} \mathrm{W}, 28.53$ feet; thence $S 20^{\circ} 23^{\prime} 46^{\prime \prime} \mathrm{W}, 184.90$ feet; thence $S 25^{\circ} 05^{\prime} 40^{\prime \prime} \mathrm{W}, 31.33$ feet to a point on a non-tangent curve concave Northwesterly having a radius of 25.00 feet, and a central angle of $33^{\circ} 58^{\prime \prime} 13^{\prime \prime}$; thence from a tangent bearing of $S$ $211^{\circ} 14^{\prime \prime} 14^{\prime \prime} \mathrm{W}$ run Sauthwesterly along the arc of said curve, 14.82 feet; thence $555^{\circ} 12^{\circ} 27^{\prime \prime} \mathrm{W}, 19.76$ feet; thence $S$ $18^{\circ} 42^{\prime} 59^{\prime \prime} \mathrm{W}, 22.23$ feet to a point on a non-tangent curve concave Southwesterly having a radius of 1908.34 feet, and a central angle of $22^{\circ} 05^{\prime} 51^{\prime \prime}$; thence from a tangent bearing of $S 75^{\circ} 17^{\prime} 36^{\prime \prime} \mathrm{E}$ run Southeasterly along the are of said curve, 736.00 feet; thence $S 53^{\circ} 11^{\prime} 44^{\prime \prime} \mathrm{E}, 1498.58$ feet to a point of curvature of a curve concave Northeasterly having a rodius of 950.92 feet, and a central angle of $14^{\circ} 29^{\prime} 06^{\prime \prime}$; thence run Southeasterly along the arc of said curve, 240.40 feet; to a point of compound curvature of a curve concove Northerly having a radius of 513.39 feet, and a central ongle of $13^{\circ} 13^{\prime} 42^{\prime \prime}$; thence run Easterly along the arc of soid curve, 118.53 feet; thence $S 80^{\circ} 54^{\prime} 32^{\prime \prime} \mathrm{E}, 34.76$ feet to a paint of curvature of a curve concave Northerly having a radius of 1109.03 feet, and a central angle of $07{ }^{\prime} 17^{\prime} 21^{\prime \prime}$; thence run Easterly along the arc of said curve, 141.09 feet; thence $S 88^{\prime} 11^{\prime} 54^{\prime \prime} \mathrm{E}, 77.05$ feet; thence $S$ $89^{\circ} 29^{\prime} 03^{\circ} \mathrm{E}, 140.11$ feet; thence $S 9^{\circ} 29^{\prime} 03^{\prime \prime} \mathrm{E}, 433.68$ feet; thence $\mathrm{N} 89^{\circ} 58^{\circ} 59^{\prime \prime} \mathrm{E}, 1465.17$ feet; thence $\mathrm{N} 00^{\circ} 00^{\prime} 00^{\prime \prime}$ $E, 131.18$ feet; thence $N 45^{\circ} 00^{\prime} 00^{\prime \prime} W_{1} 71.68$ feet; thence $N 00^{\circ} 00^{\prime} 00^{\prime \prime} \mathrm{E}, 633.08$ feet; thence $N 89^{\circ} 59^{\prime} 00^{\prime \prime} \mathrm{W}, 445.76$ feet; thence $N 00^{\circ} 27^{\prime} 46^{\prime \prime} \mathrm{E}, 673.19$ feet; thence $S 89^{\circ} 58^{\prime} 17^{\prime \prime} \mathrm{E}, 398.81$ feet; thence $N 00^{\circ} 00^{\prime} 00^{\prime \prime} \mathrm{E}, 753.74$ feet; thence $N 90.00^{\prime} 00^{\prime \prime} W, 362.43$ feet; thence $N 05^{\circ} 16^{\prime} 59^{\prime \prime} W_{1} 106.23$ feet; thence $N 26^{\circ} 33^{\prime} 54^{\prime \prime} W_{1} 135.35$ feet; thence $N$ $47^{\circ} 32^{\circ} 44^{\prime \prime}$ E, 146.69 feet; thence $N 11^{\prime} 28^{\prime} 34^{\prime \prime} \mathrm{E}, 24.04$ feet to a paint of curvature of a curve concave Westerly having a radius of 15.00 feet, and a central angle of $52.09^{\prime} 22^{\prime \prime}$; thence run Northerly along the arc of said curve, 13.65 feet; thence $N 40^{\circ} 40^{\prime} 48^{\prime \prime}$ W. 82.81 feet: thence $N 90^{\circ} 00^{\prime} 00^{\prime \prime} \mathrm{W}, 73.87$ feet to a point on a non-tangent curve concave Westerly having a radius of 1396.50 feet, and a central angle of $06^{\circ} 53^{\prime \prime} 10^{\prime \prime}$; thence from a tangent bearing of N $07.09^{\circ} 56^{\prime \prime} \mathrm{E}$ run Northerly along the orc of said curve, 167.84 feet; thence $\mathrm{N} 00^{\circ} 16^{\prime} 47^{\prime \prime} \mathrm{E}, 0.50$ feet to the Northwest corner of the Northeast $1 / 4$ of the Southwest $1 / 4$ of Section 17 Township 24 South Range 28 East; thence $S$ $89.56^{\circ} 53^{\prime \prime} \mathrm{E}, 3992.90$ feet along the North line of the South half of Section 17, to the East $1 / 4$ corner of Section 17: thence $S 0^{\circ} 24^{\prime \prime} 52^{\prime \prime}$ W, 2682.68 feet along the East section line of Section 17 to the Southeast corner of Section 17 and the Northeast comer of Section 20, Township 24 South, Range 28 East; thence $500^{\circ} 01^{\circ} 36^{\prime \prime} E, 1333.66$ feet along the East section line of Section 20 to the Southeast corner of the Northeast $1 / 4$ of the Northeast $1 / 4$ of Section 20 CONTINUED ON SHEET 5

|  | SURVEYNG AND MAPPING DEPARTMENT P.O.B. 10000 LAKE BUENA MSTA PH $32830-1000$ FAX (407)560-7869 | FLWG AREA DISNEY OVERALL | $\begin{aligned} & \text { DATE } \\ & 12 / 07 / 17 \\ & \hline \end{aligned}$ |
| :---: | :---: | :---: | :---: |
|  |  | RCID WATER/WASTE WATER TERRITORY |  |
|  |  | SKETTCH OF DESCRIPTION | DRAWN EY: <br> JLG |
|  |  | EXHISIT A1, SHEET 4 OF 31 SHEETS | FllesaMEs |

## CONTINUED FROM SHEET 4

and the Southwest corner of the Northwest $1 / 4$ of the Northwest $1 / 4$ of Section 21, Township 24 South, Range 28 East; thence $N 89.57^{\prime} 37^{\prime \prime} \mathrm{E}, 670.11$ feet to the Northwest corner of the Northeast $1 / 4$ of the Southwest $1 / 4$ of the Northwest $1 / 4$ of Section 21 ; thence $S 00.08^{\prime} 32^{\prime \prime} \mathrm{E}, 668.06$ feet to the Southwest corner thereof; thence $S 89^{\circ} 55^{\prime} 30^{\prime \prime}$ E, 671.45 feet to the Northeast corner of the Southeast $1 / 4$ of the Southwest $1 / 4$ of the Northwest $1 / 4$ of Section 21; thence $S 00^{\circ} 15^{\prime} 27^{\prime \prime} E, 669.41$ feet to the Northwest corner of the Northeast $1 / 4$ of the Southwest $1 / 4$ of Section 21; thence $500^{\circ} 44^{\prime \prime} 42^{\prime \prime} \mathrm{E}, 656.38$ feet to the Northwest corner of Lot 85 , Munger and Company Subdivision of Section 21, according to the Plat recorded in Plat Book E Page 22 of the Public Records of this County, thence $S 89^{\circ} 51^{\circ} 01^{\prime \prime} \mathrm{E}$, 335.66 feet to the Northeast corner of said Lot 85 ; thence $S 00^{\circ} 40^{\circ} 49^{\prime \prime} \mathrm{E}, 656.31$ feet to the Southeast corner of Lot 85; thence $S 89.53^{\prime} 15^{\prime \prime} \mathrm{E}, 1004.75$ feet along the North line of the Southeast $1 / 4$ of the Southwest $1 / 4$ of Section 21 to the Northeast comer thereof; thence $S 00^{\prime} 29^{\prime} 10^{\prime \prime} E, 655.63$ feet along the West line of the Northwest 1/4, Southwest $1 / 4$ of the Southeast $1 / 4$ of Section 21 to the Southwest corner thereof; thence $N 89.20^{\prime} 56^{\prime \prime}$ E. 666.99 feet along the South line of the Northwest $1 / 4$, Southwest $1 / 4$ of the Southeast $1 / 4$ of Section 21 to the Southeast corner thereof; thence $N 0021^{\prime} 22^{\prime \prime} \mathrm{W}, 652.39$ feet along the West line of the Northeast $1 / 4$, Southwest $1 / 4$ of the Southeast $1 / 4$ of Section 21 to the Northwest corner thereof; thence $N 89^{\circ} 37^{\prime} 38^{\prime \prime} \mathrm{E}, 2005.42$ feet along the North line of the South half of the Southeast $1 / 4$ of Section 21 to the Northeast corner thereof, said paint olso being the Southwest corner of the Northwest $1 / 4$ of the Southwest $1 / 4$ of Section 22, Township 24 Sauth, Range 28 East; thence $N 000^{\circ} 02^{\prime \prime} \mathrm{E}, 1285.39$ feet along the West line of Section 22 to the West $1 / 4$ corner of Section 22; thence $N$ $890^{\circ} 49^{\prime \prime} \mathrm{E}, 2691.31$ feet along the North line of the South half of Section 22 to the Westerly right-of-way of State Road 400 as shown in map section $75280-2465$ and dated $2 / 22 / 1993$; thence run along said right-of-way, 5 $38^{\prime} 29^{\prime} 42^{\prime \prime} \mathrm{W}, 7143.82$ feet to a point on the Westerly right-of-way line of State Road 536 as shown in map section 75000-2520 and dated 3/05/1998; thence departing State Road 400 run along State Road 536 the following courses; S $43^{\circ} 35^{\prime} 42^{\prime \prime} \mathrm{W}, 1571.48$ feet to a point on a non-tangent curve concave Northwesterly having a radius of 1809.86 feet, and a central angle of $37^{\circ} 23^{\prime} 06^{\prime \prime}$; thence from a tangent bearing of $\mathrm{S} 42^{\circ} 29^{\prime} 42^{\prime \prime} \mathrm{W}$ run Southwesterly along the arc of soid curve, 1180.92 feet; thence S $79.52^{\prime} 51^{\prime \prime}$ W, 1498.72 feet to a point on the West line of Section 28 , and on the East line of Section 29, Township 24 South, Range 28 East, said point lying $\mathrm{N} 00^{\circ} 00^{\prime} 07^{\prime \prime} \mathrm{W} .387 .61$ feet from the Southwest corner of Section 28; thence $579^{\circ} 52^{\prime} 53^{\prime \prime} \mathrm{W}$, 95.47 feet to a point of curvature of a curve concave Northerly having a radius of 2191.83 feet and a central angle of $32^{\prime} 28^{\prime} 09^{\prime \prime}$; thence run Westerly along the arc of said curve, 1242.10 feet; thence $N 69^{\circ} 59^{\prime} 50^{\prime \prime} \mathrm{W}, 311.61$ feet; thence run $\mathrm{S} 23^{\prime 2} 29^{\prime} 47^{\prime \prime} \mathrm{W}, 304.91$ feet to a point on a non-tangent curve concave Southwesterly, having a radius of 11402.16 feet and a central angle of $00^{\circ} 29^{\prime \prime} 43^{\prime \prime}$; thence from a tangent bearing of $565^{\circ} 33^{\prime} 17^{\prime \prime} \mathrm{E}$, run Southeasterly along the arc of said curve, 98.56 feet; thence 5 $58^{\circ} 56^{\prime} 26^{\prime \prime} \mathrm{E}, 509.41$ feet to a point on a non-tangent curve concave Sauthwesterly, having a radius of 900.00 feet and a central angle of $02^{\prime} 31^{\prime} 40^{\prime \prime}$; thence run Southeasterly along the arc of said curve 39.70 feet to a point on the South line the Southeast $1 / 4$ of Section 29 , said point lying $N 89^{\circ} 50^{\prime} 43^{\prime \prime}$ W, 1167.48 feet from the Southeast corner of Section 29; thence leaving said right-of-way, run $N 89^{\circ} 50^{\prime} 43^{\prime \prime} W$ along the South line of the Southeast $1 / 4$ of Section $29,1496.10$ feet, to the South Quarter corner thereof; thence $N 89^{\circ} 50^{\prime} 42^{\prime \prime}$ W, 2152.59 feet along the South line of the Southwest $1 / 4$ of Section 29 to a point on the right-of-way of Chelonia Parkway as shown on the Plat of Bonnet Creek Resort recorded in Plat Book 56, Page 41 of the Public Records of this County, thence run along said right-of-way the following courses; due North 163.29 feet to the point of curvature of a curve concave Southeasterly, having a radius of 675.00 feet and a central angle of $45^{\circ} 40^{\prime} 47^{\prime \prime}$; thence run Northeasterly along the arc of said curve 538.15 feet to a point of reverse curvature of a curve concave Westerly, having a radius of 825.00 feet and a central angle of $98^{\circ} 34^{\prime} 08^{\prime \prime}$; thence run Northeasterly and Northwesterly along the arc of said curve 1419.29 feet to a point of reverse curvature of a curve concave Northeasterly having a radius of 500.84 feet and a central angle of $22^{\circ} 53^{\circ} 21^{\prime \prime}$; thence run Northwesterly and Northerly along the arc of said curve 200.08 feet; thence $\mathrm{N} 30^{\circ} 00^{\circ} 00^{\prime \prime} \mathrm{W}, 607.96$ feet; thence due North, 86.60 feet; thence due West 67.60 feet to a point of curvature of a curve concave Southerly having a radius of 611.16 feet and a central angle of $19^{\circ} 01^{\prime} 18^{\prime \prime}$; thence run Westerly along the arc of said curve and Southerly right-of-way 202.90 feet; thence S $57^{\circ} 06^{\prime} 40^{\prime \prime}$ E, 167.71 feet; S $30^{\circ} 00^{\prime} 00 \prime$ E, 180.00 feet; S $06^{\circ} 15^{\prime} 02^{\prime \prime} \mathrm{E}, 54.63$ feet; S $30^{\circ} 00^{\prime} 00^{\prime \prime}$ E. 408.17 feet to a point of curvature of a curve concave Northeasterly, having a radius of 650.84 feet and a central angle of $22^{\circ} 53^{\prime} 21^{\prime \prime}$; run Southeosterly along the arc of said curve 260.00 feet; to a point of reverse CONTINUED ON SHEET 6

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|  | MAPPING DEPARTMENT P.O.B. 10000 | $\stackrel{\text { Proiect }}{\text { RCID }}$ WATER/WASTE WATER TERRITORY |  |
| mentitit mintishatens | Lake buena vista <br> FL. 32830-1000 |  | DRAWN EY: <br> JLG |
|  | FAX (407)560-7869 | EXHMEIT A1, SHEET 5 OF 31 SHEETS | FILDNAME: $\begin{aligned} & \text { FLLENANE: } \\ & 10 . J G 09096 R 2 \end{aligned}$ |

## CONTINUED FROM SHEET 5

curvature of a curve concave Westerly, having a radius of 675.00 feet and a central angle of $98^{\circ} 34^{\prime} 08^{\prime \prime}$; thence run Southeasterly and Southwesterly along the arc of said curve 1161.24 feet to a point of reverse curvature of a curve concave Southeasterly, having a radius of 825.00 feet and a central angle of $45^{\circ} 40^{\prime} 47^{\prime \prime}$; thence run Southwesterly along the arc of said curve, 657.74 feet; thence due South, 162.89 feet to the South line of the Southwest $1 / 4$ of Section 29; thence departing the right-of-way line of Chelonia Parkway run $\mathrm{N} 89^{\circ} 50^{\prime} 42^{\prime \prime} \mathrm{W}$ along the South line of the Southwest $1 / 4$ of Section 29, 360.99 feet to the Southwest corner of Section 29 and the Northeast corner of Section 31, Township 24 South, Range 28 East; thence $S 00^{\circ} 40^{\prime} 50^{\prime \prime} \mathrm{E}, 2749.41$ feet along the East line of the Northeast $1 / 4$ of Section 31 to the Southeast comer thereof; thence $S 00^{\circ} 27^{\prime} 13^{\prime \prime} \mathrm{W}, 2643.90$ feet along the East line of the Southeast $1 / 4$ of Section 31 to the Southeast corner of Section 31; thence $N 89^{\circ} 36^{\prime} 01^{\prime \prime} \mathrm{W}, 2646.94$ feet along the South line of the Southeost $1 / 4$ of Section 31 to the Southwest corner thereof; thence N 89.56'54" W, 2748.82 feet along the South line of the Southwest $1 / 4$ of Section 31 to the Southwest corner thereof and the Southeast corner of Section 36, Township 24 South Range 27 East; thence S $89.50^{\prime} 04^{\prime \prime} \mathrm{W} .2658 .48$ feet along the South line of the Southeast $1 / 4$ of Section 36 to the Southwest corner thereof; thence $S 89.46^{\prime} 36^{\prime \prime} \mathrm{W}, 2656.21$ feet along the South line of the Southwest $1 / 4$ of Section 36 to the Southwest corner thereof and the Southeast corner of Section 35, Township 24 South Range 27 East; thence $S 89^{\circ} 48^{\prime} 35^{\prime \prime} \mathrm{W}, 2652.59$ feet alang the South line of the Southeast $1 / 4$ of Section 35 to the Southwest corner thereof; thence $S 89^{\circ} 44^{\circ} 07^{\prime \prime} \mathrm{W}, 2661.05$ feet along the South line of the Southwest $1 / 4$ of Section 35 to the Southwest corner of said Section and the Southeast corner of Section 34, Township 24 South Range 27 East; thence $S 89^{\circ} 46^{\prime} 46^{\prime \prime}$ W, 3438.73 feet along the South line of Section 34 to a point on the boundary of Black Lake Village according to the Plat thereof recorded in Plat Book 75, Page 149 of the Public Records of this County, thence leaving the South line of Section 34, run along the Easterly and Northerly boundary of said Plat following courses; $N 00^{\prime} 13^{\prime} 59^{\prime \prime} \mathrm{W}, 29.01$ feet; $N 14^{\prime} 42^{\prime} 28^{\prime \prime} \mathrm{W}, 114.62$ feet; $N 06.53^{\prime} 49^{\prime \prime} \mathrm{W}, 123.97$ feet to a point of curvature of a curve concave Easterly having a radius of 25.00 feet, and a central angle of $16^{\circ} 36^{\prime} 26^{\prime \prime}$; run Northerly along the arc of said curve, 7.25 feet; $N 09^{\circ} 42^{\prime} 37^{\prime \prime} \mathrm{E}, 104.21$ feet to a point of curvature of a curve concave Southeasterly having a radius of 25.00 feet, and a central angle of $51^{\circ} 24^{\prime} 11^{\prime \prime}$; run Northeasterly alang the arc of said curve, 22.43 feet; $N 61^{\circ} 05^{\prime \prime} 48^{\prime \prime} \mathrm{E}, 53.88$ feet; $N 71^{\circ} 34^{\prime} 02^{\prime \prime} \mathrm{E}, 17.56$ feet; $N 18^{\circ} 25^{\prime} 51^{\prime \prime} \mathrm{W}, 18.21$ feet to a point on a non-tangent curve concave Northeasterly having a radius of 50.00 feet, and a central angle of $106^{\circ} 48^{\prime} 50^{\prime \prime}$; from a tangent bearing of $\mathrm{N} 80^{\circ} 45^{\prime} 36^{\prime \prime} \mathrm{W}$ run Northwesterly along the arc of said curve, 93.21 feet; $\mathrm{N} 31^{\circ} 47^{\prime} 40^{\prime \prime} \mathrm{W}, 44.69$ feet to a point on a non-tangent curve concave Northwesterly having a radius of 436.00 feet, and a central angle of $15^{\circ} 56^{\prime} 47^{\prime \prime}$; from $a$ tangent bearing of $S 58^{\prime} 12^{\prime} 21^{\prime \prime} \mathrm{W}$ run Southwesterly along the arc of said curve, 121.35 feet; $S$ $74^{\circ} 09^{\prime} 08^{\prime \prime} \mathrm{W}, 308.68$ feet to a point of curvoture of a curve concove Southeosterly having a radius of 514.00 feet, and a central angle of $20.05^{\prime} 00^{\prime \prime}$; run Southwesterly along the arc of said curve, 180.17 feet; $55^{\circ} 04^{\prime} 10^{\prime \prime} \mathrm{W}$, 67.69 feet to a point of curvature of a curve concave Northerly hoving a radius of 315.00 feet, and a central angle of $355^{\circ} 55^{\prime} 53^{\prime \prime}$; run Westerly along the arc of said curve, 197.54 feet; $N 899^{\circ} 59^{\prime} 58^{\prime \prime} \mathrm{W}, 83.84$ feet to a point of curvature of a curve concave Northerly having a radius of 381.00 feet and a central angle of $34{ }^{\circ} 07^{\prime} 58^{\prime \prime}$; run Westerly along the arc of said curve, 226.97 feet; to a point of reverse curvature of a curve concave Southerly having a radius of 384.88 feet, and a central angle of $34^{\circ} 00^{\prime} 28^{\prime \prime}$; run Westerly along the arc of said curve, 228.44 feet; ta a point of reverse curvature of a curve concave Northerly having a radius of 185.00 feet, and a central angle of $35{ }^{\circ} 39^{\prime} 45^{\prime \prime}$; run Westerly along the arc of said curve, 115.15 feet; to a point of compound curvature of a curve concave Easterly having a radius of 47.00 feet, and a central angle of $130^{\circ} 32^{\prime} 06^{\prime \prime}$; run Northerly along the arc of said curve, 107.08 feet; $N 76^{\circ} 19^{\prime} 21^{\prime \prime} E, 28.14$ feet; $S 8^{\circ} 22^{\prime} 47^{\prime \prime} E, 9.24$ feet; $N 75^{\circ} 08^{\prime} 23^{\prime \prime} E, 42.15$ feet; $N 66^{\circ} 44^{\prime} 45^{\prime \prime} E, 45.92$ feet; $N 58^{\prime \prime} 10^{\prime} 56^{\prime \prime} E$, 7.13 feet; $N$ $40^{\circ} 00^{\prime} 00^{\prime \prime} \mathrm{E}, 8.68$ feet; $N 28^{\circ} 21^{\prime \prime} 12^{\prime \prime} \mathrm{E}, 21.50$ feet; $N 19^{\circ} 11^{\prime} 05^{\prime \prime} \mathrm{E}, 7.97$ feet; $N 05^{\circ} 44^{\prime} 49^{\prime \prime} \mathrm{E}, 22.07$ feet; $N$ 09.37'03" $E$, 18.85 feet: $N 28^{\circ} 18^{\prime} 59^{\prime \prime}$ E, 25.32 feet; $N 39^{\circ} 33^{\prime} 24^{\prime \prime}$ E, 18.56 feet; N $51^{\circ} 48^{\prime} 12^{\prime \prime}$ E, 17.01 feet; $N 53^{\prime 2} 20^{\prime} 03^{\prime \prime}$ E, 12.93 feet;
 E, 25.25 feet; $S 70{ }^{\circ} 01^{\prime} 08^{\prime \prime} \mathrm{E}, 21.22$ feet; $S 76.11^{\prime} 40^{\prime \prime} \mathrm{E}, 28.29$ feet; $S 81^{\circ} 04^{\prime} 45^{\prime \prime} \mathrm{E}, 15.99$ feet; $S 63^{\prime 1} 15^{\prime} 14^{\prime \prime} \mathrm{E}, 32.58$ feet; $S 71^{\circ} 35^{\prime} 23^{\prime \prime} E, 7.28$ feet; $S ~ 83^{\circ} 45^{\prime} 15^{\prime \prime} E, 20.77$ feet; $N 86^{\circ} 06^{\prime} 18^{\prime \prime} E, 21.64$ feet; $S 75^{\circ} 49^{\prime} 09^{\prime \prime} E, 17.31$ feet; $S$ $87.55^{\prime} 16^{\prime \prime} E_{1} 10.48$ feet; $N 72^{\circ} 43^{\prime} 50^{\prime \prime} E_{1} 25.75$ feet; $N 60^{\circ} 42^{\prime} 21^{\prime \prime} E, 36.44$ feet; $N 77^{\prime} 16^{\prime} 53^{\prime \prime} E, 19.62$ feet; $N 68^{\circ} 37^{\prime} 24^{\prime \prime}$ E, 7.52 feet; $N 57^{\circ} 06^{\prime} 15^{\prime \prime}$ E, 21.62 feet; $N 48^{\circ} 30^{\prime} 29^{\prime \prime}$ E, 7.40 feet; $N 29^{\circ} 59^{\prime} 26^{\prime \prime} \mathrm{E}, 8.68$ feet; $N 13^{\circ} 42^{\prime} 55^{\prime \prime} E$, 39.82 feet;

CONTINUED ON SHEET 7

|  | surveting and MAPPING DEPARTMENT P.O.B. 10000 AL B URENA VSTA PL $32830-1000$ FAX (407)560-7869 | WLIG AREA DISNEY OVERALL | $\begin{aligned} & \text { DATE } \\ & 12 / 07 / 17 \\ & \hline \end{aligned}$ |
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|  |  | EXHIBIT A1 SHEET 6 OF 31 SHEETS | FLENAME <br> 10JG09096R2 |

## CONTINUED FROM SHEET 6

N $1006^{\circ} 24^{n}$ E, 32.03 feet; N $0143^{\prime} 31^{\prime \prime}$ W, 29.22 feet; N $05 \times 37^{\circ} 39^{\prime \prime}$ W, 26.82 feet; $N 1201^{\prime} 53^{\prime \prime} \mathrm{W}, 42.36$ feet; $N$ $21^{\circ} 06^{\prime} 43^{\prime \prime} \mathrm{W}, 7.72$ feet; $N 36^{\circ} 50^{\prime} 10^{\prime \prime} \mathrm{W}, 37.65$ feet; $N 47^{\circ} 37^{\prime} 33^{\prime \prime} \mathrm{W}, 25.00$ feet; $N 56^{\prime} 19^{\prime} 26^{\prime \prime}$ W, 44.83 feet; $N 49.30^{\prime} 53^{\prime \prime}$
 feet; $N 80^{\circ} 08^{\prime} 53^{\prime \prime} \mathrm{W}, 56.11$ feet; $N 89^{\circ} 26^{\prime} 00^{\prime \prime} \mathrm{W}, 8.09$ feet; $S 8^{\prime} 14^{\prime} 14^{\prime \prime} \mathrm{W}, 46.34$ feet; $S 78^{\circ} 42^{\prime} 25^{\prime \prime} \mathrm{W}, 40.49$ feet; $S$ $77^{\circ} 43^{\prime} 02^{\prime \prime} \mathrm{W}, 63.74$ feet; $S 79^{\circ} 09^{\prime} 43^{\prime \prime} \mathrm{W}, 47.65$ feet; S $72^{\circ} 48^{\prime} 44^{\prime \prime} \mathrm{W}, 44.03$ feet; $\mathrm{S} 63^{\prime \prime} 14^{\prime} 34^{\prime \prime} \mathrm{W}, 42.60$ feet; $S 57^{\circ} 48^{\prime} 39^{\prime \prime}$ W, 28.70 feet; $S 64^{\prime 2} 21^{\prime} 00^{\prime \prime} \mathrm{W}, 20.44$ feet; $S 67^{\circ} 06^{\prime} 48^{\prime \prime} \mathrm{W}, 29.21$ feet; $S 83^{\circ} 28^{\prime} 20^{\prime \prime} \mathrm{W}, 29.99$ feet; $S 83^{\circ} 04^{\prime} 31^{\prime \prime} \mathrm{W}, 27.06$ feet; $\mathrm{S} 84^{\circ} 19^{\prime} 19^{\prime \prime} \mathrm{W}, 42.81$ feet to a point of curvature of a curve concave Northeasterly having a radius of 50.00 feet, and a central angle of $83^{\prime} 36^{\circ} 01^{\prime \prime}$; run Northwesterly along the arc of said curve, 72.95 feet; to a point of compound curvature of a curve concove Easterly having a radius of 188.00 feet, and a centrol angle of $27^{\circ} 45^{\prime} 45^{\prime \prime}$; run Northerly along the arc of said curve, 91.10 feet; $589^{\circ} 52^{\prime} 10^{\prime \prime} \mathrm{W}, 174.16$ feet; thence departing said Plat run alang the West line of the Southwest $1 / 4$ of Section $34, \mathrm{~N} 00^{\circ} 00^{\prime} 19^{\prime \prime} \mathrm{E}, 313.89$ feet to the Northwest corner of the Southwest $1 / 4$ of the Southwest $1 / 4$ of Section 34 and the Northeast corner of the Southeast $1 / 4$ of the Southeast $1 / 4$ of Section 33, Township 24 South, Range 27 East; thence continue $\mathrm{N} 00^{\circ} 00^{\prime} 19^{\prime \prime} \mathrm{E} 498.35$ feet to the Southeast corner of the North $5 / 8$ of the Northeast $1 / 4$ of the Southeast $1 / 4$ of Section 33 ; thence run along the South line of the North $5 / 8$ of the Northeast $1 / 4$ of the Southeast $1 / 4$ of Section $33, N 89^{\prime} 47^{\prime} 57^{\prime \prime} \mathrm{W}, 1326.58$ feet to the Southwest corner thereof; thence run along the West line of the North $5 / 8$ of the Northeast $1 / 4$, of the Southeast $1 / 4$ of Section $33, N$ $00^{\circ} 00^{\prime} 31^{\prime \prime} \mathrm{E}, 835.26$ feet to the Northwest corner thereof; thence run along the West line of the Sautheast $1 / 4$ of the Northeast $1 / 4$ of Section $33, \mathrm{~N} 00^{\circ} 00^{\prime} 25^{\prime \prime} \mathrm{E}, 1321.43$ feet to the Northwest corner thereof; thence run along the North line of the Southeost $1 / 4$ of the Northeast $1 / 4$ of Section $3.3, \mathrm{~S} 89^{\circ} 55^{\prime} 44^{\prime \prime} \mathrm{E}, 1326.40$ feet; to the Northeast corner thereof; thence run along the West line of the Northwest $1 / 4$ of Section 34 Township 24 South Range 27 East, $N$ $00^{\circ} 00^{\prime} 06^{\prime \prime} \mathrm{E}, 1329.09$ feet to the Northwest corner thereof; thence $\mathrm{N} 89^{\circ} 53^{\prime} 53^{\prime \prime} \mathrm{E}, 2679.47$ feet along the North line of the Northwest $1 / 4$ of Section 34 to the Northeast corner thereof and the Southwest corner of the Southeost $1 / 4$ of Section 27. Township 24 South, Range 27 East; thence $N 00^{\circ} 01^{\prime} 11^{\prime \prime} \mathrm{W}, 3964.69$ feet along the West line of the East $1 / 2$ of Section 27 to the Southeast corner of the Northeast $1 / 4$ of the Northwest $1 / 4$ of Section 27; thence $S$ $89.37^{\prime} 54^{\prime \prime}$ W, 1332.15 feet along the South line of the Northeast $1 / 4$ of the Northwest $1 / 4$ of Section 27 to the Southwest corner thereof; thence $N 00^{\circ} 08^{\prime} 12^{\prime \prime} \mathrm{E}, 1330.97$ feet along the West line of the Northeast $1 / 4$ of the Northwest $1 / 4$ of Section 27 to the Northwest corner thereof; thence S $89^{\circ} 46^{\circ} 29^{\prime \prime} \mathrm{W}, 1328.51$ feet along the North line of the Northwest $1 / 4$ of Section 27 to the Northwest corner of Section 27 and the Northeast corner of Section 28, Township 24 South, Range 27 East; thence $S 9^{\circ} 4 B^{\prime} 06^{\prime \prime}$ W, 1331.20 feet alang the North line of the Northeast $1 / 4$ of the Northeast $1 / 4$ of Section 28, to the Northeast corner of the West $1 / 2$ of the Northeast $1 / 4$ of Section 28; thence S $00^{\prime} 12^{\prime} 18^{\prime \prime} \mathrm{W}, 882.69$ feet along the East line of the West $1 / 2$ and the Northeast $1 / 4$ of Section 28 , Township 24 South, Range 27 E to a point on the Westerly right of way line of State Road 429 as described in Official Records Book 7070, Page 2553 and Book 7106. Page 2802 of the Public Records of Orange County, Florida and a point on a non-tangent curve concave Southwesterly having a radius of 2204.09 feet, and a central angle of $07^{\circ} 27^{\prime} 37^{\prime \prime}$; thence from a tangent bearing of $N 29^{\prime} 38^{\prime} 58^{\prime \prime} \mathrm{W}$ run Northwesterly along the arc of said curve and right of way line, 286.99 feet; thence continue along said right of way line the following two courses; $N 37^{\prime} 06^{\prime} 36^{\prime \prime} \mathrm{W}, 690.17$ feet to a point on a non-tangent curve concave 'Northeasterly having a radius of 770.43 feet, and a central angle of $09^{\circ} 59^{\prime \prime} 15^{\prime \prime}$ : thence from a tangent bearing of $\mathrm{N} 39^{\circ} 00^{\prime} 55^{\prime \prime} \mathrm{W}$ run Northwesterly along the arc of said curve, 134.30 feet; thence $N$ $\mathrm{BB}^{\circ} 43^{\prime} 15^{\prime \prime} \mathrm{W}, 555.85$ feet to a point on the Easterly right of way line of Flamingo Crossing Blva. as described in Official Records Book 10815, Page 4619 of the Public Records of Orange County, Florida and a non-tangent curve concave Westerly having a radius of 1010.00 feet, and a central angle of $0159^{\prime} 18^{\prime \prime}$; thence from a tangent bearing of $S$ $05^{\circ} 40^{\prime} 55^{\prime \prime} \mathrm{E}$ run Southerly along the arc of said curve and right of way line, 35.05 ; thence $\mathrm{S} 89^{\circ} 48^{\prime \prime} 06^{\prime \prime} \mathrm{W}, 125.95$ feet along the South line of the Southeast $1 / 4$ of Section 21, Township 24 South, Range 27 East to the Southwest corner thereof: thence S $89^{\circ} 49^{\prime} 36^{\prime \prime} \mathrm{W}, 483.70$ feet; along the South line of the Southwest $1 / 4$ of Section 21, Township 24 South, Range 27 East; thence $N 4017^{\prime} 32^{\prime \prime} \mathrm{W}, 323.52$ feet; thence $N 32^{\prime} 21^{\prime} 38^{\prime \prime} \mathrm{W}, 271.63$ feet; thence $\mathrm{N} 34^{\prime} 30^{\prime} 31^{\prime \prime} \mathrm{W}$, 120.76 feet; thence $N 46^{\circ} 26^{\prime} 37^{\prime \prime} \cdot \mathrm{W}, 108.80$ feet; thence $589^{\circ} 49^{\prime} 14^{\prime \prime} \mathrm{W}, 28.71$ feet to a point of curvature of a curve concave Southerly having a radius of 934.00 feet, and a central angle of $01^{\circ} 05^{\prime} 30^{\prime \prime}$; thence run Westerly along the arc of said curve, 17.79 feet; thence $S 00^{\prime} 10^{\prime} 31^{\prime \prime} \mathrm{E}, 11.26$ feet; thence $S 89^{\circ} 49^{\prime} 29^{\prime \prime} \mathrm{W}, 28.35$ feet; thence $S 04^{\circ} 02^{\circ} 58^{\prime \prime} \mathrm{E}$, CONTINUED ON SHEET 8

|  |  | FLLIG AREA DISNEY OVERALL | $12 / 07 / 17$ |
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## CONTINUED FROM SHEET 7

4.66 feet; thence S $86^{\circ} 05^{\prime} 06^{\prime \prime}$ W, 22.85 feet; thence $N 03^{\circ} 54^{\prime \prime} 54^{\prime \prime}$ W, 6.14 feet; thence $S 89^{\circ} 49^{\prime} 29^{\prime \prime}$ W, 173.97 feet to a point of curvature of a curve concave Northerly having a radius of 2158.53 feet, and a central angle of $24.05^{\prime} 38^{\prime \prime}$; thence run Westerly along the arc of said curve, 907.70 feet; thence $N 66^{\circ} 04^{\prime} 53^{\prime \prime} \mathrm{W}, 548.81$ feet; thence $\mathrm{N} 00^{\circ} 35^{\prime} 44^{\prime \prime}$ E, 1606.72 feet along the West line of the Southwest $1 / 4$ of Section 21 . Township 24 South, Range 27 East to the Northwest corner thereof; thence $N 00^{\circ} 35^{\prime} 56^{\prime \prime}$ E, 2659.37 feet along the West line of the Northwest $1 / 4$ of Section 21 to the Northwest corner of Section 21 and the Southeast corner of Section 17, Township 24 South, Range 27 East; thence $\mathrm{N} 00^{\circ} 02^{\prime} 13^{\prime \prime} \mathrm{E}, 2669.40$ feet along the East line of the Southeast $1 / 4$ of Section 17 to the Northeast corner thereof; thence S $89^{\circ} 43^{\prime} 49^{\prime \prime} \mathrm{W}, 1347.90$ feet along the South line of the East $1 / 2$ of the Northeast $1 / 4$ of Section 17, to the Southwest corner thereof; thence $N 00118^{\prime} 18^{\prime \prime} \mathrm{W}, 2652.68$ feet along the West line of the East $1 / 2$ of the Northeast $1 / 4$ of Section 17 to the Northwest corner thereaf; thence $5899^{\prime \prime} 39^{\prime} 31^{\prime \prime} \mathrm{W}, 2661.03$ feet along the North line of Section 17 to the Northwest corner of the Northeast $1 / 4$ of the Northwest $1 / 4$ of Section 17 and the Southwest corner of the Southeast $1 / 4$ of the Southwest $1 / 4$ of Section 8 , Township 24 South. Range 27 East; thence $N$ $00 \cdot 24^{\prime} 44^{\prime \prime}$ E, 242.11 feet along the West line of the Southeast $1 / 4$ of the Southwest $1 / 4$ of Section 8 to a point on the Easterly right-of-way line of County Road 545 as described in Deed Book 402, Page 355 of the Public Records of this County, said point being a point on a non-tangent curve concave Westerly, having a radius of 2826.01 feet, and a central angle of $19^{\circ} 14^{\prime} 15^{\prime \prime}$; thence from a tangent bearing of $\mathrm{N} 18.34^{\circ} 50^{\prime \prime}$ E, run Northerly along the arc of said curve and right-of-way, 948.86 feet; thence continue along said right-of-way, $\mathrm{N} 00^{\circ} 39^{\prime} 25^{\prime \prime} \mathrm{W}, 141.86$ feet; thence N $89^{\circ} 41^{\prime} 27^{\prime \prime}$ E, 1188.92 feet along the North line of the Southeast $1 / 4$ of the Southwest $1 / 4$ of Section 8 to the Northeast corner thereof; thence $N 00^{\prime} 15^{\prime} 09^{\prime \prime} \mathrm{E}, 1315.34$ feet along the West line of the Northwest $1 / 4$ of the Southeast $1 / 4$ of Section 8 to the Northwest corner thereof; thence $N 00.14^{\prime} 57^{\prime \prime} E, 50.00$ feet along the West line of the Northeast $1 / 4$ of Section 8 to a point on the Northerly right-of-way line of Flamingo Crossings Boulevard as described in Official Records Book 9782, Page 7172 of the Public Records of this County, thence run along said right-of-way line the following three courses; $N 89^{\circ} 43^{\prime} 25^{\prime \prime} \mathrm{E}, 671.30$ feet; $N 23^{\circ} 57^{\prime} 49^{\prime \prime} \mathrm{E}, 158.82$ feet to a point on a non-tangent curve concave Southwesterly having a radius of 2750.09 feet, and a central angle of $04.43^{\prime} 07^{\prime \prime}$; from a tangent bearing of $S 3376^{\prime} 29^{\prime \prime} \mathrm{E}$ run Southeasterly along the arc of said curve. 226.49 feet; thence $\mathrm{N} 89^{\circ} 43^{\prime} 24^{\prime \prime} \mathrm{E}$, 1808.38 feet along the North line of the Southeast $1 / 4$ of Section 8 to the Northeast corner thereof and the Northwest corner of the Southwest $1 / 4$ of Section 9, Township 24 South, Range 27 East; thence run $\mathrm{N} 89^{\circ} 44^{\prime} 05^{\prime \prime} \mathrm{E}$, 1325.36 feet along the North line of the Northwest $1 / 4$ of the Southwest $1 / 4$ of Section 9 to the Northeast corner thereof, thence $S 00{ }^{\circ} 8^{\prime} 51^{\prime \prime}$ W, 1314.23 feet along the East line of the Northwest $1 / 4$ of the Southwest $1 / 4$ of Section 9 to the Southeast corner thereof; thence $N 89^{\circ} 45^{\prime \prime} 10^{\prime \prime} \mathrm{E}, 1327.55$ feet along the North line of the Southeast $1 / 4$ of the Southwest $1 / 4$ of Section 9 to the Northeast corner thereof; thence $S 00^{\circ} 03^{\prime} 05^{\prime \prime} \mathrm{W}, 1314.64$ feet along the East line of the Southeast $1 / 4$ of the Southwest $1 / 4$ of Section 9 to the Southeast corner of the Southwest $1 / 4$ of Section 9 ; thence $N 899^{\circ} 53^{\prime} 46^{\prime \prime}$ E, 2633.36 feet along the South line of the Southeast $1 / 4$ of Section 9 to the Southeast corner thereof and the Southwest corner of Section 10, Township 24 South, Range 27 East; thence $\mathrm{N} 00^{\prime} 15^{\prime} 35^{\prime \prime} \mathrm{E}$, 5286.81 feet along the West section line of Section 10 to the Northwest corner thereof and the Southwest corner of Section 3. Township 24 South. Range 27 East; thence $N 0011^{\prime} 50^{\prime \prime} \mathrm{W}, 2661.64$ feet along the West line of the Southwest 1/4, Section 3 to the Northwest corner thereof; thence $N 899^{\prime} 39^{\prime \prime} 5 \mathrm{E}, 3976.31$ feet along the North line of the South half of Section 3 to the Northeast corner of the Northwest $1 / 4$ of the Southeast $1 / 4$ of Section 3 ; thence S $00^{\circ} 04^{\prime} 39^{\prime \prime} \mathrm{E}, 1326.78$ feet along the East line of the Northwest $1 / 4$ of the Southeast $1 / 4$ of Section 3 to the Northwest corner of the Southeast $1 / 4$ of the Southeost $1 / 4$ of Section 3; thence $N 89^{\circ} 37^{\prime} 16^{\prime \prime} \mathrm{E}, 1328.99$ feet along the North line of the Southeast $1 / 4$ of the Southeast $1 / 4$ of Section 3 to the Northeast corner thereof and the Northwest comer of the Southwest $1 / 4$ of the Southwest $1 / 4$ of Section 2, Township 24 South, Range 27 East; thence $\mathrm{N} 00^{\circ} 07^{\prime} 50^{\prime \prime} \mathrm{W}, 1325.78$ feet along the West line of Northwest $1 / 4$, of the Southwest $1 / 4$, of Section 2 to the Northwest corner thereof; thence $N 00^{\circ} 07^{\prime} 43^{n} \mathrm{~W}, 400.13$ feet along the West line of the Northwest $1 / 4$, of Section 2; thence run along the Northerly boundary of a deed recorded in Official Records Book 1457. Page 934 of the Public Records of this County the following three courses; $N 86^{\circ} 46^{\prime} 13^{\prime \prime} \mathrm{E}, 1024.87$ feet; $N 77^{\prime} 37^{\prime} 23^{\prime \prime} \mathrm{E}, 1103.42$ feet; $N$ $53^{\prime} 18^{\prime} 38^{\prime \prime} \mathrm{E}, 1872.82$ feet to a point on the Southerly right-of-way line of Reoms Rood as shown on Plat book 3, Page 85 of the Public Records of this County, thence run along said right-of-way line the following three courses; $S$ CONTINUED ON SHEET 9


| surverng and P.O.B. 10000 LAKE BUENA VSTA FL 32830-1000 PHONE (407)560-7118FAX 407$)^{5660-7689}$ | WLIWG AREA DISNEY OVERALL | \| ${ }_{\text {PATE }} / 2 / 07 / 17$ |
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|  | RCID WATER/WASTE WATER TERRITORY |  |
|  | SKETCEH OF DESCRIPTION | DRAWN BY: <br> JLG |
|  | EXHIBIT A1, SHEET 8 OF 31 SHEETS | $\begin{aligned} & \text { FLDNAME: } \\ & 100 \mathrm{JGO} 9096 \mathrm{R} 2 \\ & \hline \end{aligned}$ |

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$43^{\circ} 40^{\prime} 10^{\prime \prime} \mathrm{E}, 1382.92$ feet to the beginning of a curve concave to the Northeast, having a radius of 546.86 feet and a central angle of $46^{\circ} 21^{\prime} 00^{\prime \prime}$; thence run Southeasterly along the arc of said curve 442.39 feet; thence $N \quad 89^{\circ} 58^{\prime} 50^{\prime \prime} E$, 341.61 feet; thence leaving said right-of-way, run $S 00^{\prime} 19^{\prime} 24^{\prime \prime} \mathrm{E}, 603.75$ feet along the East line of the Northeast $1 / 4$ of Section 2, to the Southeost corner thereof, and the Northwest corner of the Northwest $1 / 4$ of the Southwest $1 / 4$ of Section 1, Township 24 South, Range 27 East; thence $N 89^{\circ} 43^{\prime} 47^{\prime \prime} E_{1}$ along the North line of the Northwest $1 / 4$ of the Southwest $1 / 4$ of Section 1, 1297.19 feet to a point 25 feet West of the Northeast corner of the Northwest $1 / 4$ of the Southwest $1 / 4$ of Section 1 ; thence $N 00^{\circ} 12^{\prime} 21^{\prime \prime} \mathrm{W}, 598.76$ feet along a line that is 25.00 feet West af and parallel to the West line of the Southeast $1 / 4$ of the Northwest $1 / 4$ of Section 1 to the Southerly right-of-way line of aforesaid Reams Raad; thence $N 899^{\circ} 56^{\prime} 46^{\prime \prime}$ E, 100.00 feet along said Southerly right-of-way of Reams Raod; thence run along the Easterly and Northerly boundary of a deed recorded in Official Records Book 1465, Page 307 of the Public Records of this County the following five courses; $S 02^{\circ} 04^{\prime} 12^{\prime \prime} \mathrm{E}, 523.43$ feet; $\mathrm{N} \mathrm{B9}{ }^{\circ} 43^{\prime} 40^{\prime \prime} \mathrm{E}$, 52.00 feet; $S$ $00^{\circ} 12^{\prime} 21^{\prime \prime} \mathrm{E}, 49.00$ feet; $N B 9^{\circ} 43^{\prime} 41^{\prime \prime} \mathrm{E}, 229.00$ feet; $S 0072^{\prime} 25^{\prime \prime} \mathrm{E}, 26.23$ feet; thence $N B 9^{\prime} 43^{\prime} 47^{\prime \prime} E_{1} 1039.16$ feet along the North line of the South half of Section 1 to a point. 90.00 feet East of the Northeast corner of the Southwest $1 / 4$ of Section 1; thence $S 05^{\prime} 34^{\prime} 33^{\prime \prime} \mathrm{W}, 911.86$ feet; thence $S 005^{\prime} 18^{\prime \prime} \mathrm{E}, 420.00$ feet along the East line of the Northeast $1 / 4$ of the Southwest $1 / 4$ of Section 1 to the Southeast corner thereof; thence $N \quad 89^{\circ} 44^{\prime} 10^{\prime \prime} E$, 2649.93 feet along the North line of the South half of the Southeast $1 / 4$ of Section 1 to the Point of Beginning.

Also including the following described parcels:
A parcel of land lying in Sections 27 and 28 , Township 24 South, Range 28 East, Orange County, Flarida, and being more particularly described as follows:

Begin at the West Quarter corner of Section 27, run along the West line of the Northwest $1 / 4$ of Section, $N 00.02^{*} 53^{\prime \prime}$ E, 682.89 feet; thence run along the South line of the Northeast $1 / 4$ of the Southeast $1 / 4$ of the Northeast $1 / 4$ of Section 28, N $899^{\circ} 56^{\prime} 04^{\prime \prime}$ W, 599.53 feet to a point on the Easterly right-of-way line of State Road 400 as shown in map section $75280-2465$ and dated $2 / 22 / 1993$; thence run along said right-of-way line the following five courses; $N$ $38^{\circ} 29^{\circ} 40^{\prime \prime}$ E, 85.01 feet; thence $S 51^{\circ} 29^{\prime} 59^{\prime \prime}$ E, 24.30 feet; thence $N 42^{\circ} 29^{\prime} 47^{\circ} \mathrm{E}, 519.07$ feet to a point of curvature of a curve concave Southeasterly having a radius of 616.02 feet, and a central angle of $37{ }^{\circ} 22^{\prime} 29^{\prime \prime}$; thence run Northedsterly along the arc of said curve, 401.84 feet; thence $N 79^{\circ} 53^{\prime} 24^{\prime \prime} E, 876.12$ feet; thence run alang the westerly boundary of a deed recorded in Official Recorded Book 5128 , Page 3223 of the public Records of this County the follow six courses; $S 10^{\circ} 05^{\prime} 08^{\prime \prime} E, 841.27$ feet to a point on a non-tangent curve concave Northwesterly having a radius of 50.00 feet, and a central angle of $89^{\circ} 59^{\prime} 49^{\prime \prime}$; thence from a tangent bearing of $510^{\circ} 05^{\prime} 20^{\prime \prime} E$ run Southwesterly along the arc of said curve, 78.54 feet; thence $S 794^{\circ} 54^{\prime \prime} \mathrm{W}, 57.02$ feet to a point on a nan-tangent curve concave Southeasterly having a radius of 85.00 feet, and a central angle of $85.16^{\prime} 57^{\prime \prime}$; thence from a tangent bearing of $S 79^{\circ} 54^{\prime} 19^{\prime \prime} \mathrm{W}$ run Southwesterly along the arc of said curve, 126.52 feet; thence $505^{\circ} 22^{\prime} 41^{\prime \prime} \mathrm{E}, 31.47$ feet; thence $N 79^{\circ} 52^{\prime} 20^{\prime \prime} E, 360.78$ feet; thence run along the Westerly right-af-way line of State Road 535 as shown in map section $75560-2610$ and dated $8 / 7 / 1992,510^{\circ} 07^{\prime \prime} 19^{\prime \prime} \mathrm{E}, 100.00$ feet; thence run along the Northerly and Westerly boundary of a deed recorded in Official Recorded Book 4869, Page 2401 of the Public Records of this County the follow five courses; $S 79^{\circ} 52^{\prime} 17^{\prime \prime} \mathrm{W}, 391.52$ feet to a point on a non-tangent curve concave Southerly having a radius of 420.98 feet, and a central angle of $02^{\circ} 26^{\prime} 38^{\prime \prime}$; thence from a tangent bearing of $S 79^{\circ} 53^{\prime} 33^{\prime \prime} W$ run Westerly along the arc of said curve, 17.96 feet; thence $S 123^{\prime} 06^{\prime \prime} \mathrm{E}, 124.13$ feet; thence $N 792^{\circ} 06^{\prime \prime} \mathrm{E}, 52.23$ feet; thence $S 10^{\circ} 07^{\prime} 42^{\prime \prime} \mathrm{E}, 221.02$ feet to a point on the South line of the Northwest $1 / 4$ of Section 27; thence run along said South line $58^{\circ} 42^{\circ} 32^{\prime \prime} \mathrm{W}, 1102.84$ feet to the Point of Beginning.

CONTINUED ON SHEET 10

|  | MAPPING DEPARTMENT P.0.E 10000 LAKE BUENA USTA FL 32830-1000 FAX $40775650-7869$ | FWLWG AREA DISNEY OVERALL | DATE:07/17 |
| :---: | :---: | :---: | :---: |
|  |  | ${ }^{\text {PRo..ECT NAWE }}$ RCID WATER/WASTE WATER TERRITORY |  |
|  |  | SNRVYTPE | ${ }_{\text {dRAWN }}^{\text {dig }}$ |
|  |  | EXHIBIT A1 SHEET 9 OF 31 SHEETS | FILENAME: <br> 101609006R2 |

## CONTINUED FROM SHEET 9

Less the following described parcels:
That portion of Lots 110 and 111 of the Munger and Company Subdivision of Section 22, Township 24 South, Range 28 East according to the Plat recorded in Plat Book E Page 22 of the Public Records of Orange County, Florida, being more particularly described as:

Commence at the Northwest corner of the Southwest $1 / 4$ of the Southwest $1 / 4$ of Section 22 , run $589^{\circ} 27^{\prime \prime} 13^{\prime \prime}$ E, 464.18 feet along the North line of the Southwest $1 / 4$ of the Southwest $1 / 4$ of Section 22 ; thence $500^{\circ} 32^{\prime} 47^{\prime \prime} \mathrm{W}$, 15.00 feet to a point on the North line of soid Lot 111 and the Point of Beginning; thence $S 89^{\prime 2} 27^{\prime} 13^{\prime \prime} \mathrm{E}, 300.00$ feet along the North line of Lots 110 , and 111 to the West right-of-way of State Road 400 as shown in map section 75280-2465 and dated $2 / 22 / 1993$; thence $504^{\circ} 05^{\prime} 32^{\prime \prime} \mathrm{E}, 150.49$ feet along the said right-of-way, thence $N$ $89^{\circ} 27^{\prime} 13^{\prime \prime}$ W, 312.17 feet along the South line of the North 150.00 feet said Lots 110 and 111 ; thence $N \quad 00 \cdot 32^{\prime} 47^{\prime \prime} E$, 150.00 feet to the Point of Beginning.

And

That part of the Northwest $1 / 4$ of the Southeast $1 / 4$ of the Southwest $1 / 4$ and the Northeast $1 / 4$ of the Southwest $1 / 4$ of the Southwest $1 / 4$ of Section 22. Township 24 South. Range 28 East, being more particularly described as:

Commence at the Northwest corner of the Southwest $1 / 4$ of the Southwest $1 / 4$ of Section 22 , run along the North line of the South $1 / 2$ of the Southwest $1 / 4$ of Section 22, $S 897^{\prime \prime} 13^{\prime \prime} E, 985.26$ feet, to the Point of Beginning; thence cantinue along said line $S 9^{\circ} 27^{\prime} 13^{\prime \prime} \mathrm{E}, 642.78$ feet; thence run alang the Westerly right-of-way line af State Road 400 as shown in map section 75280-2465 and dated $2 / 22 / 1993$ the following three courses; $S 46^{\circ} 05^{\prime} 23^{\prime \prime} \mathrm{W}$, 681.12 feet to a point on a non-tangent curve concave Northerly having a radius of 60.00 feet, and a central angle of $118^{\circ} 45^{\prime} 23^{\prime \prime}$; from a tangent bearing of $546^{\circ} 06^{\prime} 36^{\prime \prime} \mathrm{W}$ run Westerly along the arc of said curve, 124.36 feet; $N$ $15^{\circ} 07^{\prime} 40^{\prime \prime}$ W, 205.41 feet; thence run along the West line of Lot 109 of the Munger and Company Subdivision of Section 22, according to the Plat recorded in Plat Book E Page 22 of the Public Records of this County, $N 00^{\circ} 14^{\prime} 30^{\prime \prime} E$, 252.64 feet to the Point of Beginning.

Containing 17,764.366 acres more or less.

|  | SURVEYNG AND MAPPING DEPARTMENT P.O.B. 10000 <br> LAKE BUENA VSTA <br> FL 32830-1000 <br> PHONE (407)560-7118 <br> FAX (407)560-7869 | FLLIG AREAWDW DISNEY OVERALLPRORETN NMERCID WTER/WASTE WATER TERRITORY |  |  |  |  |  | $\begin{aligned} & \hline \text { DATE } \\ & 12 / 07 / 17 \\ & \hline \text { SCALE } \end{aligned}$ |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |
|  |  | SWRVETME |  |  |  |  |  |  | DRAWN BY: JLG |
|  |  | COMMENTS <br> EXHIBIT A1, SHEET 10 OF 31 SHEETS |  |  |  |  |  |  | FLENAME: $10 \mathrm{JGO9096R2}$ |





















# INTERLOCAL AGREEMENT BETWEEN <br> REEDY CREEK IMPROVEMENT DISTRICT AND <br> ORANGE COUNTY <br> FOR DELIVERY OF WHOLESALE WATER SERVICES TO THE FLAMINGO CROSSINGS DEVELOPMENT 

THIS INTERLOCAL AGREEMENT (the "Agreement") is made and entered into on the date of later execution below, by and between the REEDY CREEK IMPROVEMENT DISTRICT, a public corporation and public body corporate and politic of the State of Florida, whose address is P.O. Box 10170, Lake Buena Vista, Florida 32830 ("Supplier"), and ORANGE COUNTY, a charter county and political subdivision of the State of Florida (the "County"), whose address is 201 South Rosalind Avenue, Orlando, Florida 32801 . Hereinafter, Supplier and the County may be referred to individually as a "Party" or collectively as the "Parties."

## WITNESSETH:

WHEREAS, the Florida Interlocal Cooperation Act of 1969, Section 163.01, Florida Statutes, permits local governments to make the most efficient use of their powers by enabling them to cooperate with other localities on a basis of mutual advantage and thereby provide services and facilities in a manner that will accord best with the needs and development of local communities; and

WHEREAS, Supplier and the County are retail providers of water, wastewater and reclaimed water services (collectively, "Water Services") in their respective service areas; and

WHEREAS, the County is currently the retail provider of Water Services to property hereinafter referred to "FC Ultimate," a map of which is attached hereto and incorporated herein as Exhibit "A"; and

WHEREAS, contemporaneously with the execution of this Agreement, Supplier and the County intend to amend that certain Reedy Creek Improvement District/Orange County Amended and Restated Water, Wastewater, and Reclaimed Water Service Territorial Agreement, dated September 30, 2008 (the "Territorial Agreement"), by which it is recognized that parcels FC-1 and FC-2, which are areas inside of FC Ultimate, have been removed from the water, wastewater, and reclaimed water territory of the Reedy Creek Improvement District (the "RCID's Territorial Area") and are now within the territorial jurisdiction of the County (the "Adjacent Territorial Area"); and

WHEREAS, the County has agreed to remedy any hydraulic constraints in its water system that would constrain its ability to meet the FC Ultimate water demands within ten years of the Effective Date, as that term is defined below in this Agreement; and

WHEREAS, Supplier acknowledges that the developers of FC Ultimate must construct or cause to be constructed, at their sole cost, and dedicate to the County water, wastewater, and reclaimed water transmission, collection, and distribution lines, and related appurtenances, as required to serve FC Ultimate to a point of connection as hereinafter set forth in this Agreement. No later than the execution of this Agreement, Supplier and the County intend to execute an Access and Utility Easement over Supplier's right-of-way within the FC Ultimate and between the FC Ultimate and CR 545, up to and including the County System Point of Connection, as defined in Paragraph 3.b; and

WHEREAS, while FC Ultimate will be developed by third parties, the Master Utility Plan has been approved by the County through their development review process (the "County Approved MUP"), which is attached hereto and incorporated herein as Exhibit "B;" and

WHEREAS, pursuant to the Territorial Agreement, all customers within the FC Ultimate shall be customers of the County and subject to, inter alia, connection fees, capital charges, and rates for all Water Services as established by the County from time to time; and

WHEREAS, Supplier desires to provide wholesale Water Services to the County, and the County desires to receive wholesale Water Services from Supplier under terms and conditions set forth in this Agreement; and

WHEREAS, the Parties entered into a letter agreement entitled "Amendment to Substitute Letter Agreement for Orange Lake/Reams Road Wastewater Interconnection and Wholesale Service" (the "Wastewater Letter Agreement"), which has an effective date of January 24, 2018, in accordance with the Territorial Agreement, to govern the provision of certain wholesale wastewater service by Supplier to the County; and

WHEREAS, the Parties entered into a letter agreement entitled "Amendment to 2012 Flamingo Crossings Letter Agreement for Water and Reclaimed Water Interconnection and Wholesale Service" (the "Water Letter Agreement") which has an effective date of January 24, 2018, in accordance with the Territorial Agreement, to govern the provision of certain wholesale water and reclaimed water services by Supplier to the County; and

WHEREAS, the intent of this Agreement is not to amend, modify, or in any way affect any terms or conditions or the Territorial Agreement, the Water Letter Agreement, or the Wastewater Letter Agreement; and

WHEREAS, Supplier and the County hereby determine this Agreement to be in the public interest.

NOW THEREFORE, in consideration of the commitment of Supplier to provide wholesale Water Services to the County, and the commitment of the County to accept these Water Services, and other good and valuable consideration, the receipt and sufficiency of which are hereby acknowledged, the Parties agree to the following terms and conditions.

## 1. , PREMISES

Each and all of the foregoing recitals are agreed to and form a material part of this Agreement.

## 2. TERM OF THE AGREEMENT; EXPIRATION; TERMINATION OF SERVICE

The term of this Agreement shall commence on the date it is fully executed by both Parties (the "Effective Date"). This Agreement shall expire on the last date that Supplier provides wholesale. Water Services, and the County initiates water, wastewater, and reclaimed water services to the FC Ultimate directly from its utility systems (the "Term"). Notwithstanding the foregoing, if the County experiences a hydraulic constraint in its water system due, in whole or in part, to the delivery of water to the FC Ultimate, then the Term shall be extended to the date the County eliminates the hydraulic constraint as provided in paragraph 3(d) below. Supplier's provision of wholesale water, wastewater, or reclaimed water services may be individually terminated prior to the expiration of this Agreement pursuant to Sections 3, 4, and 5 below.

## 3. PROVISION OF WHOLESALE WATER SERVICE

(a) The County agrees to complete construction and place in service the County's Malcolm Road Water Supply Facility (the "MRWSF") and the County's Avalon

Road Water Storage and Repump Facility (the "WSRF") on or before the eighth anniversary of the Effective Date.
(b) Supplier shall provide wholesale water service to the County, based on the County Approved MUP attached in Exhibit "B," for its retail customers within the FC Ultimate on a temporary basis (1) until the date of construction substantial completion and the placing into service of the MRWSF and the WSRF, County infrastructure is available at the intersection of CR 545 and Western Way (the "County System Point of Connection"), and the FC Ultimate water service is connected, at no cost to the County, to the County system as shown on Exhibit "C;" or (2) until the date of construction substantial completion of the MRWSF and WSRF, and the FC Ultimate water service is connected to the County system beyond the County System Point of Connection, at no cost to the County, at the intersection of CR 545 and Western Way.
(c) At any time after the MRWSF and WSRF have reached substantial completion and have been placed into service or after eight years from the Effective Date, whichever comes first, if Supplier requires all or a portion of the volume of water that is wholesaled to the County to provide water service to the FC Ultimate for use within Supplier's water service area, then Supplier may construct a new water interconnect with the County's water system at Reams Road or near the intersection of Buena Vista Drive and CR 535 at the Supplier's sole cost. Such interconnect shall be used to deliver water from the County's water system to Supplier's water system in an amount equivalent to that being delivered to the FC Ultimate by Supplier. A separate agreement will be required to govern the use of the interconnect and water deliveries described in this paragraph.
(d) If after the County initiates water service to the FC Ultimate and the County determines that it needs additional water to address a hydraulic constraint due to peak hourly flows or fire flow demand in its water system due to the County's delivery of water to the FC Ultimate, then at the County's request Supplier shall provide wholesale water service to the County for use by the FC Ultimate for peak hourly flow and/or fire flow until such time as the County eliminates the hydraulic constraint. The County shall eliminate the hydraulic constraint on or before the tenth anniversary of the Effective Date.
(e) Upon fulfillment of the above conditions within Section 3, wholesale water service will be terminated and the County shall provide water service from its facilities to the FC Ultimate, based on the County Approved MUP, at the County's sole cost.
(f) The Parties agree that the quantity of water that flows to the County for the FC Ultimate shall not be included in the quantities identified in the Water Letter Agreement, but rather shall be in addition thereto.

## 4. PROVISION OF WHOLESALE WASTEWATER SERVICE

(a) The County agrees to complete construction and place in service the County's new Southwest Water Reclamation Facility (the "SWWRF") and the County's Avalon Road Master Wastewater Pump Station (the "MPS") on or before the tenth anniversary of the Effective Date.
(b) Supplier shall provide wholesale wastewater service to the County, based on the County Approved MUP attached in Exhibit "B," for its retail customers in the FC Ultimate on a temporary basis (1) until the date of construction substantial completion and the placing into service of the SWWRF and the MPS, County infrastructure is available at the intersection of CR 545 and Western Way, and the FC Ultimate wastewater service is connected, at no cost to the County, to the County system as shown on Exhibit "C;" or (2) until the SWWRF and the MPS are placed into service, and the FC Ultimate wastewater services are connected to the County system beyond the County System Point of Connection, at no cost to the County, at the intersection of CR 545 and Western Way.
(c) Upon fulfillment of the above conditions within Section 4, wholesale wastewater service will be terminated and the County shall provide wastewater service from its facilities to the FC Ultimate, based on the County Approved MUP, at the County's sole cost.
(d) Parties agree that the quantity of wastewater that flows from the County from the FC Ultimate shall not be included in the quantities identified in the Wastewater Letter Agreement, but rather shall be in addition thereto.

## 5. PROVISION OF WHOLESALE RECLAIMED WATER SERVICE

(a) The County agrees to complete construction and place in service the County's Avalon Road Reclaimed Water Storage and Repump Facility (the "RWSRF") on or before the tenth anniversary of the Effective Date.
(b) Supplier shall provide wholesale reclaimed water service to the County, based on the County Approved MUP attached in Exhibit "B," for its retail customers in the FC Ultimate on a temporary basis (1) until the date of construction substantial completion and the placing into service of the RWSRF, County infrastructure that is connected to the RWSRF, as shown in Exhibit "C," is available at the intersection of CR 545 and Western Way, and the FC Ultimate reclaimed water service is connected to the County system at no cost to the County; or (2) until the RWSRF is placed into service, and at no cost to the County, the FC Ultimate reclaimed water service is connected to the County system beyond the intersection of CR 545 and Western Way, including a connection to the RWSRF.
(c) Upon fulfillment of the above conditions within Section 5, wholesale reclaimed water service will be terminated and the County shall provide reclaimed water service from its facilities to the FC Ultimate, based on the County Approved MUP, at the County's sole cost.
(d) The Parties agree that the quantity of reclaimed water that flows to the County for the FC Ultimate shall not be included in the quantities identified in the Water Letter Agreement, but rather shall be in addition thereto.

## 6. RATE, PAYMENT, AND BILLING

(a) Throughout the term of this Agreement, Supplier shall provide the Water Services to the County at the rates provided herein.

## Water Rate

Supplier will charge and the County will pay wholesale rate equivalent to $96.2 \%$ of Supplier's Potable Water GS-1 retail rate (currently $\$ 1.02$ per thousand gallons equating to a wholesale rate of $\$ 0.98$ per thousand gallons). Whenever Supplier raises its retail Potable Water GS-1 rate, the wholesale rate applied to the County shall likewise increase proportionately.

## Reclaimed Water Rate

Supplier will charge and the County will pay wholesale rate equivalent to $70.4 \%$ of Supplier's Reclaimed Water GS-1 retail rate (currently $\$ 0.84$ per thousand gallons equating to a wholesale rate of $\$ 0.59$ per thousand gallons). Whenever Supplier raises its retail Reclaimed Water GS-1 rate, the wholesale rate applied to the County shall likewise increase proportionately.

## Wastewater Rate

Supplier will charge and the County will pay a wholesale rate equivalent to $47 \%$ of Supplier's Wastewater SC-1 retail rate (currently $\$ 5.62$ per thousand gallons of wastewater flow equating to a wholesale rate of $\$ 2.64$ per thousand gallons). Whenever Supplier raises its retail SC-1 rate, the wholesale rate applied to the County shall likewise increase proportionately.
(b) Notwithstanding the foregoing, at no time shall the Water Services rates Supplier charges the County hereunder exceed the lowest respective water, wastewater, and reclaimed water services rates Supplier charges any of its wholesale customers.
(c) Payment for Water Services delivered by Supplier to the County under this Agreement must be made to the Supplier's Authorized Representative at the address set
forth below. Supplier reserves the right to notify the County of a change in the Authorized Representative or its address by providing County a minimum of ten days advance notice of such change. The Authorized Representative is:

Reedy Creek Improvement District<br>Utility Division<br>Attn: Payables Clerk<br>P.O. Box 30000<br>Orlando, Florida 32891-8132

(d) Bills for water and reclaimed water service shall be calculated based on the respective monthly meter readings at the Wholesale Points of Connection as described in Section 8.
(e) The wastewater bill shall be based on the monthly water volume use as measured by the water meter at the Wholesale Point of Connection, multiplied by a factor of 0.818 ( 225 gpd per ERU divided by 275 gpd per ERC), where an Equivalent Residential Unit (ERU) for wastewater is equal to 225 gpd and an Equivalent Residential Connection (ERC) for water is equal to 275 gallons per day (gpd). Water used during construction of the new development within FC Ultimate may be subtracted from the wastewater bill upon request by the County provided that the water use is metered.
(f) Supplier shall bill the County on a monthly basis for Water Services. The County agrees to make payments to the Supplier within forty-five days from the date it receives such bill from the Supplier.

## 7. WATER USES

The County's use of the Water Services shall be limited to service only to those customers located within the FC Ultimate area as described in Exhibit "A."

## 8. WHOLESALE POINTS OF CONNECTION

(a) At no cost to the County, Supplier shall ensure that its Water Services systems are connected to the County's utility system, with flow meters and all appurtenances thereto, at the Wholesale Points of Connection depicted in Exhibit "D," such that the volume of Water Services delivered hereunder can be accurately measured. Exhibit "D" is attached hereto and incorporated herein by reference. All connections shall meet the County standards and are subject to approval by the County, which approval shall not be unreasonably delayed, conditioned, or withheld.
(b) Meter assemblies shall be constructed on or before the second anniversary of the Effective Date. After the Effective Date, and prior to the construction of the meter
assemblies, the wholesale utility bills shall be based on the monthly water and reclaimed water volume use as measured at the retail meters.
(c) Meter assemblies to be installed at the water and reclaimed water Wholesale Points of Connection shall be as shown in Exhibit "E." The division of ownership shall be as shown in Exhibit "D." Exhibit "E" is attached hereto and incorporated herein by reference.
(d) At no cost to the County, Supplier shall operate, maintain, and modify as necessary, its distribution, collection, and transmission systems on Supplier's side of the Wholesale Points of Connection to the County's utility system to the extent necessary to ensure delivery of Water Services to the FC Ultimate.
(e) At the time when Water Services end per Sections 3, 4, and 5 of this Agreement, the Parties agree to coordinate the facilitation of the transition of each utility Water Service from Suppliers' system to the County's utility system as soon as reasonably practical.

## 9. WATER QUALITY, QUANTITY

(a) Throughout the term of this Agreement, Supplier shall:
(1) deliver to the County water in volumes necessary to meet the needs of all County customers within FC Ultimate until permanent connection at the County System Point of Connection is made, unless previously terminated prior to the expiration of the term of the Agreement.
(2) deliver to the County reclaimed water in volumes necessary to meet the needs of all County customers within FC Ultimate until permanent connection at the County System Point of Connection is made, unless previously terminated prior to the expiration of the term of the Agreement.
(3) accept from the County wastewater in volumes necessary to meet the needs of all County customers within FC Ultimate until permanent connection at the County System Point of Connection is made, unless previously terminated prior to the expiration of the term of the Agreement.
(b) All water delivered by Supplier shall be of a quality consistent with the drinking water standards of the FDEP, EPA, and all other applicable laws and regulations.
(c) All reclaimed water delivered by Supplier shall be of a quality consistent with the requirements for "public access" treatment levels as described in rules of the FDEP, Chapters 62-600 through 62-650, Florida Administrative Code (FAC), and all other applicable laws and regulations.
(d) The County represents that it will not authorize wastewater from industrial users, as defined within Chapter 62-625, FAC, to be transmitted to Supplier's system.
(e) In the event an industrial user is allowed within the area serviced by this Agreement absent a prior modification to this Agreement as contemplated above, it shall be deemed a breach of this Agreement by the responsible Party.

## 10. METERING

Supplier shall install and properly calibrate metering equipment at all water and reclaimed water Wholesale Points of Connection. Such equipment shall remain the property of Supplier, who shall be responsible for its operation, maintenance, calibration and replacement throughout the term of this Agreement. Supplier shall read the meters for billing purposes. The metering equipment shall be of standard make and type and shall meet the standards of the American Water Works Association ("AWWA") for accuracy. With the County present, Supplier shall test the metering equipment for accuracy without charge to the County once during any twelve month period. Supplier shall perform such additional testing as may be requested by the County, with the County present, at a charge to the County not to exceed Supplier's actual cost for such tests. Supplier shall provide the County with copies of the test results within thirty days of each test. Notwithstanding the foregoing, Supplier will not charge the County for tests that discover an inaccurate meter, as defined by AWWA. If an inaccurate meter is discovered, Supplier shall make bill adjustments for up to twelve months preceding the test. Bill adjustments will be accounted for in the next billing cycle and a separate bill will not be generated.

## 11. COST OF CAPACITY

The applicant for utility connections for all parcels within FC Ultimate shall pay all applicable capital charges in accordance with Chapter 37 of the Orange County Code.

## 12. DEVELOPMENT APPROVALS

All development that occurs within FC Ultimate shall be subject to the requirements and approvals of the County's development process and applicable County ordinances and regulations, and shall be in accordance with the County Approved MUP. The County is not obligated to provide Water Service under this Agreement to any property within FC Ultimate until all applicable regulations are satisfied, including payment of capital charges. Supplier shall not enter into any contracts or approve any development or uses within the Flamingo Crossing Development that will conflict with the County's development processes. No construction of utility infrastructure shall be undertaken at the cost of the County. This agreement does not create a specific duty for either Supplier or the County to pay for infrastructure to support the demands of FC Ultimate. Developers of parcels or structures within the FC Ultimate shall be solely
responsible for the cost of infrastructure needed to provide Water Services to those projects. This Agreement does not preclude Supplier from participating or acting as or on behalf of a developer. The County has no duty to provide Water Services within the FC Ultimate in addition to the Water Services set forth in this Agreement.

## 13. CONSTRUCTION AND CONVEYANCE OF FUTURE INFRASTRUCTURE

All infrastructure within the FC Ultimate that provides Water Service to FC Ultimate shall be constructed in accordance with the County's most recent version of its Standards and Specification Manual. Developers of the FC Ultimate properties shall be responsible for acquiring all required governmental permits and approvals for the construction activities. All development in unincorporated Orange County shall be in accordance with the Orange County Standards within the standard review process and timing. The County shall have the right to make periodic inspections during the construction of the mains.

## 14. EASEMENTS

At no cost to the County, Supplier shall grant the County an Access and Utility Easement over Supplier's right-of-way for existing and future Utility System infrastructure needed to serve the FC Ultimate. This conveyance will be executed on or before the Effective Date and subject to the conditions set forth in Section 12 herein.

## 15. FORCE MAJEURE

(a) As used in this Agreement, an event of "Force Majeure" shall mean an unforeseeable act or event that prevents or delays or otherwise adversely affects a Party's performance of its obligations under this Agreement or compliance with any conditions required by the other Party under this Agreement if such act or event is beyond the reasonable control of and not the fault of the affected Party, including acts of God (e.g. flood, lightning, tornado, hurricane, sinkhole), acts of public enemy, and compliance with an order of governmental authority. In no event shall either Party be excused from payment obligations under this Agreement by reason of Force Majeure.
(b) If either Party is rendered wholly or partly unable to perform its obligations under this Agreement because of a Force Majeure event, that Party will be excused from whatever performance is affected by the Force Majeure event to the extent so affected, provided that: (1) the non-performing party, within forty-eight hours after knowing of the occurrence of the Force Majeure event, gives the other Party written notice describing the particulars of the occurrence; (2) the suspension of performance is of no greater scope and of no longer duration than is reasonably required by the Force Majeure event; and the nonperforming party uses reasonable efforts to overcome or mitigate the effects of such an occurrence.

## 16. ASSIGNMENT

Neither Party shall assign this Agreement to an entity other than a public agency, as that term is defined in Section 163.01(3)(b), Florida Statutes. Furthermore, neither Party shall assign this Agreement without the express written consent of the other, which shall not be unreasonably delayed, conditioned, or withheld.

## 17. COMPLIANCE WITH LAWS AND REGULATIONS

The Parties shall comply with all applicable federal, state, and local laws and regulations relating to the performance of the obligations set forth in this Agreement.

## 18. REASONABLE APPROVALS

In those instances in this Agreement in which a Party's approval, consent or satisfaction is required and a time period is not specified, then it shall be implied that such action shall be exercised in a reasonable manner and within a reasonable time frame relative to the nature of work or act in progress.

## 19. DEFAULT AND REMEDIES

(a) Failure by a party to perform any of its obligations hereunder shall constitute a default hereunder, entitling the non-defaulting party to pursue the remedies of specific performance, injunctive relief, or damages. Prior to either Party filing any action as a result of a default by the other Party under this Agreement, the non-defaulting Party exercising such right shall first provide the defaulting party with written notice specifying such default and the actions needed to cure same, in reasonable detail. Upon receipt of said notice, the defaulting Party shall be provided a thirty day opportunity within which to cure such default, unless such default is not capable of being cured within thirty days, in which case that Party must cure the default as soon as practicable. Failure to cure within the appropriate cure period, the non-defaulting Party may seek specific performance arising from such default.
(b) Notwithstanding any other provision of this Agreement, in no event shall either Party have liability to the other Party under this Agreement, whether based in contract, in tort, or otherwise, for (a) any special, incidental, indirect, exemplary or consequential damages, (b) damages with respect to cost of capital, loss of use of plant or plant capacity or equipment or claims of customers of either Party, as the case may be, if such damages are categorized as special, incidental, indirect, exemplary, or consequential, or (c) costs, loses, damages, fines or penalties to the extent that either Party is entitled to receive insurance proceeds pursuant to an insurance policy or policies covering such costs, loses, damages, expenses, fines or penalties.
(c) The Parties shall be responsible for their individual attorney's fees, costs, and expenses in any litigation, suit, dispute, controversy, mediation, or proceeding, including appellate proceedings, arising out of, based on, or related to, this Agreement.
(d) This Section of the Agreement shall survive termination/expiration of the Agreement.

## 20. NOTICES

(a) All notices required or authorized under this Agreement shall be given in writing and shall be served by mail on the parties at the addresses listed below:

| Supplier: | District Administrator <br> Reedy Creek Improvement District <br> Post Office Box 10170 <br> Lake Buena Vista, Florida 32830 |
| :--- | :--- |
|  | Director <br> Reedy Creek Energy Services <br> 5300 Center Drive <br> Lake Buena Vista, FL 32830 |
| The County: | Director <br> Orange County Utilities <br> 9150 Curry Ford Road <br> Orlando, Florida 32825-7600 |
| With a copy to: | County Administrator <br> Orange County Administrator's Office |
|  | 201 S. Rosalind Avenue, 5 |
|  | Orlando, Florida 32801-3527 |

(b) Either Party may notify the others in writing of a change of address for Notices under this section, at least ten days prior to the effective date of the address change.

## 21. RELATIONSHIP OF THE PARTIES

The Parties do not intend to create hereby any joint venture, partnership, association, or other entity for the conduct of any business for profit. The Parties deem Supplier and the County to be independent contractors for the purposes of this Agreement, and not as agents or partners of the other.

## 22. AMENDMENTS

Any and all modifications to the provisions of this Agreement shall be made by mutual agreement of the Parties, in writing, and executed by the Parties.

## 23. DISCLAIMER OF THIRD PARTY BENEFICIARIES

This Agreement is solely for the benefit of the formal parties hereto and no right or cause of action shall accrue upon or by reason hereof, to or for the benefit of any third party not a formal party hereto.

## 24. SEVERABILITY

If any part of this Agreement is found invalid or unenforceable by any court, such invalidity or unenforceability shall not affect the other parts of this Agreement if the rights and obligations of the Parties contained therein are not materially prejudiced and if the intentions of the Parties can continue to be effectuated. To that end, this Agreement is declared severable.

## 25. NON-WAIVER

The failure of either Party to insist upon the other Party's compliance with its obligations under this Agreement in any one or more instances shall not operate to release such other Party from its duties to comply with such obligations in all other instances.

## 26. SOVEREIGN IMMUNITY

Nothing in this Agreement shall be deemed a waiver of sovereign immunity or limits of liability of either Party, including their respective commissioners, supervisors, officers, agents or employees, beyond the statutory limited waiver of immunity set forth in Section 768.28, Florida Statutes (2017), or other statute.

## 27. INDEMNIFICATION AND INSURANCE

(a) Each Party to this Agreement shall be responsible for all personal liability and property damage attributable to the negligent acts or omissions of that Party and its officials, agents, and employees, or arising out of or resulting from that Party's negligent performance under this Agreement (the "Negligent Party"). The Negligent Party agrees to defend, indernnify and hold harmless the other Party, its officials, agents, and employees from all claims, actions, losses, suits, judgments, fines, liabilities, costs and expenses in connection therewith, to the extent permitted by law.
(b) The contractor(s), subcontractor(s), consultant(s), and subconsultant(s) shall provide evidence of the hold harmless and indemnity prior to commencement of work and access to any of the property of the Parties.

## 28. APPLICABLE LAW

This Agreement is an Interlocal Agreement as provided in Section 163.01, Florida Statutes (2017). This Agreement and the provisions contained herein shall be construed, controlled, and interpreted according to the laws of the State of Florida. When required by law, the County agrees to join in any application for a required license, permit or other regulatory approval process necessary or appropriate for the operation of the Water Services that is the subject matter of this Agreement. Any litigation arising out of this Agreement shall be had in the federal or state courts located and lying within Orlando, Orange County, Florida. The Parties waive their respective rights to a jury trial.

## 29. RECORDING

This Agreement, including the Exhibits thereto, shall be recorded in the Public Records of Orange County, Florida. Supplier shall bear the costs and responsibility of such recording.

## 30. ENTIRE AGREEMENT

This Agreement constitutes the entire agreement and understanding between the Parties and shall supersede and replace any and all prior or contemporaneous representations, negotiations, statements, understandings, or agreements between the Parties, whether verbal or written, relating to the matters set forth herein and the execution of this Agreement and is merged into this Agreement. The Parties fully understand the terms and conditions of this Agreement, have entered into this Agreement voluntarily, and have received or had the opportunity to receive independent advice and legal counsel. This Agreement has been executed by the authorized representative of each Party on the date written below each signature.

## 31. TIME OF THE ESSENCE

Time is of the essence in implementing the terms of this Agreement.

## 32. HEADINGS; CONSTRUCTION OF AGREEMENT

The various section headings used in this Agreement are for convenience of reference only and are not to be used to construe, apply or enforce its substantive provisions. The Parties have participated jointly in the negotiation and drafting of this Agreement. In the event ambiguity or interpretation arises, this Agreement shall be construed as if drafted jointly by the Parties and no presumption or burden of proof shall
arise favoring any Party by virtue of the authorship of any of the provisions of this Agreement.

## [SIGNATURES FOLLOW ON NEXT TWO PAGES]

AGREED TO AND EFFECTIVE ON THE DATE on which the later of the Parties to this Agreement executes it.
"SUPPLIER"
REEDY CREEK IMPROVEMENT DISTRICT
By: Board of Supervisors

By:

[ORANGE COUNTY'S SIGNATURES ON NEXT PAGE]

"COUNTY"
ORANGE COUNTY, FLORIDA
By: Board of County Commissioners

ATTEST: Phil Diamond, CPA, Orange County Comptroller as Clerk to the Board of County Commissioners



# Exhibit "B" 

## to

Interlocal Agreement between Reedy Creek Improvement District and Orange County for Delivery of Wholesale Water Services to the Flamingo Crossings Development

## COUNTY APPROVED MUP

# Walt Disney World West District 

# Water, Wastewater, and Reclaimed Water Master Utility Plan 

October 2018

Submitted to:<br>Orange County Utilities (OCU)

Prepared for:


and
Reedy Creek Energy Services (RCES)
Prepared by:

## ^TKINS

Member of the SNC-Lavalin Group

482 South Keller Road
Orlando, Florida 32810

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## 1. Overview

### 1.1. Overview

Walt Disney Imagineering (WDI) and Reedy Creek Energy Services (RCES) authorized Atkins North America, Inc. (Atkins) to develop a master utility plan (MUP) for the existing and planned developments in the Walt Disney World (WDW) West District. The master plan includes potable water, wastewater, and reclaimed water utilities.

The WDW West District project consists of approximately 309 acres located west of State Road 429 and east of County Road 545 (Figure 1-1). The projects limits are within Sections 19, 20, 21, and 28, Township 24, Range 27, in Orange County, Florida. The District is made up of four distinct parcels:

- Existing Flamingo Crossings (FC)
- Proposed Flamingo Crossings Planned Development Phase 1 - East Parcel (FC-1)
- Proposed Flamingo Crossings Planned Development Phase 2 - West Parcel (FC-2)
- Proposed Bear Island ( $\mathrm{BI}-\mathrm{N}$ and $\mathrm{BI}-\mathrm{S}$ )

The Bl parcel will not come online within the immediate future, however, the ultimate (long-term) scenario should accommodate for this development. Therefore, Bl flows and demands have been included in the long-term scenario for adequate infrastructure sizing.

Portions of the proposed project may be de-annexed from the Reedy Creek Improvement District (RCID) into Orange County. Each section listed in this MUP outlines which municipality will own each section of the system and the criteria that were used in the design. The information provided in this report for Orange County Utilities (OCU) includes infrastructure that will be transferred to OCU. OCU's review is limited to the OCU side of the interconnects. RCID demands and models for utility infrastructure will not be reviewed by OCU. All infrastructure part of this project were developed and reviewed by RCES, WDW and RCID.

This MUP involves design of potable water, wastewater, reclaimed water systems, and utility extensions to support the WDW West District short-term and long-term buildout. Applicable utility design criteria are listed in each appropriate section. The utiity points of connection, sizes, pressures, used were all obtained from RCES and OCU and are included in Appendix A. A schematic pipe layout is given for each proposed utility, including color-coded maps depicting the proposed ownership and pipe sizes. Flows were derived using the flow development program for the existing FC Parcel and pending units included in the proposed development plans for the FC-1, FC-2 and BI Parcels. These flows were calculated and distributed through each proposed parcel. RCID and OCU allow the use of ductile iron pipe or PVC pipe, with the exception of OCU wastewater collection systems where ductile iron pipe is not to be used for underground piping.

The proposed OCU mains to be constructed as dry lines and the existing RCID dry lines to be transferred to OCU are to remain privately owned and maintained until dedicated to OCU with future development. The mains shall be tested and inspected per the OCU Standards and Construction Specifications Manual at time of installation as well as at the time of dedication to OCU. Testing immediately prior to dedication shall be a separate project. A Bill of Sale and Maintenance Guarantee to OCU covering the installed mains for a period of one year following the final certificate of completion are requirements of the project clearing the system for use.

An approved land use plan has been provided in Appendix $J$, the plan has been approved by the BCC on $2 / 6 / 2018$ and the DRC.


## 2. Potable Water System

### 2.1. Introduction

The WDW West District is located along Western Way, east of County Road 545 and west of State Road 429. For the purpose of this MUP, the District will be served by RCID from the east (short-term scenario). Once OCU infrastructure is available along County Road 545, the District will be served by OCU from the west (long-term scenario). Parcels FC-1, FC-2, BI-N, and BI-S will be OCU customers and parcel FC will remain a RCID customer The section below outlines which municipality will own what parts of the potable water system and the criteria that were used in the MUP design. Figure 2-1 presents the potable water system infrastructure by owner required for the short-term scenario, while Figure 2-2 presents the potable water system infrastructure by owner required for the long-term scenario.

There are currently four existing RCID potable water main systems near the WDW West District project area. a 16 -inch water main running along Western Way ending at Flamingo Crossings Blvd.; a 12 -inch water main west of Flamingo Crossings Blvd. that ends at the western edge of the FC parcel; a 16 -inch water main south of Western Way that runs along Flamingo Crossings Blvd.; and a 12 -inch water main system on the FC parcel. The existing potable customers are connected to the RCID water distribution system

A hydraulic model was developed using Innovyze InfoWater Scenarios were created to analyze the short-term scenario in which potable water is served by RCID and for the long-term scenario in which the potable water is served by both RCID and OCU. The RCID tie-in pressure is 85 pounds per square inch (psi) at an elevation of 108 feet (provided by RCES) and the OCU tie-in pressure available along County Road 545 was estimated to be 56 psı at an elevation of 120 feet (see Appendix A for OCU tie-in pressure). The proposed pipes were sized based on OCU design criteria to provide a minimum desired residual pressure at the points of delivery. The sizing of the mains meet RCID standards in addition to OCU standards. Topographic elevations were imported into the model through available shapefiles and utilized in the hydraulic calculations.

The short-term scenario consists of development parcels west of State Road 429 FC, FC-1, FC-2, and Orange Lake parcels In this scenario, all parcels will be distributed potable water by RCID A 16 -inch water main is proposed beginning downstream of the potable water interconnect aiong Flamingo Crossings Boulevard, continuing across Flamingo Crossings Blvd., and a 24 -inch then continuing west to the County Road 545 connection point servicing the FC-2 parcel in the short-term scenario as depicted in Figure 2-1. The western boundary of this water main will be valved closed during the shor-term scenario after the future FC-2 parcel connection. RCID and OCU mains will be connected near Western Way and a wholesale water meter and interconnect will be installed The proposed 16 -inch and 24 -inch main will tie into the existıng 16 -inch RCID main and a gate valve is included at this location separating ownership, which will be open during the short term scenario. The existing 16 -inch water main will be disconnected just north of this connection. The potable water system will have three valves: 1.) Western Way and County Road 545. 2.) Western Way and FC-2 parcel; and 3.) on the 16 -inch RCID water main at the tie-in location to the existing 16 -inch water main (see Figure 2-1).

The long-term scenario will include Parcels FC, FC-1, FC-2, Orange Lake, BI-N. and BI-S. In this scenario, when OCU infrastructure becomes available, the gate valves located at Western Way and County Road 545, and FC-2 will be opened and the 24 -inch proposed water main will tie-in to the OCU water main along County Road 545. The gate valve located on the RCID 16-inch water main
(location of ownership separation) will be closed. Under this scenario, OCU will serve all parcels FC$1, \mathrm{FC}-2, \mathrm{BI}-\mathrm{N}$, and $\mathrm{BI}-\mathrm{S}$; while RCID will serve FC .

An interim construction scenario was also analyzed. The scenario is the subset of the short-term scenario with the intent to demonstrate the ability of the existing system and proposed temporary piping serving the FC-2 parcel to provide fire flow during constriction prior to other improvements. The system would be supplied by RCID and metering would be accomplished via local construction or hydrant meter assemblies. The intent is to allow site and vertical construction during the interim construction period


Figure 2-1 Potable Water System Infrastructure Short-Term Scenario

## Legend

## Potubla Water Valvas

- Gate Vaver (Chasea)
-     - Gole vaver (cran)
- Interconnect Ascembly/Meiers (Opern)
rcio-Owned Booster Pump Location
- Patablo Proposed (RCID)
- Potable Exiating (RCIO tranater to OCU)
- Potatho Propased (OCU)
- Pocablo Existing (RCID)

OCU Ubility Service Arsa
ocu proposed Uitily Service Amer


1 inch $=926$ feet
^TKINS


### 2.2. Design Criteria

All pipes owned and maintained by RCID as shown in Figure 2-1 have been designed to their standards. Table 2-1 outlines the applicable RCID design criteria.

Table 2-1 RCID Potable Water System Design Criteria

| Item | Recommended Criteria |
| :--- | :--- |
| Max day peaking factor | 1.8 |
| Peak hour peaking factor | 3.0 |
| Maximum velocity | 8.0 feet per second (fps) |
| Hazen-Williams coefficient, ductile iron pipe | 120 |
| Hazen-Williams coefficient, PVC pipe | 130 |
| Minimum fire flow residual pressure | 20 psi |
| Commercial fire flow requirement | 2,000 gallons per minute (gpm) |

All other public mains within the short-term and long-term scenarios are to be owned and maintained by OCU and designed per the OCU Standards and Construction Specifications Manual (with revisions issued in 2014) as outlined in Table 2-2.

Table 2-2 OCU Potable Water System Design Criteria

| Item | Recommended Criteria |
| :--- | :--- |
| Max day peaking factor | 2.0 |
| Peak hour peaking factor | 4.0 |
| Maximum velocity | 8.0 ps |
| Hazen-Williams coefficient, ductile iron pipe | 120 |
| Hazen-Williams coefficient, PVC pipe | 130 |
| Minimum fire flow residual pressure | 20 psi (35 psi upstream of the FLMM/DDCVA) |
| Multifamily fire flow requirement | $2,000 \mathrm{gpm}$ |
| Commercial/Industrial fire flow requirement | $2,000 \mathrm{gpm}$ |

### 2.3. Potable Water Demand Development

Potable water demands for parcels in the WDW West District for both the short-term and long-term scenarios are presented in Table 2-3.

Table 2-3 OCU Potabis Water Demand Develapment


### 2.4. Hydraulic Analysis

### 2.4.1. Fire Flow Analysis

A maximum day demand plus fire flow analysis was performed for each parcel to assure pressures and velocities are within acceptable ranges according to OCU standards mentioned previously. Based on Orange County Fire Marshal Code of Ordinances Sec 30-247, the minimum fire flow for commercial properties is $2,000 \mathrm{gpm}$ with a residual pressure of 20 psi in the system at delivery point (residual pressure of 35 psi will be required upstream of the meter for each parcel to account for minor losses through meter assembly); RCID requires a fire flow of $2,000 \mathrm{gpm}$ with a residual pressure of 20 psi at point of delivery; 35 psi upstream of meter assembly.

### 2.4.1.1. Short-Term Scenario

Appendix B provides a summary of the fire flow analysis results for the short-term scenario. During the short-term scenario, the recommended 35 psi residual pressure requirement upstream of the meter assembly is achieved on all parcels. RCID has plans to install a booster pump station at the location presented on Figure 2-1 to achieve this residual pressure.

Per agreement between RCID and OCU dated December 19, 2012 (2012 Letter Agreement) and amended on January $19^{\text {th }} 2018$, RCID is required to provide $2,710 \mathrm{gpm}$ at 45 psi (maximum daily demand plus fire flow) to serve customers in the Orange Lake development located on Flamingo Crossings Boulevard labeled connection point in Figure 2-3.

This maximum day demand and fire flow requirement per the agreement was allocated in the model at the Orange Lake connection point and the model was simulated Appendix B includes a fire flow analysis for the Orange Lake development during the short-term scenario.

### 2.4.1.2. Long-Term Scenario

During this scenario, the potable water system adequately supplies fire flow at $\mathrm{FC}-1, \mathrm{FC}-2, \mathrm{Bl}-\mathrm{N}, \mathrm{BI}-$ $S$ and Orange Lake parcels. A comprehensive fire flow analysis for the long-term scenario is presented in Appendix B.

### 2.4.1.3. Short-Term Interim Construction Scenario

FC-1 and FC-2 parcel construction is estimated to begin in first quarter 2019. The timing of the construction of the booster pump station is estimated last quarter of 2019. Therefore, in the event the booster pump station is not online prior to FC-1 and FC-2 construction, additional scenarios were simulated to confirm adequate fire flows and pressures are provided during the "interim construction condition". During these scenarios existing pipes and proposed temporary pipes serving are used for delivery of demands and will be metered at the parcel hydrants. The scenarios simulated included the following:

1. FC-1. 2.000 gpm Fire Flow with maximum day demands only applied at the Orange Lake parcel.
2. FC-2: $2,000 \mathrm{gpm}$ Fire Flow with maximum day demands only applied at the Orange Lake parcel.
3. Orange Lake. 2.000 gpm Fire Flow with maximum day demands only applied at the Orange Lake parcel.

Modeling results for each scenario are provided in Appendix B, Table B-36 through Table B-44. these results indicate all residual pressure requirements are met. During this scenario the existing

16-inch piping along Flamingo Crossings Blvd. adequately supplies the required fire flow demand to FC-1, FC-2, and Orange Lake for construction activities. The intent is to allow vertical construction during this interim construction period. Certificates of occupancy would be contingent on certified completion of the booster pump station and all other site work permit requirements.


### 2.4.2. Peak Hour Analysis

A hydraulic analysis was performed to assure pressures and velocities are within acceptable ranges according to the standards mentioned previously during the peak hour scenario. In both the shortand long-term scenarios, the potable water system adequately supplies peak demands at each parcel per the design criteria presented in Section 2.2. The peak hour analysis for the short- and long-term scenarios are presented in Appendix B.

Peak hour analysis is not applicable during the short-term interim construction scenario.

### 2.5. System Summary

During the short-term scenario and short-term interim construction scenario, RCID will provide potable water for parcels FC, FC-1, FC-2, and Orange Lake. In the long-term scenario, ownership of the potable water system will be divided between RCID and OCU. RCID will own and maintain all potable water system infrastructure providing service to the FC parcel; while OCU will assume ownership of the utilities serving the FC-1, FC-2, and Orange Lake parcels. All water service connection shall be metered in accordance with the 2011 OCU Standards and Construction Specifications Manual (with revisions issued in 2014) Standards.

### 2.5.1. Fire Flow Analysis

### 2.5.1.1. Short Term Scenario

Parcels FC-1, FC-2 and Orange Lake are capable of achieving the required 35 psi residual pressure with a fire booster pump station. Figure $2-1$ illustrates the preliminary location for the fire booster pump station to accommodate fire flow demands and residual pressure requirements. The proposed booster pump stations will be owned and operated by RCID.

### 2.5.1.2. Long-Term Scenario

During the long-term scenario fire flow analysis, Piping is adequately sized for OCU to deliver the required residual pressure of 45 psi at the Orange Lake development per the amended Letter Agreement, dated January $19^{\text {th }}, 2018$.

### 2.5.1.3. Short-Term Interim Construction Scenario

During this scenario, existing piping is adequately sized to provide the fire flow needed for construction activities.

### 2.5.2. Peak Hour Analysis

in both the short- and long-term scenarios, the potable water system adequately supplies peak demands at each parcel per the design criteria presented in Section 2.2.

Peak hour analysis is not applicable to the short-term interim construction scenario.

## 3. Wastewater System

### 3.1. Introduction

As discussed previously, the WDW West District is east of County Road 545 and west of State Road 429. In the short-term scenario wastewater for the District will be served by RCID from the east. In the long-term scenario, once OCU infrastructure is available along County Road 545, wastewater for parcels located in the District (FC-1, FC-2, BI-N, BI-S, and Orange Lake) will be served by OCU from the west. Parcels $\mathrm{FC}-1, \mathrm{FC}-2, \mathrm{BI}-\mathrm{N}, \mathrm{BI}-\mathrm{S}$, and Orange Lake will be OCU customers. Figure 31 presents the wastewater system infrastructure by owner in the short-term scenario and Figure 32 presents the wastewater system infrastructure by owner in the long-term scenario. The section below outlines which municipality will own what parts of the wastewater system and the criteria that were used in the MUP design. BI will not come online in the short-term scenario, however, the ultimate (long-term) scenario includes Bl flows for adequate infrastructure sizing of the wastewater collection system

There are currently two existıng RCID wastewater force mains in the vicinity of WDW West District project area a 12 -inch force main running along Western Way, east of State Road 429 ending at the western edge of the FC parcel (the existing Flamingo Crossings lift station or LS-91); and a 12 -inch force main along Flamingo Crossings Blvd south of Western Way. There are also two existing gravity mains in the project area: a 10 -inch main along Flamingo Crossings Blvd. south of Western Way; and an 8 -inch/12-inch/15-inch gravity system within the FC parcel. The existing wastewater customers are currently connected to the RCID collection system.

A hydraulic model of the force main system was developed using Innovyze InfoWater. Two scenarios were created, one for the short-term in which all wastewater flow for FC, FC-1, FC-2, and Orange Lake is delivered to the RCID wastewater treatment facility (WWTF), and one for the long-term scenario in which the wastewater flow is divided between RCID and OCU The RCID tie-in pressure provided by RCES is 10 pounds per square inch (psi) and the OCU tie-in pressure along County Road 545 provided by OCU is 27 psi at an elevation of 120 feet (see Appendix A for OCU tie-in pressure). The long-term force main was sized based on OCU design criteria to provide a maximum velocity of 5 feet per second (fps).

The short-term scenario consists of FC, FC-1, FC-2, and Orange Lake parcels. In this scenario these parcels will be served by RCID. Flow from the Orange Lake and FC-1 developments will be directed through the existing 12 -inch Flamingo Crossings Blvd force main and the proposed 12 -inch force main along Western Way, flows will be discharged in the proposed manhole near LS-91. The 12inch force main along Flamingo Crossings Blvd. will be disconnected to separate ownership of the utility infrastructure between RCID and OCU (See Figure 3-1). FC-2 will discharge into the proposed 12 -inch force main along Western Way and flows will be directed to the proposed manhole near the LS-91 lift station, which will then flow by gravity to the LS-91 lift station. The wastewater from all parcels will then be pumped east to the RCID WWTF

The long-term scenario will include FC. FC-1, FC-2, $\mathrm{BI}-\mathrm{N}, \mathrm{Bl}-\mathrm{S}$, and Orange Lake. Similar to the short-term scenario, the existing LS-91 lift station will receive flows by gravity from parcel FC and will direct flows east to the RCID WWTF. The valve will be closed as shown in Figure 3-2 to direct wastewater flows west to the tie-in location along County Road 545. A 16-inch force main is proposed from $\mathrm{FC}-2$ to BI and a 20 -inch force main from BI to the tie-in location along County Road 545. Under this scenario, OCU will treat flows from parcels FC-1, FC-2, Orange Lake, and BI.



### 3.2. Gravity Collection System

Detailed wastewater flow development is outlined in Section 3.5. The OCU standard wastewater flow rate of $300 \mathrm{gpd} / E R \mathrm{U}$ was used for equivalent residential connections.

In the short-term and long-term scenanio, all existing gravity sewer west of State Road 429 will be owned and maintained by RCID and have been designed to their standards. The 10 -inch gravity main on the FC and FC-1 parcels will be plugged where the FC and FC-1 parcel boundaries meet. The remaining 10 -inch gravity main on the FC-1 parcel will be abandoned in place. All gravity sewer and the existing lift station within parcel FC will remain under the ownership of RCID.

### 3.2.1. RCID Gravity Collection System

The RCID gravity collection system will include all gravity sewer mains serving parcels east of State Road 429 and FC. Applicable RCID design criteria is outlined in Table 3-1.
Table 3-1 RCID Gravity Main Design Criteria

| Item | Recommended Criteria |
| :--- | :--- |
| Minimum slope, 8-inch | $0.40 \%$ |
| Minimum slope, 10-inch | $0.28 \%$ |
| Minimum slope, 12-inch | $0.22 \%$ |
| Minimum slope, 15-inch | $0.15 \%$ |

### 3.3. Lift Stations

In the short-term scenario there will be four (4) lift stations: FC-1, FC-2, LS-91 and Orange Lake. FC-1, FC-2 will be privately owned and operated. LS-91 will receive wastewater flows from all parcels during the short-term scenanio. In this scenario the LS-91 lift station will pump east to the RCID WWTF and will be owned and maintained by RCID.

In the long-term scenario there will be six (6) lift stations: FC-1, FC-2, LS-91, Orange Lake, BI-N and $\mathrm{BI}-\mathrm{S}$. The 12 -inch force main will be plugged prior to the proposed manhole that flows to LS-91 lift station and south of Western Way and Flagler Avenue. All wastewater flows will be directed west from the FC-1, FC-2, BI-N, BI-S, and Orange Lake parcels to the County Road 545 tie-in location. LS-91 lift station will continue to direct flows from Parcel FC east to the RCID WWTF.

### 3.3.1. RCID Lift Stations

During the shor-term scenario, parcels FC-1, FC-2 and Orange Lake will each have their own lift stations that will receive wastewater flows by gravity for each parcel respectively. The FC-1, FC-2 will be privately owned. The pumps included for each parcel lift station have been evaluated to confirm if they are capable of pumping under the short-term head conditions as well as the long-term head conditions. In conclusion, FC-1 and FC-2 lift station will require separate pumps for each the short-term and long-term head conditions. The pumps for each the short-term and long term are shown in the lift station calculations and pump curves included in Appendix $\mathbf{E}$.

LS-91 was designed based on a peak hourly flow (PHF) from all contributing parcels and will pump east to the RCID WWTF. All gravity wastewater mains and the existing lift station within parcel FC will remain under the ownership of RCID. Appendix H includes the LS-91 Lift Station pump curve.

### 3.3.2. OCU Lift Stations

OCU lift station design criteria is contingent upon the number of pumps required. Peak design flows less than $1,000 \mathrm{gpm}$ require two pumps while peak design flows between 1,000 and $2,500 \mathrm{gpm}$ require three pumps. Applicable OCU lift station design criteria is outlined in Table 3-2.

Table 3-2 OCU Lift Station Design Criteria

| Item | Recommended Criteria |  |
| :--- | :--- | :--- |
|  | 2 pumps | 3 pumps |
| Number of wet wells | 1 | 1 |
| Wet well structure type | Precast | Precast |
| Piping (below or above ground) | Below | Above |
| SCADA | Yes | Yes |
| Biofilter | No | Yes |
| Generator | FDEP | Yes |
| Level control | Float ball | Float ball |
| SCADA panel | Type 2 | Type 3 |

### 3.4. Force Mains

### 3.4.1. RCID Force Mains

In the short-term scenario the existing force main alignments can be seen in Figure 3-1. Applicable RCID design criteria is outlined in Table 3-3.

## Table 3-3 RCID Force Main Design Criteria

| Item | Recommended Criteria |
| :--- | :--- |
| Maximum headloss | 5 feet/1,000 feet of pipe |
| Maximum velocity in force mains > 10-inch | 7 fps |
| Maximum velocity in force mains $\leq 10$-inch | 5 fps |
| Hazen-Williams coefficient, ductile iron pipe | 120 |
| Hazen-Williams coefficient, PVC pipe | 130 |

* All force mains that will be transferred to OCU will be designed per the OCU 2011 Design Criteria.


### 3.4.2. OCU Force Mains

A 12-inch force main is proposed along Western Way will direct flows from the FC-1, FC-2, Orange Lake lift stations to a manhole that will flow by gravity to the LS-91 lift station during the short-term. The 12 -inch force main was sized based on the ultimate build-out flows during the long-term scenario, which will be used in the long-term scenario to direct flows to the tie-in location at County Road 545 along with a 16 -inch force main from the FC-2 parcel to the BI parcels, and 20 -inch force main form BI parcels to the tie-in location. Refer to Figure 3-2 for the force main schematic.

Table 3-4 includes the recommended OCU criteria for the design of force mains. It must be noted that all OCU maintained force mains must flow at a minimum velocity of 2.5 fps and a maximum velocity of 5 fps .

## Table 3-4 OCU Force Main Design Criteria

| Item | Recommended Criteria |
| :--- | :--- |
| Minimum Pipe Diameter | 4 inches |
| Minimum Velocity | 2.5 fps |
| Maximum Velocity | 5.0 fps at peak flow rate |

### 3.5. Wastewater Flow Generation Development

Wastewater flow generation for parcels in the WDW West District for the short- and long-term scenarios are presented in and Table 3-5.

Table 2-s OCU Wastowator Flow Dovelopment

| Preed | Description | Unit Type | Eat tinity | Pactor | Whatrwater ERC | No. Ot ERC': | Flow per Unik (gpd) | Avarnge Cally Flow <br>  | Average Dally fiow ( g pm ) | Praking Factor | Peent Howr Flow (gpm) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| FC-1 | Retumimit over 100 soth | Sentio | 2,000 | 0.1 | 300 | 200 | 30 | 60,000 | 42 |  | 123 |
| F-1 | Mulitumily - $2+$ Esdroome | Apantiment | 1,32* | 0.635 | 300 | 1.106 | 230 | 331,697 | 230 |  | 691 |
| Toter flow from FG=1 |  |  |  |  |  | 1,208 | 280 | 381,097 | 272 | 3.0 | 818 |
| FC-2 | Reateurant over 100 caste | Sente | 2,000 | 0.1 | 300 | 200 | 30 | 80,000 | 42 |  | 128 |
|  | Muliflemily - 2 + Eecroome | Apartment | 1.330 | 0.003 | 300 | 1.133 | 280 | 339,884 | 238 |  | 708 |
| Total Plow from FC.? |  |  |  |  |  | 1,323 | $2{ }^{\text {to }}$ | 300,304 | 271 | 3.0 | 288 |
| B-Nam* | Restaurant over 100 eteth | Seate | 1,000 | 0.1 | 300 | 100 | 30 | 30,000 | 21 |  | 73 |
|  | Hotel-1 Eadroom | Room | 128 | 0.5 | 300 | 64 | 150 | 18.200 | 13 |  | 47 |
|  | Hotel - Sult | Reom | 328 | 0.83 | 300 | 272 | 249 | 61,872 | 37 |  | 198 |
| Totel Plow from ${ }_{\text {clill }}$ - North |  |  |  |  |  | 458 | 425 | 130.872 | 11 | 3.5 | 31 |
| $\mathrm{Bl}=\mathrm{Sow}{ }^{\text {a }}$ | Routhurant over 100 eantio | Sation | 1.000 | 0.1 | 300 | 100 | 30 | 30,000 | 21 |  | 73 |
|  | Motel - 1 Eodroom | Room | 188 | 0.5 | 300 | 08 | 130 | 28.000 | 20 |  | 70 |
|  | Hotel - Suht | Room | 328 | 0.35 | 300 | 272 | 248 | 11,672 | 37 |  | 100 |
| Totes flow from ${ }^{\text {a }}$ - South |  |  |  |  |  | 44 | 48 | 140,472 | H | 2.8 | 341 |
| Oronge Latwe ${ }^{\text {min }}$ |  |  |  |  |  |  |  | 448,009 | 50.2 | 2.0 | 03 |
| Orand Total |  |  |  |  |  | 2843 | 1,418 | 7,498,874 | 1,098 |  | 3,20e |


NOTE; The iff etation pumpe at FC-1 and FC-2 will be required to be upsiaed in the long term.

### 3.6. System Summary

### 3.6.1. Short-Term Scenario

As shown in Figure 3-1, the RCID portion of the wastewater collection and transmission system in the short-term scenario will consist of

- The existing gravity sewer serving parcel FC;
- The existing LS-91 lift station;
- The proposed 16 -inch force main discharging flows to the proposed manhole outside of LS91; and
- The existing 12 -inch force main from the LS-91 lift station along Western Way to the RCID WWTF

The portion of the collection system that OCU will own and maintain during the short term includes

- The existing 12 -inch force main along Flamingo Crossings Blvd.
- The proposed 12 -inch force main from the disconnected force main along Flaming Crossings Blvd and continues on Western Way to the FC-2 parcel (Note: BI will be constructed and served during the long-term scenario) and the 16 -inch force main from FC-2 to the BI parcels, and the 20 -inch from Bl to the tie-in location on County Road 545;

The following infrastructure will be privately owned and maintained:

- The proposed gravity sewer and lift station serving the FC-1 parcel; and
- The proposed gravity sewer and lift station serving the FC-2 parcel


### 3.6.1.1. Short-Term Scenario Hydraulic Model Results

The modeling analysis concluded that the long-term pumps for FC-1 and FC-2 cannot perform under the low head conditions during the short-term scenario In conclusion, specific pumps have been selected for this scenario that included the required lower head. They are included in Appendix $E$ During the long term scenario these pumps will need to be replaced with higher head pumps

### 3.6.2. Long-Term Scenario

As shown in Figure 3-2, the RCID portion of the wastewater collection and transmission system in the long-term scenario will consist of:

- The existing gravity sewer serving parcel FC:
- The existing LS-91 lift station;
- The existing 12 -inch force mains from the LS-91 lift station along Western Way to the RCID WWTF: and
- The proposed 16 -inch FM discharging flow to the proposed MH outside of LS-91 lift station

The portion of the collection system that OCU will own and maintain during the long term includes:

- The existing 12 -inch force main along Flamingo Crossings Blva.
- The proposed 12 -inch and 16 -inch/20-inch force main along Western Way to the tie-in location on County Road 545;

The following infrastructure will be privately owned and maintained:

- The proposed gravity sewer and lift station serving the FC-1 parcel;
- The proposed gravity sewer and lift station serving the FC-2 parcel; and
- The proposed gravity sewer and lift stations serving the BI-N and BI-S parcel.


### 3.6.2.1. Long-Term Scenario Hydraulic Model Results

The maximum velocity in the proposed force mains flowing towards the OCU tie-in location at County Road 545 is 5.0 fps .

A summary of each proposed lift stations design points and downstream junctions can be seen below in Table 3-6.

Table 3-6 Wastewater Model Lift Station Summary

| Parcel | Junction ID | Pump ID | PHF (gpm) | Pump Operating Point | Phase |
| :---: | :---: | :---: | :---: | :---: | :---: |
| FC-1 | J98 | FC_1_PMP1 | 816 | 846 gpm @ 31 tt | Short Term |
|  |  | FC_1_PMP2 |  | 891 gpm @ 142 ft | Long Term |
| FC-2 | $J 106$ | FC_2_PMP1 | 833 | 856 gpm @ 14 ft | Short Term |
|  |  | FC_2_PMP2 |  | 907 gpm @ 111 ft | Long Term |
| BI-N | J116 | BI_N_PMP1 | 318 | 395 gpm @ 107 ft | Long Term |
|  |  | BI_N_PMP2 |  |  |  |
| BI-S | J108 | BI_S_PMP1 | 341 | 343 gpm @ 94 色 | Long Term |
|  |  | BI_S_PMP2 |  |  |  |
| Orange Lake | J34 | - | 900 | - | Shor/Long |

NOTE: Minor losses were not accounted for in the hydraulic models. These losses shall be accounted for in the lift station calculations at the time of the construction plan submittal.

## 4. Reclaimed Water System

### 4.1. Introduction

As previously discussed, the WDW West District is located along Western Way, east of County Road 545 and west of State Road 429. In the short-term scenario, reclaimed water for the District will be served by RCID from the east. In the long-term scenario, reclaimed water will be provided by OCU from the west Figure 4-1 presents the reclaimed water infrastructure by owner in the short-term scenario and Figure $4-2$ presents the reclaimed water infrastructure by owner in the long-term scenario. The section below outlines which municipality will own what parts of the reclaimed water system and the criteria that were used in the MUP design.

There are currently four existing reclaimed water mains near the project: a 12 -inch reclaimed water main along Western Way east of Flamingo Crossings Blvd.; an 8 -inch reclaimed water main west of Flamingo Crossings Blvd. that ends at the western edge of the FC parcel; an 8 inch reclaimed water main along Flamingo Crossings Blvd. south of Western Way; and an 8 -inch / 6 -inch reclaimed water main system on the FC parcel.

A hydraulic model was developed using Innovyze InfoWater. Two scenarios were created: one for the short-term in which all reclaimed water is served by RCID and one for the long-term scenario in which the reclaimed water service is divided between RCID and OCU. The tie-in pressure assumed for the RCID reclaimed distribution system is 83 psi (provided by RCES) and 56 psi for OCU tie-in pressure along County Road 545 (see Appendix A for OCU tie-in pressure). The pipes were sized to provide a minimum pressure of 35 psi at the points of delivery. per OCU Manual Section 2510 , Part 6B. Topographic elevations were imported into the model through available shapefiles and utilized in the hydraulic calculations

The short-term scenario consists of FC, FC-1, FC-2, and Orange Lake Parcels. $\mathrm{Bl}-\mathrm{N}$ and $\mathrm{BI}-\mathrm{S}$ will are not included in the short term scenario $\mathrm{BI}-\mathrm{N}$ and $\mathrm{BI}-\mathrm{S}$ will be constructed during the long-term scenario In this scenario, all parcels will be served by RCID. A booster pump station will be required to increase the pressure at the Orange Lake Development per the amended 2018 Letter Agreement (Appendix F).

The western boundary of the 20-inch reclaimed main will be valved closed during the short-term scenario after serving the FC-2 parcels from the east. RCID and OCU mains will be connected near Flamingo Crossings Boulevard and a wholesale reclaimed water meter and interconnect will be installed (See Figure 4-1). The existing 8-inch reclaimed water main along Flamingo Crossings Blvd will remain in place during the short-term scenario OCU will own and maintain the 8 -inch main along the FC-1 parcel, RCID will own and maintain the portion along the FC parcel.

The reclaimed water system will have valves in the following locations: 1.) Western Way and County Road 545, and 2.) Western Way and FC-2 parcel. (see Figure 4-1).

The long-term scenario will include parcels FC-1, FC-2, Orange Lake, BI-N, and BI-S. In this scenario, when OCU infrastructure becomes available, the gate valves located on Western Way at County Road 545 and FC-2 will be opened and the 20 -inch proposed reclaimed water main will tiein to the OCU reclaimed main along County Road 545 . Under this scenario, OCU will serve all parcels $\mathrm{FC}-1$, $\mathrm{FC}-2$ Orange Lake, $\mathrm{BI}-\mathrm{N}$. and $\mathrm{Bi}-\mathrm{S}$, while RCID will serve FC



### 4.2. Design Criteria

Please refer to Figure 4-1 and Figure 4-2 for pipe ownership. Applicable RCID design criteria is presented in Table 4-1. Detailed reclaimed water demands per parcel are outlined in Section 4.3.

Table 4-1 RCID Reclaimed Water Design Criteria

| Item | Recommended Criteria |
| :--- | :--- |
| Maximum velocity | 8 fps |
| Peaking Factor | 3 |
| Hazen-Williams coefficient for ductile iron | 120 |
| Hazen-Williams coefficient for PVC | 130 |

Please refer to Figure 4-1 and Figure 4-2 for all public reclaimed water mains in the short-term and long-term scenarios that are to be owned and maintained by OCU and will be designed to the 2011 OCU Standards and Construction Specifications Manual (with revisions issued in 2014) Standards.

Applicable OCU design criteria is presented in Table 4-2.

## Table 4-2 OCU Service Area Reclaimed Water Design Criteria

| Item | Recommended Criterla |
| :--- | :--- |
| Maximum velocity, ductile iron pipe | 8 fps |
| Maximum velocity, PVC pipe | 5 fps |
| Peaking Factor | 6 |
| Hazen-Williams coefficient for ductile iron | 120 |
| Hazen-Williams coefficient for PVC | 130 |

### 4.3. Reclaimed Water Demand Development

The long-term demands for the system were developed using the 2011 OCU Standards and Construction Specifications Manual (with revisions issued in 2014). The actual irrigable acres for the FC-1 and FC-2 parcels were taken from the design plans for the development on those parcels. For all other parcels it was assumed that $20 \%$ of each parcel was irrigable. The irrigation rate for all parcels utilized was 1 -inch per week, two days of irrigation per week for commercial parcels are listed below in Table 4-3.

Table 4-3 OCU Reclaimed Water Demand Development

| Parcel | Junction ID | Land Use | Jurlsdiction |  | Irrigable Area (\%) | Irrigable Acres | Irrigation Rate (in/week) | Gallons f Week | Reciaim Average Dally De | Water nnual and | Peaking Factor | Reclaimed Water Peak Hourly Demand (gpm) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  | GPD | GPM |  |  |
| FC- 1 $^{1}$ | . 988 | Multi- <br> Family/Reta | OCU | 80.3 | - | 13 | 1 | 351,895 | 175,948 | 122.0 | 6 | 733 |
| FC- ${ }^{1}$ | J24 | MultiFamily/Retail | OCU | 60.7 | - | 15 | 1 | 408,101 | 204,050 | 142.0 | 6 | 850 |
| $\mathrm{BI}-\mathrm{N}^{2}$ | J22 | Tourist Commercial | OCU | 49.6 | 20\% | - | 1 | 269,352 | 134,676 | 93.5 | 6 | 561 |
| Bl-S ${ }^{2}$ | J20 | Tourist Commercial | OCU | 496 | 20\% | - | 1 | 269,352 | 134,676 | 93.5 | 6 | 561 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
| Grand Total |  |  |  |  |  |  |  | 1,298,699 | 793,350 | 551 |  | 3,305 |

"Bear island parcel demands are not included in the short-term scenanio

1 The actual irrigable acres for the FC-1 and FC-2 parcels were taken from the design plens for the development of those parcels. Reclaimed water demands were calculated based on the irrigable acres
2 The irnigable area was estimated based on recommended $20 \%$ area per OCU, demand astimates should be confirmed when the conceptual layouts for each parcel are finalized
3 Per 2017 Amended Letter Agreement between RCID and OCU

### 4.4. System Summary

During the short-term scenario RCID will provide reclaimed water for all parcels in the WDW West District. In order for RCID to supply 600 gpm at 50 psi to the Orange Lake development per the 2012 Letter Agreement (amended in 2018), a booster pump station is required. A preliminary design point for the pump was determined to be $2,500 \mathrm{gpm} @ 75$ feet of head. This design point was based on the peak hour flows presented in Table 4-3. The booster pump will be located East of 429 on RCID property, near the potable water booster pump station presented in Section 2. The interconnect assembly was sized using an 8 -inch dual bidirectional backflow preventers, to decrease the amount of losses through this assembly a 12 -inch may be used

In the long-term scenario, ownership of the reclaimed water system will be divided between RCID and OCU. OCU will serve parcels $\mathrm{FC}-1, \mathrm{FC}-2$, Orange Lake, $\mathrm{BI}-\mathrm{N}$, and $\mathrm{BI}-\mathrm{S}$. RCID will serve the FC parcel. All peak system pressure requirements are met in the long term scenario with proposed infrastructure.

See Appendix D for detailed model results, for the short and long term scenarios

## Appendix A. Tie-in Pressure Data



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February 2. 2018
Mr. Kunal Nayee EI
Atkıns Group
482 south Keller Road
Orlando. Florida 32810

## E-mail: Kunal.Nayeeotatkinsqlobal.com

Subject: Hydraulic Analysis for Connection to Collection/Transmission System Southern Service Area
Flamingo Crossings - Reclaimed - Revised Build Out

## Dear Mr Nayee

We are responding to your request for hydraulic information for the above-mentioned project. located in the southwest corner of Section 21 Township 24 South. Range 27 East This analysis is based on a development having the following flows Please note that a review of the proposed development flows was not evaluated as part of this response. and that water and wastewater capacity is not reserved untll capital charges are paid pursuant to Orange County Code This letter includes boundary conditions to be used to perform preliminary designs for water and/or wastewater networks Please note that the connection elevations were assumed and that all utility elevations should be field verfied

Table 1. Estimated Flows - Buildout Conditions

| Descnption | Reclaimed Water |  | Wastewater |  |
| :--- | :---: | :---: | :---: | :---: |
|  | Flow <br> (gpm) | Peaking <br> Factor | Flow <br> (gpm) | Peaking <br> Factor |
| Average Daily Flow |  |  |  |  |
| Max Day Flow |  |  |  |  |
| Peak Flow | $* 3,097$ |  |  |  |
| Fire Flow |  |  |  |  |
| Fire Flow + MDF |  |  |  |  |

* Denotes flow provided by customer.

With respect to our reclaimed water system. assume a connection to a future reclaimed main along Avalon Road approximately 15 miles north of the intersection with Hartzog Road For design purposes assume a minimum workıng hydraulic grade of 250 feet will be maintained in the future reclaimed water main for flows up to the above referenced estimated flows This HGL is composed of 120 feet of elevation and 130 feet ( 56 psi) of pressure head

Please note that the hydraulic conditions presented above will be avallable after improvements to the reclaimed system are operational Note that these projects are outside of our 5 year CIP window

Please call me at 407-254-9917 if you have additional questions


Paul E Partlow. P E
Senior Engineer

February 2. 2018
Mr Kunal Nayee. E I
Atkins Group
482 south Keller Road
Orlando. Florida 32810

## E-mail: Kunal.Naveo@atkinsglobal.com

Subject: Hydraulic Analysis for Connection to Collection/Transmission System Southern Service Area Flamingo Crossings - Water - Revised Build Out

## Dear Mr Nayee

We are responding to your request for hydraulic information for the above-mentioned project. located in and around Section 21 Township 24 South, Range 27 East. This analysis is based on a development having the following flows Please note that a review of the proposed development flows was not evaluated as part of this response. and that water and wastewater capacity is not reserved until capital charges are paid pursuant to Orange County Code This letter includes boundary conditions to be used to perform prelıminary designs for water andfor wastewater networks

Table 1. Estimated Flows - Buildout Conditions

| Description | Water |  | Wastewater |  |
| :--- | ---: | ---: | ---: | ---: |
|  | Flow <br> $(\mathrm{gpm})$ | Peaking <br> Factor | Flow <br> $(\mathrm{gpm})$ | Peaking <br> Factor |
| Average Daily Flow | 1,216 | 1.0 |  |  |
| Max Day Flow | 2,432 | 2.0 |  |  |
| Peak Flow | 4,864 | 4.0 |  |  |
| Fire Flow | 2,000 |  |  |  |
| Fire Flow + MDF | 4,432 |  |  |  |

* Denotes flow provided by customer.

With respect to our water system, we assumed a connection to the existing 24 -inch water main along Avalon Road approximately 15 miles north of the intersection with Hartzog Road According to our model, for design purposes assume a minimum working hydraulic grade of 250 feet will be maintained in the existing water main for flows up to the above referenced Estımated Flows This HGL is composed of 120 feet of elevation and 130 feet ( 56 psi ) of pressure head Please note that the connection elevation was assumed and that all water main elevations should be field verified
Please note that these pressure and demand conditions assume that the planned Malcolm Road WSF is operational, all major loops within the Horizons West Developments are in service, and that a future storage re-pump facility in the vicinity of Avalon and Seidel Roads is in operation. These flows will NOT be avallable until this condition is met
Sincerely.

[^0]February 5. 2018
Mr Kunal Nayee. EI
Atkins Group
482 south Keller Road
Orlando, Florida 32810

## E-mail: Kunal.Nayoe@atkinsglobal.com

Subject: Hydraulic Analysis for Connection to Collection/Transmission System Southern Service Area
Flamingo Crossings - Wastewater - Revised Build Out

## Dear Mr Nave

We are responding to your request for hydraulic information for the above-mentioned project. located in the southwest corner of Section 21 Township 24 South. Range 27 East This analysis is based on a development having the following flows. Please note that a review of the proposed development flows was not evaluated as part of this response, and that water and wastewater capacity is not reserved untIl capital charges are paid pursuant to Orange County Code This letter includes boundary conditions to be used to perform preliminary designs for water and/or wastewater networks Please note that the connection elevations were assumed and that all utility elevations should be field verified

Table 1. Estimated Flows - Buildout Conditions

| Descnption | Water |  | Wastewater |  |
| :--- | :---: | :---: | ---: | ---: |
|  | Flow <br> $(\mathrm{gpm})$ | Peaking <br> Factor | Flow <br> (gm) | Peaking <br> Factor |
| Average Daily Flow |  |  | 1,041 | 1.0 |
| Max Day Flow |  |  |  | 2.603 |
| Peak Flow |  |  | 2,6 |  |
| Fire Flow |  |  |  |  |
| Fire Flow + MDF |  |  |  |  |

Denotes flow provided by customer
With respect to our wastewater system. we assumed a connection to a proposed 20 inch force main along Avalon Road approximately 15 miles north of the intersection with Hertzog Road This force main will flow to a future Avalon Road master pumping station located in the general vicinity of N28 24.9, W81 380 with an assumed outfall based on ground elevation of 130 feet. According to our model, the hydraulic grade line at your point of connection for flows up to the above referenced estimated flows is 182 feet. which is composed of 120 feet of elevation and 62 feet ( 27 psi ) of pressure head
Please note that the hydraulic conditions presented above will be available after the new water reclamation facility and proposed Avalon Road master pump station is operational Note that these projects are outside of our 5 year CIP window.

Please call me at 407-254-9917 if you have additional questions
Sincerely.


Paul E Partlow. PE
Senior Engineer

## Appendix B. Potable Water



Table B-1 Short-Term Scenario Orange Lake Fire Flow Results - Junction Report

| 10 | Demand (gpm) | Elevation (fi) | Head (ti) | Pressure (psi] |
| :---: | :---: | :---: | :---: | :---: |
| J98 | 0 | 111 | 201 | 39.0a |
| J96 | 0 | 111 | 202 | 39.5 |
| J32 | 0 | 111 | 204 | 40.3 |
| J94 | 634 | 120 | 215 | 41.3 |
| J78 | 0 | 113 | 211 | 42.5 |
| J86 | 0 | 116 | 216 | 43.1 |
| J88 | 0 | 114 | 218 | 45.1 |
| J66 | 2,710 | 106 | 210 | 45.2 |
| J64 | 0 | 114 | 223 | 47.2 |
| J18 | 0 | 114 | 227 | 49.0 |
| J118 | 0 | 114 | 227 | 49.1 |
| J120 | 0 | 114 | 227 | 49.1 |
| J56 | 0 | 107 | 223 | 50.2 |
| J54 | 432 | 109 | 227 | 51.0 |
| J22 | 0 | 105 | 223 | 51.1 |
| J34 | 648 | 104 | 223 | 51.4 |
| J116 | 0 | 111 | 231 | 51.8 |
| J100 | 0 | 111 | 231 | 52.0 |
| J20 | 0 | 102 | 223 | 52.4 |
| J110 | 0 | 111 | 234 | 53.4 |
| J52 | 0 | 102 | 227 | 54.2 |
| J16 | 392 | 98 | 227 | 56.1 |
| J114 | 0 | 98 | 227 | 56.1 |
| J38 | 94 | 96 | 227 | 56.9 |
| J46 | 0 | 95 | 272 | 76.8 |
| J50 | 0 | 95 | 281 | 80.7 |
| J10 | 0 | 105 | 300 | 84.6 |
| J40 | 0 | 97 | 297 | 86.8 |
| J30 | - | - | - | - |
| J36 | - | - | - | - |
| J24 | - | - | - | - |
| J112 | - | - |  | - |

a. Denotes critical node during fire flow simulation

Table B-2 Short-Term Scenario Orange Lake Fire Flow Results - Reservoir Report

| ID | Flow (gpm) | Head (ft) | Description |
| :--- | :--- | :--- | :--- |
| RES9002 | $-4,910$ | 304 | HGL provided by RCID |

Table B-3 Short-Term Scenario Orange Lake Fire Flow Results - Pipe Report

| ID | Owner | From Node | To Node | Length (f) | Diameter (in) | Roughness | $\begin{aligned} & \text { Flow } \\ & \text { (gpm) } \end{aligned}$ | $\begin{aligned} & \text { Velocity } \\ & \text { (ft's) } \end{aligned}$ | Headloss (ft) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P11 | RCID | J10 | J40 | 212 | 16 | 120 | 4,910 | 7.8 | 2.9 |
| P117 | OCU | J86 | J78 | 990 | 16 | 120 | 2,710 | 4.3 | 4.5 |
| P121 | OCU | J88 | J86 | 367 | 16 | 120 | 3,344 | 5.3 | 2.5 |
| P13 | Private | J114 | J16 | 376 | 16 | 120 | 392 | 0.6 | 0.1 |
| P133 | Private | J86 | J94 | 184 | 12 | 120 | 634 | 1.8 | 0.2 |
| P137 | RCID | J96 | J18 | 271 | 16 | 120 | 0 | 0.0 | 0.0 |
| P149 | RCID | J96 | J98 | 102 | 16 | 120 | 4,424 | 7.1 | 1.2 |
| P15 | RCID | J18 | J52 | 1225 | 12 | 120 | 162 | 0.5 | 0.1 |
| P155 | RCID | J100 | J116 | 50 | 16 | 120 | 4,424 | 7.1 | 0.6 |
| P159 | RCID | J110 | J100 | 33 | 16 | 120 | 4,424 | 7.1 | 3.1 |
| P17 | OCU | J20 | J22 | 2631 | 24 | 120 | 0 | 0.0 | 0.0 |
| P177 | RCID | J114 | J32 | 2071 | 16 | 120 | 4,424 | 7.1 | 23.5 |
| P189 | RCID | J116 | J120 | 285 | 16 | 120 | 4,424 | 7.1 | 3.2 |
| P191 | RCID | $J 98$ | U7014 | 57 | 16 | 120 | 4,424 | 7.1 | 0.7 |
| P193 | RCID | U7014 | J110 | 45 | 16 | 120 | 4,424 | 7.1 | 0.5 |
| P195 | RCID | J118 | J18 | 408 | 16 | 120 | 432 | 0.7 | 0.1 |
| P197 | RCID | J120 | $J 64$ | 459 | 16 | 120 | 3,992 | 6.4 | 4.3 |
| P199 | RCID | J120 | J118 | 109 | 12 | 120 | 432 | 1.2 | 0.1 |
| P23 | RCID | J18 | J54 | 1461 | 12 | 120 | 270 | 0.8 | 0.4 |
| P29 | Private | J20 | J34 | 329 | 16 | 120 | 648 | 1.0 | 0.1 |
| P35 | Private | J114 | J38 | 231 | 12 | 120 | 93.6 | 0.3 | 0.0 |
| P37 | RCID | RES9002 | J10 | 291 | 16 | 120 | 4,910 | 7.8 | 4.0 |
| P39 | RCID | J40 | J50 | 1171 | 16 | 120 | 4,910 | 7.8 | 16.2 |
| P43 | RCID | J46 | J114 | 3242 | 16 | 120 | 4,910 | 7.8 | 44.7 |
| P47 | RCID | J50 | J46 | 662 | 16 | 120 | 4,910 | 7.8 | 9.1 |
| P51 | RCID | J32 | J96 | 155 | 16 | 120 | 4,424 | 7.1 | 1.8 |
| P57 | RCID | J52 | J54 | 2515 | 12 | 120 | 162 | 0.5 | 0.3 |
| P59 | OCU | J56 | J20 | 1408 | 24 | 120 | 648 | 0.5 | 0.1 |
| P63 | OCU | J56 | J64 | 1903 | 24 | 120 | -648 | 0.5 | 0.1 |
| P67 | OCU | J64 | J88 | 735 | 16 | 120 | 3,344 | 5.3 | 5.0 |
| P69 | OCU | J78 | J66 | 151 | 16 | 120 | 2,710 | 4.3 | 0.7 |
| P31 | - | - | - | - | - | - | - | - | - |
| P25 | - | - | - | - | - | - | - | - | - |
| P19 | - | - | - | - | - | - | - | - | - |
| P183 | - | - | - | - | - | - | - | - | - |
| P173 | - | - | - | - | - | - | - | - | - |
| P61 | - | - | - | - | - | - | - | - | - |
| P143 | - | - | - | - | - | - | - | - | - |

## Table B-4 Short-Term Fire Flow analysis

| ID | Static Demand (gpm) | Static Pressure (psi) | Static <br> Head (f) | Fire-Flow Demand (gpm) | Residual Pressure (psi) | Available Flow at Hydrant (gpm) | Available Flow Pressure (psi) | Critical Pipe ID | Critical Pipe Velocity (fis) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| J34 | 648 | 86.9 | 304.5 | 2,000 | 50 | 3,268 | 35.0 | P29 | 5.2 |
| J66 | 710 | 85.3 | 302.9 | 2,000 | 45 | 3,099 | 35.0 | P69 | 4.9 |
| J94 | 634 | 79.4 | 303.2 | 2,000 | 40 | 2,820 | 35.3 | P133 | 8.0 |

Table B-5 Short-Term Scenario FC-1 Fire Flow Results - Junction Report

| ID | Demand (gpm) | Elevation (ft) | Head (ft) | Pressure (psi) |
| :---: | :---: | :---: | :---: | :---: |
| J98 | 0 | 111 | 201 | 39.0a |
| J96 | 0 | 111 | 202 | 39.5 |
| J94 | 2,634 | 120 | 212 | 40.0 |
| J32 | 0 | 111 | 204 | 40.3 |
| J86 | 0 | 116 | 216 | 43.1 |
| J78 | 0 | 113 | 215 | 44.3 |
| J88 | 0 | 114 | 218 | 45.1 |
| J64 | 0 | 114 | 223 | 47.2 |
| J66 | 710 | 106 | 215 | 47.3 |
| J18 | 0 | 114 | 227 | 49.0 |
| J118 | 0 | 114 | 227 | 49.1 |
| J120 | 0 | 114 | 227 | 49.1 |
| J56 | 0 | 107 | 223 | 50.2 |
| J54 | 432 | 109 | 227 | 51.0 |
| J22 | 0 | 105 | 223 | 51.1 |
| J34 | 648 | 104 | 223 | 51.4 |
| J116 | 0 | 111 | 231 | 51.8 |
| J100 | 0 | 111 | 231 | 52.0 |
| J20 | 0 | 102 | 223 | 52.4 |
| J110 | 0 | 111 | 234 | 53.4 |
| J52 | 0 | 102 | 227 | 54.2 |
| J16 | 392 | 98 | 227 | 56.1 |
| J114 | 0 | 98 | 227 | 56.1 |
| J38 | 94 | 96 | 227 | 56.9 |
| J46 | 0 | 95 | 272 | 76.8 |
| J50 | 0 | 95 | 281 | 80.7 |
| J10 | 0 | 105 | 300 | 84.6 |
| J40 | 0 | 97 | 297 | 86.8 |
| J30 | - | - | - | - |
| J36 | - | - | - | - |
| J24 | - | - | - | - |
| J112 | - | - | - | - |

a. Denotes critical node during fire flow simulation

Table B-6 Short-Term Scenario FC-1 Fire Flow Results - Reservoir Report

| ID | Description | Flow (gpm) | Head (ft) |
| :--- | :--- | :--- | :--- |
| RES9002 | HGL provided by RCID | $-4,910$ | 304 |

Table B-7 Short-Term Scenario FC-1 Fire Flow Results - Pipe Report

| ID | Owner | From Node | To Node | Length <br> ( ft ) | Diameter (in) | Roughness | Flow (gpm) | Velocity (ft/s) | Headloss <br> ( ft ) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P11 | RCID | J10 | J40 | 212 | 16 | 120 | 4,910 | 7.8 | 2.9 |
| P117 | OCU | J86 | J78 | 990 | 16 | 120 | 710 | 1.1 | 0.4 |
| P121 | OCU | J88 | J86 | 367 | 16 | 120 | 3,344 | 5.3 | 2.5 |
| P13 | Private | J114 | J16 | 376 | 16 | 120 | 392 | 0.6 | 0.1 |
| P133 | Private | J86 | J94 | 184 | 12 | 120 | 2,634 | 7.5 | 3.3 |
| P137 | RCID | J96 | J18 | 271 | 16 | 120 | 0 | 0.0 | 0.0 |
| P149 | RCID | J96 | J98 | 102 | 16 | 120 | 4,424 | 7.1 | 1.2 |
| P15 | RCID | J18 | J52 | 1,225 | 12 | 120 | 162 | 0.5 | 0.1 |
| P155 | RCID | J100 | J116 | 50 | 16 | 120 | 4,424 | 7.1 | 0.6 |
| P159 | RCID | J110 | J100 | 33 | 16 | 120 | 4,424 | 7.1 | 3.1 |
| P17 | OCU | J20 | J22 | 2,631 | 24 | 120 | 0 | 0.0 | 0.0 |
| P177 | RCID | J114 | J32 | 2,071 | 16 | 120 | 4,424 | 7.1 | 23.5 |
| P189 | RCID | J116 | J120 | 285 | 16 | 120 | 4,424 | 7.1 | 3.2 |
| P191 | RCID | J98 | U7014 | 57 | 16 | 120 | 4,424 | 7.1 | 0.7 |
| P193 | RCID | U7014 | J110 | 45 | 16 | 120 | 4,424 | 7.1 | 0.5 |
| P195 | RCID | J118 | J18 | 408 | 16 | 120 | 432 | 0.7 | 0.1 |
| P197 | RCID | J120 | J64 | 459 | 16 | 120 | 3,992 | 6.4 | 4.3 |
| P199 | RCID | J120 | J118 | 109 | 12 | 120 | 432 | 1.2 | 0.1 |
| P23 | RCID | J18 | J54 | 1,461 | 12 | 120 | 270 | 0.8 | 0.4 |
| P29 | Private | J20 | J34 | 329 | 16 | 120 | 648 | 1.0 | 0.1 |
| P35 | Private | J114 | J38 | 231 | 12 | 120 | 94 | 0.3 | 0.0 |
| P37 | RCID | RES9002 | J10 | 291 | 16 | 120 | 4,910 | 7.8 | 4.0 |
| P39 | RCID | J40 | J50 | 1,171 | 16 | 120 | 4,910 | 7.8 | 16.2 |
| P43 | RCID | J46 | J114 | 3,242 | 16 | 120 | 4,910 | 7.8 | 44.7 |
| P47 | RCID | J50 | J46 | 662 | 16 | 120 | 4,910 | 7.8 | 9.1 |
| P51 | RCID | J32 | J96 | 155 | 16 | 120 | 4,424 | 7.1 | 1.8 |
| P57 | RCID | J52 | J54 | 2,515 | 12 | 120 | 162 | 0.5 | 0.3 |
| P59 | OCU | J56 | J20 | 1,408 | 24 | 120 | 648 | 0.5 | 0.1 |
| P63 | OCU | J56 | J64 | 1,903 | 24 | 120 | -648 | 0.5 | 0.1 |
| P67 | OCU | J64 | J88 | 735 | 16 | 120 | 3,344 | 5.3 | 5.0 |
| P69 | OCU | J78 | J66 | 151 | 16 | 120 | 710 | 1.1 | 0.1 |
| P31 | - | - | - | - | - | - | - | - | - |
| P25 | - | - | - | - | - | - | - | - | - |
| P19 | - | - | - | - | - | - | - | - | - |
| P183 | - | - | - | - | - | - | - | - | - |
| P173 | - | - | - | - | - | - | - | - | - |
| P61 | - | - | - | - | - | - | - | - | - |


| ID | Owner | From <br> Node | To <br> Node | Length <br> $(\mathrm{ft})$ | Diameter <br> $(\mathrm{in})$ | Roughness | Flow <br> (gpm) | Velocity <br> (fts) | Headloss <br> $(\mathrm{ft})$ |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| P143 | - | - | - | - | - | - | - | - | - |

Table B-8 Short-Term Scenario FC-2 Fire Flow Results - Junction Report

| ID | Demand (gpm) | Elevation ( ft ) | Head (ft) | Pressure (psi) |
| :---: | :---: | :---: | :---: | :---: |
| J98 | 0 | 111 | 201 | 39.0a |
| J96 | 0 | 111 | 202 | 39.5 |
| J32 | 0 | 111 | 204 | 40.3 |
| J94 | 634 | 120 | 221 | 43.9 |
| J86 | 0 | 116 | 222 | 45.8 |
| J88 | 0 | 114 | 222 | 46.8 |
| J78 | 0 | 113 | 221 | 46.9 |
| J64 | 0 | 114 | 223 | 47.2 |
| J18 | 0 | 114 | 227 | 49.0 |
| J118 | 0 | 114 | 227 | 49.1 |
| J120 | 0 | 114 | 227 | 49.1 |
| J56 | 0 | 107 | 222 | 49.8 |
| J66 | 710 | 106 | 221 | 49.9 |
| J34 | 2,648 | 104 | 220 | 50.1 |
| J22 | 0 | 105 | 221 | 50.3 |
| J54 | 432 | 109 | 227 | 51.0 |
| J20 | 0 | 102 | 221 | 51.6 |
| J116 | 0 | 111 | 231 | 51.8 |
| J100 | 0 | 111 | 231 | 52.0 |
| J110 | 0 | 111 | 234 | 53.4 |
| J52 | 0 | 102 | 227 | 54.2 |
| J16 | 392 | 98 | 227 | 56.1 |
| J114 | 0 | 98 | 227 | 56.1 |
| J38 | 94 | 96 | 227 | 56.9 |
| J46 | 0 | 95 | 272 | 76.8 |
| J50 | 0 | 95 | 281 | 80.7 |
| J10 | 0 | 105 | 300 | 84.6 |
| J40 | 0 | 97 | 297 | 86.8 |
| J30 | - | - | - | - |
| J36 | - | $\bullet$ | $\bullet$ | - |
| J24 | - | - | - | - |
| J112 | - | - | - | - |

## a. Denotes critical node during fire flow simulation

Table B-9 Short-Term Scenario FC-2 Fire Flow Results - Reservoir Report

| ID | Description | Flow (gpm) | Head (ft) |
| :--- | :--- | :--- | :--- |
| RES9002 | HGL provided by RCID | $-4,910$ | 304 |

Table B-10 Short-Term Scenario FC-2 Fire Flow Results - Pipe Report

| ID | Owner | From Node | To Node | Length (ft) | Diamete r (In) | Roughness | Flow (gpm) | Velocity (ft/s) | Headloss <br> (f) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P11 | RCID | J10 | J40 | 212 | 16 | 120 | 4,910 | 7.8 | 2.9 |
| P117 | OCU | J86 | J78 | 990 | 16 | 120 | 710 | 1.1 | 0.4 |
| P121 | OCU | J88 | J86 | 367 | 16 | 120 | 1,344 | 2.1 | 0.5 |
| P13 | Private | J114 | J16 | 376 | 16 | 120 | 392 | 0.6 | 0.1 |
| P133 | Private | J86 | J94 | 184 | 12 | 120 | 634 | 1.8 | 0.2 |
| P137 | RCID | J96 | J18 | 271 | 16 | 120 | 0 | 0.0 | 0.0 |
| P149 | RCID | J96 | J98 | 102 | 16 | 120 | 4,424 | 7.1 | 1.2 |
| P15 | RCID | J18 | $J 52$ | 1,225 | 12 | 120 | 162 | 0.5 | 0.1 |
| P155 | RCID | J100 | J116 | 50 | 16 | 120 | 4,424 | 7.1 | 0.6 |
| P159 | RCID | J110 | J100 | 33 | 16 | 120 | 4,424 | 7.1 | 3.1 |
| P17 | OCU | J20 | J22 | 2,631 | 24 | 120 | 0 | 0.0 | 0.0 |
| P177 | RCID | J114 | J32 | 2,071 | 16 | 120 | 4,424 | 7.1 | 23.5 |
| P189 | RCID | J116 | J120 | 285 | 16 | 120 | 4,424 | 7.1 | 3.2 |
| P191 | RCID | J98 | U7014 | 57 | 16 | 120 | 4,424 | 7.1 | 0.7 |
| P193 | RCID | U7014 | J110 | 45 | 16 | 120 | 4,424 | 7.1 | 0.5 |
| P195 | RCID | J118 | J18 | 408 | 16 | 120 | 432 | 0.7 | 0.1 |
| P197 | RCID | J120 | J64 | 459. | 16 | 120 | 3,992 | 6.4 | 4.3 |
| P199 | RCID | J120 | J118 | 109 | 12 | 120 | 432 | 1.2 | 0.1 |
| P23 | RCID | J18 | J54 | 1,461 | 12 | 120 | 270 | 0.8 | 0.4 |
| P29 | Private | J20 | J34 | 329 | 16 | 120 | 2,648 | 4.2 | 1.5 |
| P35 | Private | J114 | J38 | 231 | 12 | 120 | 94 | 0.3 | 0.0 |
| P37 | RCID | RES9002 | J10 | 291 | 16 | 120 | 4,910 | 7.8 | 4.0 |
| P39 | RCID | J40 | J50 | 1,171 | 16 | 120 | 4,910 | 7.8 | 16.2 |
| P43 | RCID | J46 | J114 | 3,242 | 16 | 120 | 4,910 | 7.8 | 44.7 |
| P47 | RCID | J50 | J46 | 662 | 16 | 120 | 4.910 | 7.8 | 9.1 |
| P51 | RCID | J32 | J96 | 155 | 16 | 120 | 4,424 | 7.1 | 1.8 |
| P57 | RCID | J52 | $J 54$ | 2,515 | 12 | 120 | 162 | 0.5 | 0.3 |
| P59 | OCU | J56 | J20 | 1,408 | 24 | 120 | 2,648 | 1.9 | 0.9 |
| P63 | OCU | J56 | J64 | 1,903 | 24 | 120 | -2,648 | 1.9 | 1.2 |
| P67 | OCU | J64 | J88 | 735 | 16 | 120 | 1,344 | 2.1 | 0.9 |
| P69 | OCU | J78 | J66 | 151 | 16 | 120 | 710 | 1.1 | 0.1 |
| P31 | - | - | - | - | - | - | - | - | - |
| P25 | - | - | - | - | - | - | - | - | - |
| P19 | - | - | - | - | - | - | - | - |  |
| P183 | - | - | - | - | - | - | - | - | - |
| P173 | - | - | - | - | - | - | - | - | - |
| P143 | - | - | - | - | - | - | - | - | - |
| P61 | - | - |  | - | - | $\bigcirc$ | - | - | $\cdot$ |

[^1]Table B-11 Long-Term Scenario Orange Lake Fire Flow Results - Junction Report

| ID | Demand (gpm) | Elevation (ft) | Head (t) | Pressure (psi) |
| :---: | :---: | :---: | :---: | :---: |
| J94 | 634 | 120 | 230 | 47.6a |
| J78 | 0 | 113 | 225 | 48.7 |
| J86 | 0 | 116 | 230 | 49.4 |
| J88 | 0 | 114 | 233 | 51.4 |
| J66 | 2,710 | 106 | 225 | 51.5 |
| J64 | 0 | 114 | 237 | 53.5 |
| J56 | 0 | 107 | 239 | 57.3 |
| J34 | 648 | 104 | 240 | 59.1 |
| J20 | 0 | 102 | 241 | 60.1 |
| J30 | 212 | 105 | 244 | 60.2 |
| J36 | 228 | 105 | 244 | 60.2 |
| J22 | 0 | 105 | 244 | 60.2 |
| J24 | 0 | 102 | 246 | 62.4 |
| J112 | 0 | 103 | 247 | 62.4 |
| J118 | 0 | 114 | 302 | 81.4 |
| J120 | 0 | 114 | 302 | 81.4 |
| J18 | 0 | 114 | 302 | 81.4 |
| J116 | 0 | 111 | 302 | 82.7 |
| J110 | 0 | 111 | 302 | 82.7 |
| J100 | 0 | 111 | 302 | 82.7 |
| J 98 | 0 | 111 | 302 | 82.7 |
| J96 | 0 | 111 | 302 | 82.7 |
| J32 | 0 | 111 | 302 | 82.7 |
| J54 | 432 | 109 | 302 | 83.4 |
| J10 | 0 | 105 | 304 | 86.3 |
| J52 | 0 | 102 | 302 | 86.6 |
| J16 | 191 | 98 | 302 | 88.5 |
| J114 | 0 | 98 | 302 | 88.5 |
| J38 | 76 | 96 | 302 | 89.4 |
| J40 | 0 | 97 | 304 | 89.8 |
| J46 | 0 | 95 | 303 | 90.3 |
| J50 | 0 | 95 | 304 | 90.4 |

a. Denotes critical node during fire flow simulation

Table B-12 Long-Term Scenario Orange Lake Fire Flow Results - Reservoir Report

| ID | Flow (gpm) | Head (ft) | Comment |
| :---: | :---: | :---: | :--- |
| RES9002 | -698 | 304 | Source: RCID 108' <br> Elevation and pressure <br> 85 psi |
| RES9004 | $-4,432$ | 250 | Source: OCU 120' <br> elevation and 56 psi |

Table B-13 Long-Term Scenario Orange Lake Fire Flow Results - Pipe Report

| ID | Owner | From Node | To Node | Length (t) | Diameter (in) | Roughness | Flow (gpm) | Velocity (f/s) | Headioss (f) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P11 | RCID | J10 | J40 | 212 | 16 | 120 | 698 | 1.1 | 0.1 |
| P117 | OCU | J86 | J78 | 990 | 16 | 120 | 2,710 | 4.3 | 4.5 |
| P121 | OCU | J88 | J86 | 367 | 16 | 120 | 3,344 | 5.3 | 2.5 |
| P13 | Private | J114 | J16 | 376 | 16 | 120 | 191 | 0.3 | 0.0 |
| P133 | Private | J86 | J94 | 184 | 12 | 120 | 634 | 1.8 | 0.2 |
| P137 | RCID | J96 | J18 | 271 | 16 | 120 | 432 | 0.7 | 0.0 |
| P149 | RCID | J96 | J98 | 102 | 16 | 120 | 0 | 0.0 | 0.0 |
| P15 | RCID | J18 | J52 | 1,225 | 12 | 120 | 162 | 0.5 | 0.1 |
| P155 | RCID | J100 | J116 | 50 | 16 | 120 | 0 | 0.0 | 0.0 |
| P159 | RCID | J110 | J100 | 33 | 16 | 120 | 0 | 0.0 | 0.0 |
| P17 | OCU | J20 | J22 | 2,631 | 24 | 120 | -3,992 | 2.8 | 3.4 |
| P173 | OCU | $J 112$ | RES9004 | 1,922 | 24 | 120 | -4,432 | 3.14 | 3.04 |
| P177 | RCID | J114 | J32 | 2,071 | 16 | 120 | 432 | 0.7 | 0.3 |
| P183 | OCU | J24 | J112 | 655 | 24 | 120 | -4,432 | 3.1 | 1.0 |
| P189 | RCID | J116 | J120 | 285 | 16 | 120 | 0 | 0.0 | 0.0 |
| P19 | OCU | J22 | J24 | 1,193 | 24 | 120 | -4,432 | 3.1 | 1.9 |
| P191 | RCD | J98 | U7014 | 57 | 16 | 120 | 0 | 0.0 | 0.0 |
| P193 | RCID | U7014 | J110 | 45 | 16 | 120 | 0 | 0.0 | 0.0 |
| P195 | RCID | J118 | J18 | 408 | 16 | 120 | 0 | 0.0 | 0.0 |
| P197 | RCID | J120 | J64 | 459 | 16 | 120 | 0 | 0.0 | 0.0 |
| P199 | RCID | J120 | J118 | 109 | 12 | 120 | 0 | 0.0 | 0.0 |
| P23 | RCID | J18 | J54 | 1,461 | 12 | 120 | 270 | 0.8 | 0.4 |
| P25 | Private | J22 | J30 | 524 | 16 | 120 | 212 | 0.3 | 0.0 |
| P29 | Private | J20 | J34 | 329 | 16 | 120 | 648 | 1.0 | 0.1 |
| P31 | Private | J22 | J36 | 344 | 16 | 120 | 228 | 0.4 | 0.0 |
| P35 | Private | J114 | J38 | 231 | 12 | 120 | 76 | 0.2 | 0.0 |
| P37 | RCID | RES9002 | J10 | 291 | 16 | 120 | 698 | 1.1 | 0.1 |
| P39 | RCID | J40 | J50 | 1,171 | 16 | 120 | 698 | 1.1 | 0.4 |
| P43 | RCID | J46 | J114 | 3,242 | 16 | 120 | 698 | 1.1 | 1.2 |
| P47 | RCID | J50 | J46 | 662 | 16 | 120 | 698 | 1.1 | 0.3 |
| P51 | RCID | J32 | J96 | 155 | 16 | 120 | 432 | 0.7 | 0.0 |


| ID | Owner | From Node | To Node | Length (t) | Diameter (in) | Roughness | Flow (gpm) | Velocity (fls) | Headloss (t) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P57 | RCID | J52 | J54 | 2,515 | 12 | 120 | 162 | 0.5 | 0.3 |
| P59 | OCU | J56 | J20 | 1,408 | 24 | 120 | -3,344 | 2.4 | 1.3 |
| P63 | OCU | J56 | J64 | 1,903 | 24 | 120 | 3,344 | 2.4 | $1: 8$ |
| P67 | OCU | $J 64$ | J88 | 735 | 16 | 120 | 3,344 | 5.3 | 5.0 |
| P69 | OCU | J78 | J66 | 151 | 16 | 120 | 2,710 | 4.3 | 0.7 |

Table B-14 Long-Term Scenario FC-1 Fire Flow Results - Junction Report

| ID | Demand (g pm) | Elevation (ft) | Head (ft) | Pressure (psi) |
| :---: | :---: | :---: | :---: | :---: |
| J94 | 2,634 | 120 | 227 | 46.3a |
| J86 | 0 | 116 | 230 | 49.4 |
| J78 | 0 | 113 | 230 | 50.6 |
| J88 | 0 | 114 | 233 | 51.4 |
| J64 | 0 | 114 | 237 | 53.5 |
| J66 | 710 | 106 | 230 | 53.6 |
| J56 | 0 | 107 | 239 | 57.3 |
| J34 | 648 . | 104 | 240 | 59.1 |
| J20 | 0 | 102 | 241 | 60.1 |
| J30 | 212 | 105 | 244 | 60.2 |
| J36 | 228 | 105 | 244 | 60.2 |
| J22 | 0 | 105 | 244 | 60.2 |
| J24 | 0 | 102 | 246 | 62.4 |
| J112 | 0 | 103 | 247 | 62.4 |
| J118 | 0 | 114 | 302 | 81.4 |
| J120 | 0 | 114 | 302 | 81.4 |
| J18 | 0 | 114 | 302 | 81.4 |
| J116 | 0 | 111 | 302 | 82.7 |
| J110 | 0 | 111 | 302 | 82.7 |
| J100 | 0 | 111 | 302 | 82.7 |
| J98 | 0 | 111 | 302 | 82.7 |
| J96 | 0 | 111 | 302 | 82.7 |
| J32 | 0 | 111 | 302 | 82.7 |
| J54 | 432 | 109 | 302 | 83.4 |
| J10 | 0 | 105 | 304 | 86.3 |
| J52 | 0 | 102 | 302 | 86.6 |
| J16 | 191 | 98 | 302 | 88.5 |
| J114 | 0 | 98 | 302 | 88.5 |
| J38 | 76 | 96 | 302 | 89.4 |
| J40 | 0 | 97 | 304 | 89.8 |


| ID | Demand $(\mathrm{gpm})$ | Elevation $(\mathrm{ft})$ | Head $(\mathrm{ft})$ | Pressure $(\mathrm{psi})$ |
| :--- | :--- | :--- | :--- | :--- |
| J 46 | 0 | 95 | 303 | 90.3 |
| J 50 | 0 | 95 | 304 | 90.4 |

a. Denotes critical node during fire flow simulation

Table B-15 Long-Term Scenario FC-1 Fire Flow Results - Reservoir Report

| ID |  | Flow (gpm) | Head (ft) |
| :--- | :--- | :--- | :--- |
| RES9002 | 108' Elevation and <br> pressure 85 psi | -698 | 304 |
| RES9004 | 120' elevation and 56 psi | $-4,432$ | 250 |

Table B-16 Long-Term Scenario FC-1 Fire Flow Results - Pipe Report

| ID | Owner | From <br> Node | To Node | Length (f) | Diameter <br> (In) | Roughness | Flow (gpm) | Velocity <br> (flts) | Headloss <br> (ft) |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| P11 | RCID | J10 | J40 | 212 | 16 | 120 | 698 | 1.1 | 0.1 |
| P117 | OCU | J86 | J78 | 990 | 16 | 120 | 710 | 1.1 | 0.4 |
| P121 | OCU | J88 | J86 | 367 | 16 | 120 | 3,344 | 5.3 | 2.5 |
| P13 | Private | J114 | J16 | 376 | 16 | 120 | 191 | 0.3 | 0.0 |
| P133 | Private | J86 | J94 | 184 | 12 | 120 | 2,634 | 7.5 | 3.3 |
| P137 | RCID | J96 | J18 | 271 | 16 | 120 | 432 | 0.7 | 0.0 |
| P149 | RCID | J96 | J98 | 102 | 16 | 120 | 0 | 0.0 | 0.0 |
| P15 | RCID | J18 | J52 | 1,225 | 12 | 120 | 162 | 0.5 | 0.1 |
| P155 | RCID | J100 | J116 | 50 | 16 | 120 | 0 | 0.0 | 0.0 |
| P159 | RCID | J110 | J100 | 33 | 16 | 120 | 0 | 0.0 | 0.0 |
| P17 | OCU | J20 | J22 | 2,631 | 24 | 120 | $-3,992$ | 2.8 | 3.4 |
| P173 | OCU | J112 | RES9004 | 1,922 | 24 | 120 | $-4,432$ | 3.1 | 3.0 |
| P177 | RCID | J114 | J32 | 2,071 | 16 | 120 | 432 | 0.7 | 0.3 |
| P183 | OCU | J24 | J112 | 655 | 24 | 120 | $-4,432$ | 3.1 | 1.0 |
| P189 | RCID | J116 | J120 | 285 | 16 | 120 | 0 | 0.0 | 0.0 |
| P19 | OCU | J22 | J24 | 1,193 | 24 | 120 | $-4,432$ | 3.1 | 1.9 |
| P191 | RCD | J98 | U7014 | 57 | 16 | 120 | 0 | 0.0 | 0.0 |
| P193 | RCID | U7014 | J110 | 45 | 16 | 120 | 0 | 0.0 | 0.0 |
| P195 | RCID | J118 | J18 | 408 | 16 | 120 | 0 | 0.0 | 0.0 |
| P197 | RCID | J120 | J64 | 459 | 16 | 120 | 0 | 0.0 | 0.0 |
| P199 | RCID | J120 | J118 | 109 | 12 | 120 | 0 | 0.0 | 0.0 |
| P23 | RCID | J18 | J54 | 1,461 | 12 | 120 | 270 | 0.8 | 0.4 |
| P25 | Private | J22 | J30 | 524 | 16 | 120 | 212 | 0.3 | 0.0 |
| P29 | Private | J20 | J34 | 329 | 16 | 120 | 648 | 1.0 | 0.1 |
| P31 | Private | J22 | J36 | 344 | 16 | 120 | 228 | 0.4 | 0.0 |
| P35 | Private | J114 | J38 | 231 | 12 | 120 | 76 | 0.2 | 0.0 |
|  |  |  |  |  |  |  |  |  |  |

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| ID | Owner | From Node | To Node | Length (ft) | Diameter (in) | Roughness | Flow (gpm) | Velocity (fts) | Headloss <br> (ft) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P37 | RCID | RES9002 | J10 | 291 | 16 | 120 | 698 | 1.1 | 0.1 |
| P39 | RCID | J40 | J50 | 1,171 | 16 | 120 | 698 | 1.1 | 0.4 |
| P43 | RCID | J46 | J114 | 3,242 | 16 | 120 | 698 | 1.1 | 1.2 |
| P47 | RCID | J50 | J46 | 662 | 16 | 120 | 698 | 1.1 | 0.3 |
| P51 | RCID | J32 | J96 | 155 | 16 | 120 | 432 | 0.7 | 0.0 |
| P57 | RCID | J52 | J54 | 2,515 | 12 | 120 | 162 | 0.5 | 0.3 |
| P59 | OCU | J56 | J20 | 1,408 | 24 | 120 | -3,344 | 2.4 | 1.3 |
| P63 | OCU | J56 | J64 | 1,903 | 24 | 120 | 3,344 | 2.4 | 1.8 |
| P67 | OCU | J64 | J88 | 735 | 16 | 120 | 3,344 | 5.3 | 5.0 |
| P69 | OCU | J78 | J66 | 159 | 16 | 120 | 710 | 1.1 | 0.1 |

Table B-17 Long-Term Scenario FC-2 Fire Flow Results - Junction Report

| ID | Demand (gpm) | Elevation (ft) | Head (ft) | Pressure (psi) |
| :---: | :---: | :---: | :---: | :---: |
| J94 | 634 | 120 | 238 | 51.3a |
| J86 | 0 | 116 | 239 | 53.1 |
| J88 | 0 | 114 | 239 | 54.2 |
| J78 | 0 | 113 | 238 | 54.3 |
| J64 | 0 | 114 | 240 | 54.6 |
| J66 | 710 | 106 | 238 | 57.3 |
| J56 | 0 | 107 | 240 | 57.8 |
| J34 | 2,648 | 104 | 239 | 58.6 |
| J20 | 0 | 102 | 241 | 60.1 |
| J30 | 212 | 105 | 244 | 60.2 |
| J36 | 228 | 105 | 244 | 60.2 |
| J22 | 0 | 105 | 244 | 60.2 |
| J24 | 0 | 102 | 246 | 62.4 |
| J112 | 0 | 103 | 247 | 62.4 |
| J118 | 0 | 114 | 302 | 81.4 |
| J120 | 0 | 114 | 302 | 81.4 |
| J18 | 0 | 114 | 302 | 81.4 |
| J116 | 0 | 111 | 302 | 82.7 |
| J110 | 0 | 111 | 302 | 82.7 |
| J100 | 0 | 111 | 302 | 82.7 |
| J98 | 0 | 111 | 302 | 82.7 |
| J96 | 0 | 111 | 302 | 82.7 |
| J32 | 0 | 111 | 302 | 82.7 |
| J54 | 432 | 109 | 302 | 83.4 |
| J10 | 0 | 105 | 304 | 86.3 |
| J52 | 0 | 102 | 302 | 86.6 |

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| ID | Demand (gpm) | Elevation (ft) | Head (ft) | Pressure (psi) |
| :--- | :--- | :--- | :--- | :--- |
| J16 | 191 | 98 | 302 | 88.5 |
| J114 | 0 | 98 | 302 | 88.5 |
| J38 | 76 | 96 | 302 | 89.4 |
| J40 | 0 | 97 | 304 | 89.8 |
| J46 | 0 | 95 | 303 | 90.3 |
| J50 | 0 | 95 | 304 | 90.4 |

a. Denotes critical node during fire flow simulation

Table B-18 Long-Term Scenario FC-2 Fire Flow Results - Reservoir Report

| ID |  | Flow (gpm) | Head (ft) |
| :--- | :--- | :--- | :--- |
| RES9002 | $108^{\prime}$ Elevation and <br> pressure 85 psi | -698 | 304 |
| RES9004 | $120^{\prime}$ elevation and 56 psi | $-4,432$ | 250 |

Table B-19 Long-Term Scenario FC-2 Fire Flow Results - Pipe Report

| ID | Owner | From Node | To Node | Length <br> (ft) | Diameter (in) | Roughness | Flow (gpm) | Velocity (ftls) | Headloss <br> (ft) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P11 | RCID | J10 | J40 | 212 | 16 | 120 | 698 | 1.1 | 0.1 |
| P117 | OCU | J86 | J78 | 990 | 16 | 120 | 710 | 1.1 | 0.4 |
| P121 | OCU | J88 | J86 | 367 | 16 | 120 | 1,344 | 2.1 | 0.5 |
| P13 | Private | J114 | J16 | 376 | 16 | 120 | 191 | 0.3 | 0.0 |
| P133 | Private | J86 | J94 | 184 | 12 | 120 | 634 | 1.8 | 0.2 |
| P137 | RCID | J96 | J18 | 271 | 16 | 120 | 432 | 0.7 | 0.0 |
| P149 | RCID | J96 | J98 | 102 | 16 | 120 | 0 | 0.0 | 0.0 |
| P15 | RCID | J18 | J52 | 1,225 | 12 | 120 | 162 | 0.5 | 0.1 |
| P155 | RCID | J100 | J116 | 50 | 16 | 120 | 0 | 0.0 | 0.0 |
| P159 | RCID | J110 | J100 | 33 | 16 | 120 | 0 | 0.0 | 0.0 |
| P17 | OCU | J20 | J22 | 2,631 | 24 | 120 | -3,992 | 2.8 | 3.4 |
| P173 | OCU | J112 | RES9004 | 1,922 | 24 | 120 | -4,432 | 3.1 | 3.0 |
| P177 | RCID | J114 | J32 | 2,074 | 16 | 120 | 432 | 0.7 | 0.3 |
| P183 | OCU | J24 | J112 | 655 | 24 | 120 | -4,432 | 3.1 | 1.0 |
| P189 | RCID | J116 | J120 | 285 | 16 | 120 | 0 | 0.0 | 0.0 |
| P19 | OCU | J22 | J24 | 1,193 | 24 | 120 | -4,432 | 3.1 | 1.9 |
| P191 | RCD | J98 | U7014 | 57 | 16 | 120 | 0 | 0.0 | 0.0 |
| P193 | RCID | U7014 | J110 | 45 | 16 | 120 | 0 | 0.0 | 0.0 |
| P195 | RCID | J1.18 | J18 | 408 | 16 | 120 | 0 | 0.0 | 0.0 |
| P197 | RCID | J120 | J64 | 459 | 16 | 120 | 0 | 0.0 | 0.0 |
| P199 | RCID | J120 | J118 | 109 | 12 | 120 | 0 | 0.0 | 0.0 |
| P23 | RCID | J18 | J54 | 1,461 | 12 | 120 | 270 | 0.8 | 0.4 |
| P25 | Private | J22 | J30 | 524 | 16 | 120 | 212 | 0.3 | 0.0 |
| P29 | Private | J20 | J34 | 329 | 16 | 120 | 2,648 | 4.2 | 1.5 |
| P31 | Private | J22 | J36 | 344 | 16 | 120 | 228 | 0.4 | 0.0 |
| P35 | Private | J114 | J38 | 231 | 12 | 120 | 76 | 0.2 | 0.0 |
| P37 | RCID | RES9002 | J10 | 291 | 16 | 120 | 698 | 1.1 | 0.1 |
| P39 | RCID | J40 | J50 | 1,471 | 16 | 120 | 698 | 1.1 | 0.4 |
| P43 | RCID | J46 | J114 | 3,242 | 16 | 120 | 698 | 1.1 | 1.2 |
| P47 | RCID | J50 | J46 | 662 | 16 | 120 | 698 | 1.1 | 0.3 |
| P51 | RCID | J32 | J96 | 155 | 16 | 120 | 432 | 0.7 | 0.0 |
| P57 | RCID | J52 | J54 | 2.515 | 12 | 120 | 162 | 0.5 | 0.3 |
| P59 | OCU | J56 | J20 | 1,408 | 24 | 120 | -1,344 | 1.0 | 0.2 |
| P63 | OCU | J56 | J64 | 1,903 | 24 | 120 | 1,344 | 1.0 | 0.3 |
| P67 | OCU | J64 | J88 | 735 | 16 | 120 | 1,344 | 2.1 | 0.9 |
| P69 | OCU | J78 | J66 | 151.17 | 16 | 120 | 710 | 1.13 | 0.1 |

Table B-20 Long-Term Scenario BI-N Fire Flow Results - Junction Report

| ID | Demand (gpm) | Elevation (ft) | Head (ft) | Pressure (psi) |
| :---: | :---: | :---: | :---: | :---: |
| J94 | 634 | 120 | 241 | 52.4a |
| J86 | 0 | 116 | 241 | 54.2 |
| J88 | 0 | 114 | 242 | 55.3 |
| J78 | 0 | 113 | 241 | 55.4 |
| J64 | 0 | 114 | 243 | 55.7 |
| J66 | 710 | 106 | 241 | 58.4 |
| J56 | 0 | 107 | 243 | 58.9 |
| J30 | 2,212 | 105 | 242 | 59.5 |
| J34 | 648 | 104 | 243 | 60.2 |
| J36 | 228 | 105 | 244 | 60.2 |
| J22 | $0^{\prime}$ | 105 | 244 | 60.2 |
| J20 | 0 | 102 | 243 | 61.1 |
| J24 | 0 | 102 | 246 | 62.4 |
| J112 | 0 | 103 | 247 | 62.4 |
| J118 | 0 | 114 | 302 | 81.4 |
| J120 | 0 | 114 | 302 | 81.4 |
| J18 | 0 | 114 | 302 | 81.4 |
| J116 | 0 | 111 | 302 | 82.7 |
| J110 | 0 | 111 | 302 | 82.7 |
| J100 | 0 | 111 | 302 | 82.7 |
| J96 | 0 | 111 | 302 | 82.7 |
| J98 | 0 | 111 | 302 | 82.7 |
| J32 | 0 | 111 | 302 | 82.7 |
| J54 | 432 | 109 | 302 | 83.4 |
| J10 | 0 | 105 | 304 | 86.3 |
| J52 | 0 | 102 | 302 | 86.6 |
| J16 | 191 | 98 | 302 | 88.5 |
| $J 114$ | 0 | 98 | 302 | 88.5 |
| J38 | 76 | 96 | 302 | 89.4 |
| J40 | 0 | 97 | 304 | 89.8 |
| J46 | 0 | 95 | 303 | 90.3 |
| J50 | 0 | 95 | 304 | 90.4 |

a. Denotes critical node during fire flow simulation

Table B-21 Long-Term Scenario BI-N Fire Flow Results - Reservoir Report

| ID |  | Flow (gpm) | Head (ft) |
| :--- | :--- | :--- | :--- |
| RES9002 | $108^{\prime}$ Elevation and <br> pressure 85 psi | -698 | 304 |
| RES9004 | $120^{\prime}$ elevation and 56 psi | $-4,432$ | 250 |

Table B-22 Long-Term Scenario BI-N Fire Flow Results - Pipe Report

| ID | Owner | From Node | To Node | Length (ft) | Diameter (in) | Roughness | Flow (gpm) | Velocity (fts) | Headloss <br> (ft) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P11 | RCID | J10 | J40 | 212 | 16 | 120 | 698 | 1.1 | 0.1 |
| P117 | OCU | J86 | J78 | 990 | 16 | 120 | 710 | 1.1 | 0.4 |
| P121 | OCU | J88 | J86 | 367 | 16 | 120 | 1,344 | 2.1 | 0.5 |
| P13 | Private | J114 | J16 | 376 | 16 | 120 | 191 | 0.3 | 0.0 |
| P133 | Private | J86 | J94 | 184 | 12 | 120 | 634 | 1.8 | 0.2 |
| P137 | RCID | J96 | J18 | 271 | 16 | 120 | 432 | 0.7 | 0.0 |
| P149 | RCID | J96 | J98 | 102 | 16 | 120 | 0 | 0.0 | 0.0 |
| P15 | RCID | J18 | J52 | 1,225 | 12 | 120 | 162 | 0.5 | 0.1 |
| P155 | RCID | J100 | J116 | 50 | 16 | 120 | 0 | 0.0 | 0.0 |
| P159 | RCID | $J 110$ | $J 100$ | 33 | 16 | 120 | 0 | 0.0 | 0.0 |
| P17 | OCU | J20 | J22 | 2,631 | 24 | 120 | -1,992 | 1.4 | 1.0 |
| P173 | OCU | J112 | RES9004 | 1,922 | 24 | 120 | -4,432 | 3.1 | 3.0 |
| P177 | RCID | $J 114$ | J32 | 2,071 | 16 | 120 | 432 | 0.7 | 0.3 |
| P183 | OCU | J24 | J112 | 655 | 24 | 120 | -4,432 | 3.1 | 1.0 |
| P189 | RCID | J116 | J120 | 285 | 16 | 120 | 0 | 0.0 | 0.0 |
| P19 | OCU | J22 | J24 | 1,193 | 24 | 120 | -4,432 | 3.1 | 1.9 |
| P191 | RCD | J98 | U7014 | 57 | 16 | 120 | 0 | 0.0 | 0.0 |
| P193 | RCID | U7014 | J110 | 45 | 16 | 120 | 0 | 0.0 | 0.0 |
| P195 | RCID | J118 | J18 | 408 | 16 | 120 | 0 | 0.0 | 0.0 |
| P197 | RCID | J120 | J64 | 459 | 16 | 120 | 0 | 0.0 | 0.0 |
| P199 | RCID | J120 | J118 | 109 | 12 | 120 | 0 | 0.0 | 0.0 |
| P23 | RCID | J18 | J54 | 1,461 | 12 | 120 | 270 | 0.8 | 0.4 |
| P25 | Private | J22 | J30 | 524 | 16 | 120 | 2,212 | 3.5 | 1.7 |
| P29 | Private | J20 | J34 | 329 | 16 | 120 | 648 | 1.0 | 0.1 |
| P31 | Private | J22 | J36 | 344 | 16 | 120 | 228 | 0.4 | 0.0 |
| P35 | Private | J114 | J38 | 231 | 12 | 120 | 76 | 0.2 | 0.0 |
| P37 | RCID | RES9002 | J10 | 291 | 16 | 120 | 698 | 1.1 | 0.1 |
| P39 | RCID | J40 | J50 | 1,171 | 16 | 120 | 698 | 1.1 | 0.4 |
| P43 | RCID | J46 | J114 | 3,242 | 16 | 120 | 698 | 1.1 | 1.2 |
| P47 | RCID | J50 | J46 | 662 | 16 | 120 | 698 | 1.1 | 0.3 |


| ID | Owner | From <br> Node | To Node | Length <br> $(\mathrm{ft})$ | Diameter <br> $(\mathrm{in})$ | Roughness | Flow <br> $(\mathrm{gpm})$ | Velocity <br> $(\mathrm{ft} / \mathrm{s})$ | Headloss <br> $(\mathrm{ft})$ |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| P51 | RCID | J 32 | J 96 | 155 | 16 | 120 | 432 | 0.7 | 0.0 |
| P57 | RCID | J 52 | J 54 | 2,515 | 12 | 120 | 162 | 0.5 | 0.3 |
| P59 | OCU | J 56 | J 20 | 1,408 | 24 | 120 | $-1,344$ | 1.0 | 0.2 |
| P63 | OCU | J 56 | J 64 | 1,903 | 24 | 120 | 1,344 | 1.0 | 0.3 |
| P67 | OCU | $\mathrm{J64}$ | J 88 | 735 | 16 | 120 | 1,344 | 2.1 | 0.9 |
| P69 | OCU | $\mathrm{J78}$ | J 66 | 151.17 | 16 | 120 | 710 | 1.13 | 0.06 |

Table B-23 Long-Term Scenario BI-S Fire Flow Results - Junction Report

| ID | Demand (gpm) | Elevation (ft) | Head ( $\dagger$ ) | Pressure (psi) |
| :---: | :---: | :---: | :---: | :---: |
| J94 | 634 | 120 | 241 | 52.4a |
| J86 | 0 | 116 | 241 | 54.2 |
| J88 | 0 | 114 | 242 | 55.3 |
| J78 | 0 | 113 | 241 | 55.4 |
| J64 | 0 | 114 | 243 | 55.7 |
| J66 | 710 | 106 | 241 | 58.4 |
| J56 | 0 | 107 | 243 | 58.9 |
| J36 | 2,228 | 105 | 243 | 59.8 |
| J34 | 648 | 104 | 243 | 60.2 |
| J30 | 212 | 105 | 244 | 60.2 |
| J22 | 0 | 105 | 244 | 60.2 |
| J20 | 0 | 102 | 243 | 61.1 |
| J24 | 0 | 102 | 246 | 62.4 |
| J112 | 0 | 103 | 247 | 62.4 |
| J118 | 0 | 114 | 302 | 81.4 |
| J120 | 0 | 114 | 302 | 81.4 |
| J18 | 0 | 114 | 302 | 81.4 |
| J116 | 0 | 111 | 302 | 82.7 |
| J100 | 0 | 111 | 302 | 82.7 |
| J110 | 0 | 111 | 302 | 82.7 |
| J98 | 0 | 111 | 302 | 82.7 |
| J96 | 0 | 111 | 302 | 82.7 |
| J32 | 0 | 111 | 302 | 82.7 |
| J54 | 432 | 109 | 302 | 83.4 |
| J10 | 0 | 105 | 304 | 86.3 |
| J52 | 0 | 102 | 302 | 86.6 |
| J16 | 191 | 98 | 302 | 88.5 |
| J114 | 0 | 98 | 302 | 88.5 |
| J38 | 76 | 96 | 302 | 89.4 |
| J40 | 0 | 97 | 304 | 89.8 |


| ID | Demand (gpm) | Elevation $(\mathrm{ft})$ | Head $(\mathrm{ft})$ | Pressure (psi) |
| :--- | :--- | :--- | :--- | :--- |
| J 46 | 0 | 95 | 303 | 90.3 |
| J50 | 0 | 95 | 304 | 90.4 |

a. Denotes critical node during fire flow simulation

Table B-24 Long-Term Scenario Bl-S Fire Flow Results - Pipe Report
$\left.\begin{array}{|l|l|l|l|l|l|l|l|l|l|}\hline \text { ID } & \text { Owner } & \begin{array}{l}\text { From } \\ \text { Node }\end{array} & \text { To Node } & \begin{array}{l}\text { Length } \\ \text { (ft) }\end{array} & \begin{array}{l}\text { Diameter } \\ \text { (In) }\end{array} & \text { Roughness }\end{array} \begin{array}{l}\text { Flow } \\ \text { (gpm) }\end{array}\right)$

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| ID | Owner | From <br> Node | To Node | Length <br> (ft) | Diameter <br> (in) | Roughness | Flow <br> (gpm) | Velocity <br> (ft/s) | Headloss (ft) |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| P63 | OCU | J 56 | $\mathrm{J64}$ | 1,903 | 24 | 120 | 1,344 | 1.0 | 0.3 |
| P67 | OCU | J 64 | J 88 | 735 | 16 | 120 | 1,344 | 2.1 | 0.9 |
| P69 | OCU | J 78 | J 66 | 151.17 | 16 | 120 | 710.0 | 1.1 | 0.1 |

Table B-25 Long-Term Scenario BI-S Fire Flow Results - Reservoir Report

| ID |  | Flow (gpm) | Head (ft) |
| :--- | :--- | :--- | :--- |
| RES9002 | $108^{\prime}$ Elevation and <br> pressure 85 psi | -698 | 304 |
| RES9004 | $120^{\prime}$ elevation and <br> 56 psi | $-4,432$ | 250 |

Table B-26 Long-Term Fire Flow analysis

| ID | Static <br> Demand <br> $(\mathrm{gpm})$ | Static <br> Pressure <br> $(\mathrm{psi})$ | Static <br> Head <br> (ft) | Fire-Flow <br> Demand <br> $(\mathrm{gpm})$ | Resldual <br> Pressure <br> (psi) | Avallable <br> Flow at <br> Hydrant <br> (gpm) | Available <br> Flow <br> Pressure <br> $(\mathrm{psi})$ | Critical <br> Pipe ID | Critical <br> Pipe <br> Velocity <br> (fts) |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| J 30212 | 62.0 | 248 | 2,000 | 59.5 | 5013 | 53.2 | P25 | 8.0 |  |
| J 34648 | 62.0 | 247 | 2,000 | 58.6 | 5013 | 52.0 | P29 | 8.0 |  |
| J 36 | 228 | 62.0 | 248 | 2,000 | 59.8 | 5013 | 54.3 | P31 | 8.0 |
| $\mathrm{J66} 710$ | 60.1 | 245 | 2,000 | 51.5 | 4,920 | 35.0 | P69 | 7.9 |  |
| J 94634 | 54.1 | 245 | 2,000 | 46.3 | 2,820 | 45.3 | P133 | 8.0 |  |

Table B-27 Short-Term Scenario Peak Hour Flow Junction Report

| [ | Damand (gpm) | Elevation (ii) | Haad (ti) | Pressure (asi) |
| :---: | :---: | :---: | :---: | :---: |
| J98 | 0 | 111 | 191 | 34.6a |
| J96 | 0 | 111 | 192 | 35.1 |
| J32 | 0 | 111 | 194 | 36.0 |
| J94 | 1268 | 120 | 205 | 36.6 |
| J86 | 0 | 116 | 205 | 38.7 |
| J78 | 0 | 113 | 204 | 39.4 |
| J88 | 0 | 114 | 207 | 40.3 |
| J64 | 0 | 114 | 210 | 41.7 |
| J66 | 1420 | 106 | 204 | 42.4 |
| J18 | 0 | 114 | 214 | 43.5 |
| J118 | 0 | 114 | 214 | 43.5 |
| J120 | 0 | 114 | 215 | 43.6 |
| J56 | 0 | 107 | 210 | 44.6 |
| J54 | 720 | 109 | 213 | 45.2 |
| J22 | 0 | 105 | 210 | 45.4 |
| J34 | 1296 | 104 | 209 | 45.7 |
| J116 | 0 | 111 | 218 | 46.5 |
| J20 | 0 | 102 | 210 | 46.7 |
| J100 | 0 | 111 | 219 | 46.8 |
| J110 | 0 | 111 | 222 | 48.3 |
| J52 | 0 | 102 | 214 | 48.5 |
| J16 | 318 | 98 | 220 | 53.0 |
| J114 | 0 | 98 | 220 | 53.0 |
| J38 | 126 | 96 | 220 | 53.9 |
| J46 | 0 | 95 | 269 | 75.5 |
| J50 | 0 | 95 | 279 | 79.8 |
| J10 | 0 | 105 | 300 | 84.5 |
| J40 | 0 | 97 | 297 | 86.6 |
| J30 | - | - | - | - |
| J36 | - | - | - | - |
| J24 | - | - | - | - |
| J112 | - | - | - | - |

Table B-28 Short-Term Peak Hour Reservoir Report

| ID | Flow (gpm) | Head (ft) | Description |
| :---: | ---: | :---: | :--- |
| RES9002 | $-5,148$ | 304 | HGL Provided by RCID |

Table B-29 Short-Term Scenario Peak Hour Pipe Report
$\left.\begin{array}{|l|l|l|l|l|l|l|l|l|l|}\hline \text { ID } & \text { Owner } & \begin{array}{l}\text { From } \\ \text { Node }\end{array} & \begin{array}{l}\text { To } \\ \text { Node }\end{array} & \begin{array}{l}\text { Length } \\ \text { (ft) }\end{array} & \begin{array}{l}\text { Diameter } \\ \text { (in) }\end{array} & \text { Roughness } & \text { Flow (gpm) } \\ \text { Velocity } \\ \text { (ft/s) }\end{array}\right\}$

Table B-30 Long-Term Scenario Peak Hour Flow Junction Report

| ID | Demand (gpm) | Elevation (ft) | Head (ft) | Pressure (psi) |
| :---: | :---: | :---: | :---: | :---: |
| J94 | 1,268 | 120 | 232 | 48.4a |
| J86 | 0 | 116 | 232 | 50.5 |
| J78 | 0 | 113 | 231 | 51.2 |
| J88 | 0 | 114 | 234 | 52.0 |
| J64 | 0 | 114 | 237 | 53.5 |
| J66 | 1,420 | 106 | 231 | 54.1 |
| J56 | 0 | 107 | 239 | 57.0 |
| J34 | 1,296 | 104 | 239 | 58.5 |
| J20 | 0 | 102 | 239 | 59.6 |
| J30 | 424 | 105 | 243 | 59.7 |
| J36 | 456 | 105 | 243 | 59.7 |
| J22 | 0 | 105 | 243 | 59.8 |
| J24 | 0 | 102 | 245 | 62.0 |
| J112 | 0 | 103 | 246 | 62.1 |
| J118 | 0 | 114 | 298 | 79.7 |
| J120 | 0 | 114 | 298 | 79.7 |
| J18 | 0 | 114 | 298 | 79.7 |
| J116 | 0 | 111 | 298 | 81.0 |
| J100 | 0 | 111 | 298 | 81.0 |
| J110 | 0 | 111 | 298 | 81.0 |
| J98 | 0 | 111 | 298 | 81.1 |
| J96 | 0 | 111 | 298 | 81.1 |
| J32 | 0 | 111 | 298 | 81.1 |
| J54 | 720 | 109 | 297 | 81.5 |
| J52 | 0 | 102 | 298 | 84.8 |
| J10 | 0 | 105 | 304 | 86.3 |
| J16 | 318 | 98 | 299 | 87.1 |
| J114 | 0 | 98 | 299 | 87.1 |
| J38 | 126 | 96 | 299 | 88.0 |
| J40 | 0 | 97 | 304 | 89.6 |
| J46 | 0 | 95 | 302 | 89.7 |
| J50 | 0 | 95 | 303 | 90.0 |

a. Denotes critical node during fire flow simulation

Table B-31 Long-Term Peak Hour Reservoir Report

| ID | Flow (g.pm) | Head (ft) | Comment |
| :---: | :---: | :---: | :---: |
| RES9002 | -1,164 | 304 | Source: RCID 108' Elevation and pressure 85 psi |
| RES9004 | -4,864 | 250 | Source: OCU $120^{\prime}$ elevation and 56 psi |

Table B-32 Long-Term Scenario Peak Hour Pipe Report

| ID | Owner | From Node | To Node | Length (ft) | Dlameter (in) | Roughness | Flow (gpm) | Velocity (ft's) | Headloss (ft) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P11 | RCID | J10 | J40 | 212 | 16 | 120 | 1164 | 1.9 | 0.2 |
| P117 | OCU | J86 | J78 | 990 | 16 | 120 | 1420 | 2.3 | 1.4 |
| P121 | OCU | J88 | J86 | 367 | 16 | 120 | 2688 | 4.3 | 1.7 |
| P13 | Private | J114 | J16 | 376 | 16 | 120 | 318 | 0.5 | 0.0 |
| P133 | Private | J86 | J94 | 184 | 12 | 120 | 1268 | 3.6 | 0.8 |
| P137 | RCID | J96 | J18 | 271 | 16 | 120 | 720 | 1.2 | 0.1 |
| P149 | RCID | J96 | J98 | 102 | 16 | 120 | 0 | 0.0 | 0.0 |
| P15 | RCID | J18 | J52 | 1225 | 12 | 120 | 271 | 0.8 | 0.3 |
| P155 | RCID | J100 | J116 | 50 | 16 | 120 | 0 | 0.0 | 0.0 |
| P159 | RCID | J110 | J100 | 33 | 16 | 120 | 0 | 0.0 | 0.0 |
| P17 | OCU | J20 | J22 | 2631 | 24 | 120 | -3984 | 2.8 | 3.4 |
| P173 | OCU | J112 | RES9004 | 1922 | 24 | 120 | -4864 | 3.45 | 3.61 |
| P177 | RCID | J114 | J32 | 2071 | 16 | 120 | 720 | 1.2 | 0.8 |
| P183 | OCU | J24 | J112 | 655 | 24 | 120 | -4864 | 3.5 | 1.2 |
| P189 | RCID | J116 | J120 | 285 | 16 | 120 | 0 | 0.0 | 0.0 |
| P19 | OCU | J22 | J24 | 1193 | 24 | 120 | -4864 | 3.5 | 2.2 |
| P191 | RCD | J98 | U7014 | 57 | 16 | 120 | 0 | 0.0 | 0.0 |
| P193 | RCID | U7014 | J110 | 45 | 16 | 120 | 0 | 0.0 | 0.0 |
| P195 | RCID | J118 | J18 | 408 | 16 | 120 | 0 | 0.0 | 0.0 |
| P197 | RCID | J120 | J64 | 459 | 16 | 120 | 0 | 0.0 | 0.0 |
| P199 | RCID | J120 | J118 | 109 | 12 | 120 | 0 | 0.0 | 0.0 |
| P23 | RCID | J18 | J54 | 1461 | 12 | 120 | 449 | 1.3 | 1.0 |
| P25 | Private | J22 | J30 | 524 | 16 | 120 | 424 | 0.7 | 0.1 |
| P29 | Private | J20 | J34 | 329 | 16 | 120 | 1296 | 2.1 | 0.4 |
| P31 | Private | J22 | J36 | 344 | 16 | 120 | 456 | 0.7 | 0.1 |
| P35 | Private | J114 | J38 | 231 | 12 | 120 | 126 | 0.4 | 0.0 |
| P37 | RCID | RES9002 | J10 | 291 | 16 | 120 | 1164 | 1.9 | 0.3 |
| P39 | RCID | J40 | J50 | 1171 | 16 | 120 | 1164 | 1.9 | 1.1 |
| P43 | RCID | J46 | J114 | 3242 | 16 | 120 | 1164 | 1.9 | 3.1 |
| P47 | RCID | J50 | J46 | 662 | 16 | 120 | 1164 | 1.9 | 0.6 |
| P51 | RCID | J32 | J96 | 155 | 16 | 120 | 720 | 1.2 | 0.1 |


| ID | Owner | From <br> Node | To Node | Length <br> (ft) | Diameter <br> (in) | Roughness | Flow <br> (gpm) | Velocity <br> (fts) | Headloss <br> (ft) |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| P57 | RCID | J52 | J54 | 2515 | 12 | 120 | 271 | 0.8 | 0.7 |
| P59 | OCU | J56 | J20 | 1408 | 24 | 120 | -2688 | 1.91 | 0.88 |
| P63 | OCU | J56 | J64 | 1903 | 24 | 120 | 2688 | 1.91 | 1.19 |
| P67 | OCU | J64 | J88 | 735 | 16 | 120 | 2688 | 4.29 | 3.32 |
| P69 | OCU | J78 | J66 | 151.17 | 16 | 120 | 1,420 | 2.27 | 0.21 |

Table B-33 Short-Term Orange Lake Fire Flow Interim Construction Scenario - Junction Report

| ID | Demand (g pm) | Elevation (ft) | Head (ft) | Pressure (psi) |
| :---: | :---: | :---: | :---: | :---: |
| J94 | 0 | 120 | 237 | 50.6a |
| J78 | 0 | 113 | 232 | 51.7 |
| J86 | 0 | 116 | 237 | 52.4 |
| J88 | 0 | 114 | 239 | 54.0 |
| J66 | 2710 | 106 | 232 | 54.4 |
| J64 | 0 | 114 | 242 | 55.4 |
| J118 | 0 | 114 | 243 | 56.1 |
| J18 | 0 | 114 | 245 | 56.9 |
| J54 | 432 | 109 | 245 | 58.9 |
| J96 | 0 | 111 | 247 | 58.9 |
| J32 | 0 | 111 | 248 | 59.3 |
| J122 | 0 | 107 | 245 | 59.9 |
| J52 | 0 | 102 | 245 | 62.1 |
| J16 | 392 | 98 | 260 | 70.4 |
| J114 | 0 | 98 | 260 | 70.4 |
| J38 | 94 | 96 | 260 | 71.2 |
| J46 | 0 | 95 | 286 | 82.7 |
| J50 | 0 | 95 | 291 | 85.0 |
| J10 | 0 | 105 | 302 | 85.4 |
| J40 | 0 | 97 | 300 | 88.1 |
| J34 | - | - | - | - |
| J20 | - | - | - | - |
| J56 | - | - | - | - |
| J120 | - | - | - | - |
| J116 | - | - | - | - |
| J100 | - | - | - | - |
| J110 | - | - | - | - |



Table B-34 Short Term Orange Lake Fire Flow Interim Construction Scenario- Pipe Report

| ID | Owner | From Node | To Node | Length (ft) | Diameter (in) | Roughness | Flow (gpm) | Velocity (ft/s) | Headloss (iti) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P11 | RCID | J10 | J40 | 212 | 16 | 120 | 3628 | 5.8 | 1.7 |
| P117 | OCU | J86 | J78 | 990 | 16 | 120 | 2710 | 4.3 | 4.5 |
| P121 | OCU | J88 | J86 | 367 | 16 | 120 | 2710 | 4.3 | 1.7 |
| P13 | Private | J114 | J16 | 376 | 16 | 120 | 392 | 0.6 | 0.1 |
| P133 | Private | J86 | J94 | 184 | 12 | 120 | 0 | 0.0 | 0.0 |
| P137 | RCID | J96 | J18 | 271 | 16 | 120 | 3142 | 5.0 | 1.6 |
| P15 | RCID | J18 | J52 | 1225 | 12 | 120 | 162 | 0.5 | 0.1 |
| P177 | RCID | J114 | J32 | 2071 | 16 | 120 | 3142 | 5.0 | 12.5 |
| P195 | RCID | J118 | J18 | 408 | 16 | 120 | -2710 | 4.3 | 1.9 |
| P201 | RCID | J52 | J122 | 462 | 12 | 100 | 0 | 0.0 | 0.0 |
| P23 | RCID | J18 | J54 | 1461 | 12 | 120 | 270 | 0.8 | 0.4 |
| P35 | Private | J114 | J38 | 231 | 12 | 120 | 94 | 0.3 | 0.0 |
| P37 | RCID | RES9002 | J10 | 291 | 16 | 120 | 3628 | 5.8 | 2.3 |
| P39 | RCID | J40 | J50 | 1171 | 16 | 120 | 3628 | 5.8 | 9.2 |
| P43 | RCID | J46 | J114 | 3242 | 16 | 120 | 3628 | 5.8 | 25.5 |
| P47 | RCID | J50 | J46 | 662 | 16 | 120 | 3628 | 5.8 | 5.2 |
| P51 | RCID | J32 | J96 | 155 | 16 | 120 | 3142 | 5.0 | 0.9 |
| P57 | RCID | J52 | J54 | 2515 | 12 | 120 | 162 | 0.5 | 0.3 |
| P61 | RCID | J64 | J118 | 341 | 16 | 120 | -2710 | 4.3 | 1.6 |
| P67 | OCU | J64 | J88 | 735 | 16 | 120 | 2710 | 4.3 | 3.4 |
| P69 | OCU | J78 | J66 | 151 | 16 | 120 | 2710 | 4.3 | 0.7 |
| P63 | - | - | - | - | - | - | - | - | - |
| P59 | - | - | - | - | - | - | - | - | - |
| P29 | - | - | - | - | - | - | - | - | - |
| P143 | - | - | - | - | $\bullet$ | - | - | - | - |
| P149 | - | - | - | - | - | - | - | - | - |
| P155 | - | - | - | - | - | - | - | - | - |
| P159 | - | - | - | - | - | - | - | - | - |
| P189 | - | - | - | - | - | - | - | - | - |


| ID | Owner | From <br> Node | To <br> Node | Length <br> (ft) | Diameter <br> (in) | Roughness | Flow (gpm) | Velocity <br> (fts) | Headloss <br> (ft) |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| P191 | - | - | - | - | - | - | - | - | - |
| P193 | - | - | - | - | - | - | - | - | - |
| P197 | - | - | - | - | - | - | - | - | - |
| P199 | - | - | - | - | - | - | - | - | - |
| P173 | - | - | - | - | - | - | - | - | - |
| P183 | - | - | - | - | - | - | - | - |  |
| P19 | - | - | - | - | - | - | - | - | - |

Table B-35 Short Term Orange Lake Fire Flow Interim Construction scenario- Reservoir Report

| ID | Flow (gpm) | Head (ft) | Comment |
| :---: | :---: | :---: | :--- |
| RES9002 |  | $-3,628$ | 304 |

Table B-36 Short Term FC-1 Fire Flow Interim Construction Scenario- Junction Report

| ID | Demand <br> (gpm) | Elavation <br> (ft) | Head (fit) | Pressure <br> (psi) |
| :--- | :--- | :--- | :--- | :--- |
| J94 | 2000 | 120 | 235 | 49.8 a |
| J86 | 0 | 116 | 237 | 52.4 |
| J78 | 0 | 113 | 236 | 53.5 |
| J88 | 0 | 114 | 239 | 54.0 |
| J64 | 0 | 114 | 242 | 55.4 |
| J118 | 0 | 114 | 243 | 56.1 |
| J66 | 710 | 106 | 236 | 56.5 |
| J18 | 0 | 114 | 245 | 56.9 |
| J54 | 432 | 109 | 245 | 58.9 |
| J96 | 0 | 111 | 247 | 58.9 |
| J32 | 0 | 111 | 248 | 59.3 |
| J122 | 0 | 107 | 245 | 59.9 |
| J52 | 0 | 102 | 245 | 62.1 |
| J16 | 392 | 98 | 260 | 70.4 |
| J114 | 0 | 98 | 260 | 70.4 |
| J38 | 94 | 96 | 260 | 71.2 |
| J46 | 0 | 95 | 286 | 82.7 |
| J50 | 0 | 95 | 291 | 85.0 |
|  |  |  |  |  |


| ID | Demand <br> (gpm) | Elevallon <br> (ft) | Head (fi) | Pressure <br> (psi) |
| :--- | :--- | :--- | :--- | :--- |
| J10 | 0 | 105 | 302 | 85.4 |
| J40 | 0 | 97 | 300 | 88.1 |
| J34 | - | - | - | - |
| J20 | - | - | - | - |
| J56 | - | - | - | - |
| J120 | - | - | - | - |
| J116 | - | - | - | - |
| J100 | - | - | - | - |
| J110 | - | - | - | - |
| J98 | - | - | - | - |
| J36 | - | - | - | - |
| J30 | - | - | - | - |
| J24 | - | - | - | - |
| J112 | - | - | - |  |

Denotes critical node during fire flow simulation

Table B-37 Short Term FC-1 Fire Flow Interim Construction Scenario- pipe report

| 10 | Owner | $\begin{aligned} & \text { From } \\ & \text { Node } \end{aligned}$ | Ta Node | Length (ti) | Diameter (in) | Roughneas | $\begin{aligned} & \text { Flow } \\ & \text { (gpm) } \end{aligned}$ | Volocliy (ft's) | Heudlass <br> (ft) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P11 | RCID | J10 | J40 | 211.62 | 16 | 120 | 3628 | 6 | 1.7 |
| P117 | OCU | J86 | J78 | 990.15 | 16 | 120 | 710 | 1 | 0.4 |
| P121 | OCU | J88 | J86 | 366.9 | 16 | 120 | 2710 | 4 | 1.7 |
| P13 | Private | J114 | J16 | 376.06 | 16 | 120 | 392 | 1 | 0.1 |
| P133 | Private | J86 | J94 | 184 | 12 | 120 | 2000 | 6 | 2.0 |
| P137 | RCID | J96 | J18 | 270.9 | 16 | 120 | 3142 | 5 | 1.6 |
| P15 | RCID | J18 | J52 | $1,224.9$ | 12 | 120 | 162 | 0 | 0.1 |
| P177 | RCID | J114 | J32 | $\begin{gathered} 2,070.5 \\ 9 \end{gathered}$ | 16 | 120 | 3142 | 5 | 12.5 |
| P195 | RCID | J118 | $J 18$ | 408.38 | 16 | 120 | -2710 | 4 | 1.9 |
| P201 | RCID | J52 | J122 | 461.98 | 12 | 100 | 0 | 0 | 0.0 |
| P23 | RCID | J18 | J54 | $\begin{gathered} \hline 1,461.0 \\ 8 \end{gathered}$ | 12 | 120 | 270 | 1 | 0.4 |
| P35 | Private | J114 | J38 | 231.49 | 12 | 120 | 94 | 0 | 0.0 |
| P37 | RCID | $\begin{gathered} \text { RES9 } \\ 002 \end{gathered}$ | J10 | 290.9 | 16 | 120 | 3628 | 6 | 2.3 |
| P39 | RCID | J40 | J50 | $\begin{gathered} \hline 1,171.4 \\ 8 \end{gathered}$ | 16 | 120 | 3628 | 6 | 9.2 |
| P43 | RCID | J46 | J114 | $\begin{gathered} 3,242.2 \\ 5 \end{gathered}$ | 16 | 120 | 3628 | 6 | 25.5 |
| P47 | RCID | J50 | J46 | 661.55 | 16 | 120 | 3628 | 6 | 5.2 |


| ID | Owner | From <br> Node | To Node | Length (ft) | Diameter (in) | Roughness | Flow (gpm) | Velocity (ft/s) | Headloss <br> (ft) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P51 | RCID | J32 | J96 | 155.03 | 16 | 120 | 3142 | 5 | 0.9 |
| P57 | RCID | J52 | J54 | $\begin{gathered} 2,514.6 \\ 8 \\ \hline \end{gathered}$ | 12 | 120 | 162 | 0 | 0.3 |
| P61 | RCID | J64 | J118 | 341.28 | 16 | 120 | -2710 | 4 | 1.6 |
| P67 | OCU | J64 | J88 | 734.64 | 16 | 120 | 2710 | 4 | 3.4 |
| P69 | OCU | J78 | J66 | 151.17 | 16 | 120 | 710 | 1 | 0.1 |
| P63 | - | - | - | - | - | - | - | - | - |
| P59 | - | - | - | - | - | - | - | - | - |
| P29 | - | - | - | - | - | - | - | - | - |
| P143 | - | - | - | - | - | - | - | - | - |
| P149 | - | - | - | - | - | - | - | - | - |
| P155 | - | - | - | - | - | - | - | - | - |
| P159 | - | - | - | - | - | - | - | - | - |
| P189 | - | - | - | - | - | - | - | - | - |
| P191 | - | - | - | - | - | - | - | - | - |
| P193 | - | - | - | - | - | - | - | - | - |
| P197 | - | - | - | - | - | - | $\bullet$ | - | - |
| P199 | - | - | - | - | - | - | - | - | - |
| P173 | - | - | - | - | - | - | - | - | - |
| P183 | - | - | - | - | - | - | - | - | - |
| P19 | - | - | - | - | - | - | - | - | - |
| P31 | - | - | - | - | - | - | - | - | - |
| P25 | - | - | - | - | - | - | - | - | - |

Table B-38 Short Term FC-1 Fire Flow Interim Construction Scenario- Reservoir report

| ID | Description | Flow (gpm) | Head (ft) |
| :---: | :---: | :---: | :---: |
| RES9002 | HGL provided by RCID | $-3,628$ | 304 |

Table B-39 Short term FC-2 Fire Flow Interim Construction Scenario- Junction report

| ID | Demand (gpm) | Elevation (ft) | Head (fi) | Pressure (psi) |
| :---: | :---: | :---: | :---: | :---: |
| J 122 | 2000 | 107 | 232 | 54.0 a |
| J 94 | 0 | 120 | 245 | 54.0 |
| J 86 | 0 | 116 | 245 | 55.8 |
| J 88 | 0 | 114 | 245 | 56.7 |
| J 64 | 0 | 114 | 245 | 56.8 |
| J 118 | 0 | 114 | 245 | 56.9 |
| J 78 | 0 | 113 | 244 | 56.9 |


| 11 | Demand (gnm) | Elevation (fi) | Head (ti) | Pressure (psi) |
| :---: | :---: | :---: | :---: | :---: |
| J18 | 0 | 114 | 245 | 56.9 |
| J54 | 432 | 109 | 241 | 57.2 |
| J96 | 0 | 111 | 247 | 58.9 |
| J52 | 0 | 102 | 238 | 59.1 |
| J32 | 0 | 111 | 248 | 59.3 |
| J66 | 710 1 | 106 | 244 | 59.9 |
| J16 | 392 | 98 | 260 | 70.4 |
| J114 | 0 | 98 | 260 | 70.4 |
| J38 | 94 | 96 | 260 | 71.2 |
| J46 | 0 | 95 | 286 | 82.7 |
| J50 | 0 | 95 | 291 | 85.0 |
| J10 | 0 | 105 | 302 | 85.4 |
| J40 | 0 | 97 | 300 | 88.1 |
| J34 | - | - | - | - |
| J20 | - | - | - | - |
| J56 | - | - | - | - |
| J120 | - | - | - | - |
| J116 | - | - | - | - |
| J100 | - | - | - | - |
| J110 | - | - | - | - |
| J98 | - | - | - | - |
| J36 | - | - | - | - |
| J30 | - | - | - | - |
| J24 | - | - | - | - |
| J112 | - | - | - | - |

Table B-40 Short term FC-2 Fire Flow Interim Construction Scenario- Pipe Report

| ID | Owner | From Node | To Node | Lengt $h$ (ft) | Diameter (in) | Roughness | Flow (gym) | Velocity (tus) | Headlos s (ft) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P11 | RCID | J10 | J40 | 212 | 16 | 120 | 3628 | 6 | 1.7 |
| P117 | OCU | J86 | J78 | 990 | 16 | 120 | 710 | 1 | 0.4 |
| P121 | OCU | J88 | J86 | 367 | 16 | 120 | 710 | 1 | 0.1 |
| P13 | Private | J114 | J16 | 376 | 16 | 120 | 392 | 1 | 0.1 |
| P133 | Private | J86 | J94 | 184 | 12 | 120 | 0 | 0 | 0.0 |
| P137 | RCID | J96 | J18 | 271 | 16 | 120 | 3142 | 5 | 1.6 |
| P15 | RCID | J18 | J52 | 1225 | 12 | 120 | 1428 | 4 | 7.0 |
| P177 | RCID | J114 | J32 | 2071 | 16 | 120 | 3142 | 5 | 12.5 |
| P195 | RCID | $J 118$ | J18 | 408 | 16 | 120 | -710 | 1 | 0.2 |


| ID | Owner | From Node | To Node | Lengt <br> h (1t) | Diameter (in) | Roughness | $\begin{aligned} & \text { Flow } \\ & \text { (opmet } \end{aligned}$ | Velocity ( $\mathrm{H} / \mathrm{s}$ ) | Headlos s ( m |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P201 | RCID | J52 | J122 | 462 | 12 | 100 | 2000 | 6 | 6.9 |
| P23 | RCID | J18 | J54 | 1461 | 12 | 120 | 1004 | 3 | 4.3 |
| P35 | Private | J114 | J38 | 231 | 12 | 120 | 94 | 0 | 0.0 |
| P37 | RCID | $\begin{gathered} \text { RES900 } \\ 2 \end{gathered}$ | J10 | 291 | 16 | 120 | 3628 | 6 | 2.3 |
| P39 | RCID | J40 | J50 | 1171 | 16 | 120 | 3628 | 6 | 9.2 |
| P43 | RCID | J46 | J114 | 3242 | 16 | 120 | 3628 | 6 | 25.5 |
| P47 | RCID | J50 | J46 | 662 | 16 | 120 | 3628 | 6 | 5.2 |
| P51 | RCID | J32 | J96 | 155 | 16 | 120 | 3142 | 5 | 0.9 |
| P57 | RCID | J52 | J54 | 2515 | 12 | 120 | -572 | 2 | 2.6 |
| P61 | RCID | J64 | J118 | 341 | 16 | 120 | -710 | 1 | 0.1 |
| P67 | OCU | J64 | J88 | 735 | 16 | 120 | 710 | 1 | 0.3 |
| P69 | OCU | J78 | J66 | 151 | 16 | 120 | 710 | 1 | 0.1 |
| P63 | - | - | - | - | - | - | - | - | - |
| P59 | - | - | - | - | - | - | - | - | - |
| P29 | - | - | - | - | - | - | - | - | - |
| P143 | - | - | - | - | - | - | - | - | - |
| P149 | - | - | - | - | - | - | - | - | - |
| P155 | - | - | - | - | - | - | - | - | - |
| P159 | - | - | - | - | - | - | - | - | - |
| P189 | - | - | - | - | - | - | - | - | - |
| P191 | - | - | - | - | - | - | - | - | - |
| P193 | - | - | - | - | - | - | - | - | - |
| P197 | - | - | - | - | - | - | - | - | - |
| P199 | - | - | - | - | - | - | - | - | - |
| P173 | - | - | - | - | - | - | - | - | - |
| P183 | - | - | - | - | - | - | - | - | - |
| P19 | - | - | - | - | - | - | - | - | - |
| P31 | - | - | - | - | - | - | - | - | - |
| P25 | - | - | - | - | - | - | - | - | - |

Table B-41 Short term FC-2 Fire Flow Interim Construction Scenario - Reservoir Report

| ID | Description | Flow (gpm) | Head (位) |
| :---: | :---: | :---: | :---: |
|  |  |  |  |
| RES9002 | HGL provided by RCID | $-3,628$ | 304 |

## Appendix C. Wastewater

Figure C-1 Wastewater Pipe and Node Diagram


NOTE: Minor losses were not accounted for in the hydraulic models. These losses shall be accounted for in the lift station calculations at the time of the construction plan submittal.

Table C-1 Short-Term Scenario Peak Hour Flow Results High Head - Junction Report

| ID | Demand <br> $(\mathrm{gpm})$ | Elevation (ft) | Head (ft) | Pressure <br> $(\mathrm{psi})$ |
| :--- | :--- | :--- | :--- | :--- |
| J106 | 0 | 105 | 106 | 0.6 |
| J126 | 0 | 114 | 117 | 1.2 |
| $J 128$ | 0 | 114 | 117 | 1.4 |
| $J 34$ | -900 | 106 | 128 | 9.4 |
| J62 | 0 | 103 | 106 | 1.3 |
| $J 64$ | 0 | 105 | 106 | 0.4 |
| $J 72$ | 0 | 116 | 122 | 2.5 |
| $J 76$ | 0 | 114 | 120 | 2.6 |
| J82 | 0 | 101 | 117 | 6.8 |
| $J 86$ | 0 | 114 | 117 | 1.3 |
| $J 92$ | 0 | 102 | 103 | 0.6 |
| $J 98$ | 0 | 114 | 126 | 5.3 |
| $J 116$ | - | - | - | - |
| $J 114$ | - | - | - | - |
| $J 58$ | - | - | - | - |
| $J 108$ | - | - | - | - |

Table C-2 Short-Term Scenario Peak Hour Flow Results High Head - Reservoir Report

| ID | HGL Information | Flow (gpm) | Head (fi) |
| :--- | :--- | :--- | :--- |
| RES9032 | Source: LS Calcs FC-2 Pumps Off Water Elevation | -856 | 93 |
| RES9028 | Source: Proposed MH invert Elevation | 2,602 | 103 |
| RES9030 | Source: LS Calcs FC-1 Pumps Off Water Elevation | -846 | 96 |

Table C-3 Short-Term Scenario Peak Hour Flow Results High Head - Pipe Report

| ID | Owner | From Node | To Node | Length (ft) | Diameter (in) | Roughness | Flow (gpm) | Velocity (fits) | Headloss (ft) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P137 | OCU | J64 | J62 | 10 | 12 | 130 | 856 | 2.4 | 0.0 |
| P143 | OCU | J72 | J76 | 867 | 12 | 130 | 900 | 2.6 | 1.8 |
| P147 | OCU | J76 | J86 | 1391 | 12 | 130 | 900 | 2.6 | 2.9 |
| P185 | OCU | J62 | J92 | 1351 | 12 | 130 | 856 | 2.4 | 2.6 |
| P199 | OCU | J86 | J126 | 49 | 12 | 130 | 1746 | 5.0 | 0.4 |
| P205 | OCU | J92 | $J 126$ | 1878 | 12 | 130 | -1746 | 5.0 | 13.4 |
| P273 | RCID | J92 | RES9028 | 87 | 16 | 120 | 2602 | 4.2 | 0.4 |
| P279 | OCU | J128 | J86 | 23 | 12 | 130 | 846 | 2.4 | 0.0 |
| P91 | OCU | J34 | J72 | 2777 | 12 | 130 | 900 | 2.6 | 5.8 |
| P217 | Private | RES9030 | FC_1_PMP2 | 1 | 6 | 120 | 846 | 9.6 | 0.1 |
| P219 | Private | RES9030 | FC_1_PMP1 | 1 | 6 | 120 | 0 | 0.0 | 0.0 |
| P221 | Private | FC_1_PMP2 | J98 | 16 | 6 | 120 | 846 | 9.6 | 1.0 |
| P223 | Private | FC_1_PMP1 | J98 | 16 | 6 | 120 | 0 | 0.0 | 0.0 |
| P225 | Private | J98 | J128 | 678 | 8 | 130 | 846 | 5.4 | 9.1 |
| P231 | Private | RES9032 | FC_2_PMP1 | 1 | 6 | 120 | 856 | 9.7 | 0.1 |
| P233 | Private | RES9032 | FC_2_PMP2 | 1 | 6 | 120 | 0 | 0.0 | 0.0 |
| P235 | Private | FC_2_PMP2 | J106 | 14 | 6 | 120 | 0 | 0.0 | 0.0 |
| P237 | Private | FC_2_PMP1 | J106 | 14 | 6 | 120 | 856 | 9.7 | 0.9 |
| P239 | Private | J106 | J64 | 188 | 12 | 130 | 856 | 2.4 | 0.4 |
| P151 | RCID | J82 | J16 | 225 | 12 | 130 | 0 | 0.0 | 0.0 |
| P277 | RCID | J126 | J82 | 600 | 12 | 130 | 0 | 0.0 | 0.0 |
| 61 | - | - | - | - | - | - | - | - | - |
| 63 | - | - | - | - | - | - | - | - | - |
| 118 | - | - | - | - | - | - | - | - | - |
| -119 | - | - | - | - | - | - | - | - | - |
| 120 | - | - | - | - | - | - | - | - | - |
| 121 | - | - | - | - | - | - | - | - | - |
| 124 | - | - | - | - | - | - | - | - | - |
| 125 | - | - | - | - | - | - | - | - | - |
| 127 | - | - | - | - | - | - | - | - | - |
| 128 | - | - | - | - | - | - | - | - | - |
| 129 | - | - | - | - | - | - | - | - | - |
| 130 | - | - | - | - | - | - | - | - | - |
| 131 | - | - | - | - | - | - | - | - | - |

Table C-4 Short-Term Scenario Peak Hour Flow Results High Head - Pump Report

| ID | Elevation <br> (ft) | Upstream <br> Pressure <br> (psi) | Downstream <br> Pressure <br> (psi) | Flow (gpm) | Head Gain <br> (ft) | Status |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| FC_2_PMP1 | 92.82 | 0.6 | 6.9 | 856 | 14 | Open |
| FC_2_PMP2 | 92.82 | 0.7 | 6.5 | 0 | 0 | Closed |
| FC_1_PMP1 | 95.96 | 0.7 | 13.8 | 0 | 0 | Closed |
| FC_1_PMP2 | 95.96 | 0.6 | 14.2 | 846 | 31 | Open |
| BI_N_PMP1 | - | - | - | - | - | - |
| BI_N_PMP2 | - | - | - | - | - | - |
| BI_S_PMP1 | - | - | - | - | - | - |
| BI_S_PMP2 | - | - | - | - | - | - |

NOTE: the pumps for FC-1 and FC-2 are required to be changed prior to connecting to OCU om the long term scenario.

Pump FC_1_PMP2 at 00:00 hrs


Figure C-2 FC-1 High Head Pump Curve

Pump FC_2_PMP1 at 00:00 hrs


Figure C-3 FC-2 High Head Pump Curve

Table C-5 Long-Term Scenario Peak Hour Flow Results High Head - Junction Report

| ID | Demand <br> $(\mathrm{gpm)}$ | Elevation <br> (ft) | Head (ft) | Pressure <br> (psil) |
| :--- | :--- | :--- | :--- | :--- |
| J 58 | 0 | 101 | 189 | 38.0 |
| J 108 | 0 | 94 | 190 | 41.3 |
| J 114 | 0 | 91 | 189 | 42.7 |
| J 116 | 0 | 91 | 190 | 42.9 |
| J 62 | 0 | 103 | 202 | 42.8 |
| J 64 | 0 | 105 | 202 | 42.0 |
| J 92 | 0 | 102 | 212 | 47.6 |
| J 106 | 0 | 105 | 202 | 42.2 |
| J 82 | 0 | 101 | 226 | 54.1 |
| J 86 | 0 | 114 | 226 | 48.7 |
| J 98 | 0 | 114 | 236 | 53.0 |
| J 126 | 0 | 114 | 226 | 48.5 |
| J 128 | 0 | 114 | 226 | 48.7 |
| J 34 | -900 | 106 | 237 | 56.7 |
| J 72 | 0 | 116 | 231 | 49.8 |
| J 76 | 0 | 114 | 229 | 49.9 |

Table C-6 Long-Term Scenario Peak Hour Flow Results High Head- Reservoir Report

| ID | HGL Information | Flow <br> (gpm) | Head (ft) |
| :--- | :--- | :--- | :--- |
| RES9010 | Source: OCU Elevation 120' and 27 psi pressure | 3436 | 182 |
| RES9030 | Source: LS Calcs FC-1 Pumps Off Water Elevation | -891 | 96 |
| RES9032 | Source: LS Calcs FC-2 Pumps Off Water Elevation | -907 | 93 |
| RES9034 | Source: LS Calcs BI-N Pumps Off Water Elevation | -395 | 84 |
| RES9036 | Source: LS Calcs BI-S Pumps Off Water Elevation | -343 | 97 |

Table C-7 Long-Term Scenario Peak Hour Flow Results High Head - Pipe Report

| ID | Owner | From Node | To Node | Length (ft) | Diameter (iin) | Roughness | Flow (gpm) | Velocity (fts) | Headloss (ft) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P131 | OCU | J58 | RES9010 | 3,179 | 20 | 130 | 3436 | 3.5 | 6.6 |
| P135 | OCU | J62 | J58 | 3,381 | 16 | 130 | 2698 | 4.3 | 13.3 |
| P137 | OCU | J64 | J62 | 10 | 12 | 130 | 907 | 2.6 | 0.0 |
| P143 | OCU | J72 | J76 | 867 | 12 | 130 | 900 | 2.6 | 1.8 |
| P147 | OCU | J76 | J86 | 1,391 | 12 | 130 | 900 | 2.6 | 2.9 |
| P185 | OCU | J62 | J92 | 1,351 | 12 | 130 | -1791 | 5.0 | 10.1 |
| P199 | OCU | J86 | J126 | 49 | 12 | 130 | 1791 | 5.0 | 0.4 |
| P205 | OCU | J92 | J126 | 1,878 | 12 | 130 | -1791 | 5.0 | 14.0 |
| P279 | OCU | J128 | J86 | 23 | 12 | 130 | 891 | 2.5 | 0.1 |
| P91 | OCU | J34 | J72 | 2,777 | 12 | 130 | 900 | 2.6 | 5.8 |
| P217 | Private | RES9030 | FC_1_PMP2 | 1 | 6 | 120 | 891 | 10.1 | 0.1 |
| P219 | Private | RES9030 | FC_1_PMP1 | 1 | 6 | 120 | 0 | 0.0 | 0.0 |
| P221 | Private | FC_1_PMP2 | J98 | 16 | 6 | 120 | 891 | 10.1 | 1.1 |
| P223 | Private | FC_1_PMP1 | J98 | 16 | 6 | 120 | 0 | 0.0 | 0.0 |
| P225 | Private | J98 | J128 | 678 | 8 | 130 | 891 | 5.7 | 10.0 |
| P231 | Private | RES9032 | FC_2_PMP1 | 1 | 6 | 120 | 907 | 10.3 | 0.1 |
| P233 | Private | RES9032 | FC_2_PMP2 | 1 | 6 | 120 | 0 | 0.0 | 0.0 |
| P235 | Private | FC_2_PMP2 | J106 | 14 | 6 | 120 | 0 | 0.0 | 0.0 |
| P237 | Private | FC_2_PMP1 | J106 | 14 | 6 | 120 | 907 | 10.3 | 1.0 |
| P239 | Private | J106 | J64 | 188 | 12 | 130 | 907 | 2.6 | 0.4 |
| P241 | Private | Bl_S_PMP1 | J108 | 8 | 4 | 120 | 0 | 0.0 | 0.0 |
| P243 | Private | BI_S_PMP2 | J108 | 8 | 4 | 120 | 343 | 8.8 | 0.7 |
| P245 | Private | RES9036 | BI_S_PMP1 | 1 | 6 | 100 | 0 | 0.0 | 0.0 |
| P247 | Private | RES9036 | BI_S_PMP2 | 1 | 8 | 120 | 343 | 2.2 | 0.0 |
| P253 | Private | J108 | J58 | 104 | 6 | 130 | 343 | 3.9 | 1.1 |
| P255 | Private | J114 | J58 | 53 | 6 | 130 | 395 | 4.5 | 0.7 |
| P259 | Private | RES9034 | BI_N_PMP1 | 1 | 4 | 120 | 0 | 0.0 | 0.0 |
| P261 | Private | RES9034 | BI_N_PMP2 | 1 | 4 | 120 | 395 | 4.5 | 0.0 |
| P263 | Private | BI_N_PMP1 | J116 | 9 | 4 | 120 | 0 | 0.0 | 0.0 |
| P265 | Private | BI_N_PMP2 | J116 | 9 | 4 | 120 | 395 | 10.1 | 1.0 |
| P267 | Private | J116 | J114 | 34 | 6 | 130 | 395 | 4.5 | 0.5 |
| P277 | RCID | J126 | J82 | 600 | 12 | 130 | 0.00 | 0 | 0.0 |

Table C-8 Long-Term Scenario Peak Hour Flow Results High Head - Pump Report

| ID | Elevation <br> $(\mathrm{ft})$ | Upstream <br> Pressure <br> $(\mathrm{psi})$ | Downstream <br> Pressure <br> $(\mathrm{psi})$ | Flow <br> $(\mathrm{gpm})$ | Head <br> Gain <br> $(\mathrm{ft})$ | Status | Setting | Available <br> NPSH (ft) | Cavitation <br> Index |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| BI_N_PMP1 | 83.85 | 1.1 | 47.0 | 0 | 0 | Closed 0 | 0 | 0 |  |
| BI_N_PMP2 | 83.85 | 1.1 | 47.4 | 395 | 107 | Open 1 | 36 | 0 |  |
| BI_S_PMP1 | 96.56 | 0.9 | 41.2 | 0 | 0 | Closed 0 | 0 | 0 |  |
| BI_S_PMP2 | 96.56 | 0.9 | 41.5 | 343 | 94 | Open 1 | 35 | 0 |  |
| FC_1_PMP1 | 95.96 | 0.7 | 61.5 | 0 | 0 | Closed 0 | 0 | 0 |  |
| FC_1_PMP2 | 95.96 | 0.6 | 62.0 | 891 | 142 | Open 1 | 35 | 0 |  |
| FC_2_PMP1 | 92.82 | 0.6 | 48.5 | 907 | 111 | Open 1 | 35 | 0 |  |
| FC_2_PMP2 | 92.82 | 0.7 | 48.1 | 0 | 0 | Closed 0 | 0 | 0 |  |

NOTE: the pumps for FC- 4 and FC-2 are required to be changed prior to connecting to OCU in the long term scenario.

Pump FC_1_PMP2 at 00:00 hrs


Figure C-4 Long Term Scenario FC-1 High Head Pump Curve

Pump FC_2_PMP1 at 00:00 hrs


Figure C-6 Long Term Scenario FC-2 High Head Pump Curve
Pump BI_N_PMP2 at 00:00 hrs


Figure C-6 Long Term Scenario BI-N High Head Pump Curve

## Pump BI_S_PMP2 at 00:00 hrs



Figure C-7 Long Term Scenario Bl-S High Head Pump Curve

## Appendix D. Reclaimed Water



Table D-1 Short-Term Scenario Peak Hour Demand Results - Junction Report

| ID | Demand (gpm) | Elevation (ft) | Head (ft) | Pressure (psi) |
| :---: | :---: | :---: | :---: | :---: |
| J10 | 0 | 111 | 145 | 15 |
| J100 | 0 | 114 | 241 | 55 |
| J102 | 0 | 114 | 240 | 55 |
| J12 | 0 | 115 | 238 | 53 |
| J14 | 0 | 103 | 237 | 58 |
| J16 | 0 | 105 | 237 | 57 |
| J24 | 850 | 105 | 234 | 56 |
| J28 | 488 | 109 | 233 | 54 |
| J36 | 149 | 98 | 184 | 37 |
| J38 | 19 | 96 | 185 | 38 |
| J40 | 0 | 105 | 298 | 84 |
| J42 | 0 | 100 | 287 | 81 |
| J48 | 0 | 99 | 253 | 67 |
| J52 | 0 | 98 | 269 | 74 |
| J54 | 0 | 106 | 237 | 57 |
| J56 | 0 | 112 | 235 | 53 |
| J58 | 0 | 110 | 237 | 55 |
| J60 | 0 | 113 | 223 | 47 |
| J62 | 0 | 114 | 234 | 52 |
| J66 | 0 | 116 | 228 | 49 |
| J72 | 0 | 107 | 237 | 56 |
| J74 | 0 | 114 | 238 | 54 |
| J76 | 600 | 106 | 221 | 50 |
| J80 | 0 | 101 | 185 | 36 |
| J82 | 0 | 111 | 249 | 60 |
| J84 | 0 | 111 | 253 | 62 |
| J90 | 0 | 111 | 144 | 14 |
| J92 | 0 | 111 | 148 | 16 |
| J94 | 0 | 114 | 232 | 51 |
| J98 | 733 | 114 | 236 | 53 |

Table D-2 Short-Term Scenario Peak Hour Demand Results - Reservoir Report

| ID | Flow (gpm) | Head (ft) | Comment |
| :--- | :--- | :--- | :--- |
| RES9000 | Elevation 108' and <br> pressure of 83 psi | $-2,839$ | 299 |

Table D-3 Short-Term Scenario Peak Hour Demand Results - Pipe Report
$\left.\begin{array}{|l|l|l|l|l|l|l|l|l|l|}\hline \text { ID } & \text { Owner } & \text { From Node } & \text { To Node } \\ \text { Length } \\ \text { (f) }\end{array}\right]$

| ID | Owner | From Node | To Node | Length <br> (ft) | Diameter (in) | Roughness | Flow (gpm) | Velocity <br> $(\mathrm{ft} / \mathrm{s})$ | Headloss <br> $(\mathrm{ft})$ |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| P159 | - | - | - | - | - | - | - | - | - |

Table D-4 Long-Term Scenario Peak Hour Demand Results - Junction Report

| ID | Demand <br> (gpm) | Elevation (f) | Head (ft) | Pressure (psi) |
| :--- | :--- | :--- | :--- | :--- |
| J60 | 0 | 113 | 222 | 47 |
| J66 | 0 | 116 | 227 | 48 |
| J76 | 600 | 106 | 220 | 49 |
| J94 | 0 | 114 | 231 | 51 |
| J62 | 0 | 114 | 232 | 51 |
| J98 | 733 | 114 | 235 | 53 |
| J74 | 0 | 114 | 237 | 53 |
| J72 | 0 | 107 | 237 | 56 |
| J24 | 850 | 105 | 235 | 56 |
| J20 | 561 | 108 | 239 | 57 |
| J22 | 561 | 106 | 238 | 57 |
| J14 | 0 | 103 | 238 | 58 |
| J16 | 0 | 105 | 241 | 59 |
| J18 | 0 | 105 | 244 | 60 |
| J12 | 0 | 115 | 290 | 76 |
| J56 | 0 | 112 | 287 | 76 |
| J102 | 0 | 114 | 290 | 76 |
| J28 | 488 | 109 | 285 | 76 |
| J10 | 0 | 111 | 290 | 77 |
| J90 | 0 | 111 | 290 | 77 |
| J92 | 0 | 111 | 290 | 77 |
| J58 | 0 | 110 | 289 | 78 |
| J54 | 0 | 106 | 289 | 79 |
| J80 | 0 | 101 | 291 | 83 |
| J36 | 149 | 98 | 291 | 84 |
| J40 | 0 | 105 | 299 | 84 |
| J38 | 19 | 96 | 291 | 85 |
| J48 | 0 | 99 | 296 | 85 |
| J42 | 0 | 100 | 298 | 86 |
| J52 | 0 | 98 | 297 | 86 |
| J84 | 0 | 111 | 436 | 141 |
| J82 | 0 | 111 | 436 | 141 |
|  |  |  |  |  |

Table D-5 Long-Term Scenario Peak Hour Demand Results - Reservoir Report

| ID | Flow (gpm) | Head (f) | Cominent |
| :--- | :--- | :--- | :--- |
| RES9000 | -656 | 299 | Source: RCID Elevation 108' <br> and pressure of 83 psi |
| RES9002 | -3305 | 250 | Source: OCU Elevation 120' <br> and pressure of 56 psi |

Table D-6 Long-Term Scenario Peak Hour Demand Results - Pipe Report

| ID | Owner | From <br> Node | To Node | Length (ft) | Diameter <br> (in) | Roughne <br> ss | Flow <br> (gpm) | Velocity <br> (fts) | Headloss <br> (ft) |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| P11 | RCID | RES9000 | J40 | 53 | 12 | 120 | 656 | 2 | 0 |
| P119 | RCID | J80 | J92 | 2,022 | 12 | 120 | 488 | 1 | 2 |
| P127 | RCID | J84 | J82 | 54 | 12 | 120 | 0 | 0 | 0 |
| P13 | OCU | J18 | RES9002 | 2,897 | 20 | 120 | $-3,305$ | 3 | 6 |
| P135 | RCID | J10 | J90 | 83 | 12 | 120 | 0 | 0 | 0 |
| P139 | RCID | J12 | J58 | 819 | 8 | 130 | 122 | 1 | 0 |
| P143 | RCID | J92 | J10 | 135 | 12 | 120 | 488 | 1 | 0 |
| P145 | RCID | J90 | U7002 | 47 | 12 | 120 | 0 | 0 | 0 |
| P147 | RCID | U7002 | J84 | 43 | 12 | 120 | 0 | 0 | 0 |
| P149 | Private | J74 | J98 | 733 | 12 | 120 | 733 | 2 | 1 |
| P151 | OCU | J94 | J66 | 508 | 8 | 130 | 600 | 4 | 4 |
| P159 | RCID | J102 | J74 | 215 | 8 | 130 | 0 | 0 | 0 |
| P17 | OCU | J14 | J16 | 2,532 | 20 | 120 | $-2,183$ | 2 | 3 |
| P19 | OCU | J16 | J18 | 1,350 | 20 | 120 | $-3,305$ | 3 | 3 |
| P21 | Private | J16 | J20 | 281 | 8 | 130 | 561 | 4 | 2 |
| P23 | Private | J16 | J22 | 426 | 8 | 130 | 561 | 4 | 3 |
| P25 | Private | J24 | J14 | 193 | 8 | 130 | -850 | 5 | 3 |
| P29 | RCID | J28 | J12 | 1,522 | 8 | 130 | -366 | 2 | 4 |
| P37 | Private | J80 | J36 | 277 | 6 | 130 | 149 | 2 | 1 |
| P39 | Private | J80 | J38 | 175 | 6 | 130 | 19 | 0 | 0 |
| P43 | RCID | J42 | J52 | 928 | 12 | 120 | 656 | 2 | 1 |
| P47 | RCID | J12 | J10 | 261 | 12 | 130 | -488 | 1 | 0 |
| P49 | RCID | J48 | J80 | 3,359 | 12 | 120 | 656 | 2 | 5 |
| P53 | RCID | J52 | J48 | 783 | 12 | 120 | 656 | 2 | 1 |
| P59 | RCID | J54 | J56 | 1,448 | 6 | 130 | 122 | 1 | 2 |
| P61 | RCID | J56 | J28 | 1,116 | 6 | 130 | 122 | 1 | 2 |
| P63 | RCID | J58 | J54 | 326 | 8 | 130 | 122 | 1 | 0 |
| P71 | OCU | J62 | J94 | 245 | 8 | 130 | 600 | 4 | 2 |
| P73 | OCU | J62 | J74 | 577 | 8 | 130 | -600 | 4 | 4 |
| P75 | OCU | J66 | J60 | 784 | 8 | 130 | 600 | 4 | 6 |
| P79 | RCID | J12 | J102 | 433 | 8 | 130 | 0 | 0 | 0 |
|  |  |  |  |  |  |  |  |  |  |


| ID | Owner | From <br> Node | To Node | Length (ft) | Diameter <br> (in) | Roughne <br> ss | Flow <br> $(\mathrm{gpm})$ | Velocity <br> $(\mathrm{ft}$ s) | Headloss <br> (ft) |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| P81 | OCU | J72 | $\mathrm{J14}$ | 1,509 | 20 | 120 | $-1,333$ | 1 | 1 |
| P83 | OCU | J72 | J74 | 1,718 | 20 | 120 | 1,333 | 1 | 1 |
| P87 | OCU | J60 | J76 | 267 | 8 | 130 | 600 | 4 | 2 |
| P91 | RCID | J40 | J42 | 515 | 12 | 120 | 656 | 2 | 1 |
| P157 | - | - | - | - | - | - | - | - | - |
| P137 | - | - | - | - | - | - | - | - | - |
| P161 | - | - | - | - | - | - | - | - | - |
| P125 | - | - | - | - | - | - | - | - | - |

## Appendix E. Lift Station Calculations

## FC-1 Short-Term Lift Station Calculations

## Wet Well Design Worksheet

## Short-Term Flamingo East Parcel - FC1

Diameter =
Area =
Vol./ft =
Peak Flow In=
Actual Pump Rate $=$

8 ft
50.27 Sq ft
376.0 gal/ft 816 gpm 846 gpm
$\mathrm{V}=(\mathrm{QT}) / 4$
where,
Q = Design Flow Rate =
T = Assumed Cycle Time =
V = Volume =

| 846 | gpm |
| ---: | :--- |
| 2115.0 | minutes |
| gallons |  |

The height to which this volume will rise in the wet well is calculated by the following equation.
$\mathrm{H}=$ Volume/(Volume/foot of the wet well $)=$

| 5.63 feet |
| ---: |
| 6.00 |

Check Pump Run-Time
Actual Pump Flow Rate $=$
Run-time $=$ Volume/ flow rate $=$

| 846 | gpm |
| :---: | :--- |
| 2.50 | minutes |


| Finished Grade Elevation <br> 113.84 ft | Top of Wet Well | 114.09 ft | Grade to Top of Wet Well 0.25 feet |
| :---: | :---: | :---: | :---: |
|  |  |  |  |
|  |  | $\rightarrow$ | Total Wet Well Depth |
|  |  |  | 20.13 feet |
|  | 103.46 ft |  |  |
|  |  |  | 0.5 feet |
|  | Alarm | 102.96 ft |  |
|  |  |  | 0.5 feet |
|  | Lag Pump On | 102.46 ft |  |
|  |  |  | 0.5 feet |
|  | Lead Pump On | 101.96 ft |  |
|  |  |  | 6.00 feet |
|  | Both Pumps Off | 95.96 ft |  |
|  |  |  | 1.50 feet |
|  | Top of Grout | 94.46 ft |  |
|  |  |  | 0.5 feet |
|  | Bottom | 93.96 ft |  |

NP 3127 MT 3- Adaptive 438

Technical specification


Installation: P-Semi parmanent, Wet


Different pumps are used for FC-1 in the short-term and long-term scenarios


Note: Picture mioth not correspond to the current configuretion.

General
Patented sell cioening semi-open channel inpetar, ideal for pumping in maste witer epilications. Poasth lo be uporaded ith Guidepmin for even buttur clogging mesistance. Modupir based denion with high adaptation grade.

Impaller

| Impelmor materian | HerdHron ${ }^{\text {m }}$ |
| :---: | :---: |
| Olscharge Fiange Oiameter | 31516 lnch |
| Suction Flange biameter | $57 / 8$ men |
| Imperer ditincter | 202 mm |
| Number of blades | 2 |

Motor

| Motor | N3127.060 21-12-ANLW 10hp Standiard |
| :---: | :---: |
| Stator variant | 38 |
| Frequency | 00 Hz |
| Rated vollage | 400 V |
| Number of poles | 4 |
| Phases | 3- |
| Rated power | 10 hp |
| Rated curment | 13 A |
| Starting current | 79.8 A |
| Rated speed | 1735 rpm |
| Power İactor |  |
| 1/1 Loed | 0.85 |
| 3/4 Land | 0.81 |
| 1/2 Lond | 0.73 |
| Motor fflliciency |  |
| $1 / 1$ Loed | 83.4\% |
| 9/4 Loed | $83.9 \%$ |
| 1/2 Land | 82.2\% |

## Conilguration

# Different pumps are used for FC-1 in the short-term and long-term scenarios 

FITMET
NP 3127 MT 3~ Adaptive 438
Performance curve

| Pump |  | Motor |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Discharge Fange Diameter | $315 / 16$ inch | Motor* | N3127.060 21-12-4AL-W 10np | Powerfactor |  |
| Suction Range Diameter | 150 mm |  |  | 1/1 Load | 0.85 |
| Impeller diameter | 74\% | Stator variant | 38 | 3/4 Load | 0.81 |
| Number of blades | 2 | Frequency | 60 Hz | 1/2 Load | 0.73 |
|  |  | Rated voltage | 460 V |  |  |
|  |  | Number of poles | 4 | Motor efliciency |  |
|  |  | Phases | 3- | 1/1 Load | 83.4\% |
|  |  | Rated power | 10 hp | 3/4 Load | 83.9\% |
|  |  | Rated cument | 13 A | 1/2 Load | 82.2 \% |
|  |  | Starting cument Prated speed | 79.9 A 1735 mm |  |  |



## xylem

NP 3127 MT 3~ Adaptive 438

## Duty Analysis



## VFD Curve


xylem
NP 3127 MT 3~ Adaptive 438
VFD Analysis


| Frequency | Flow | Head | Shaft power | Flow | Heed | Shatit power | Hyd eft. | Specifle energy | NPSHT |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 60H2 | 86usgpm | 3074 | 94310 | gmusgnm | 3074 | 9435 | 71\% | 15.7 MMUSNG |  |
| 55 Hz | Thusgam | 2887 | 7260 | 7risinm. | 258\% | 7264 | 7\% | 136 WHOUSMG |  |
| 5012 | $79458 . p m$ | 23.31 | 5.469 | 794S9pm. | $213 n$ | 5469 | $71 \%$ | 114 MnUSMG |  |
| 45 Hz | 670 | 17.3 n | 3989 | 677 USgpm. | 17.3n | 3889 | 718 | 952MMUSMG |  |
| 40 Hz | spusgam | 136 ${ }^{\text {f }}$ | 27919 | 5\%usgpm | 1367 | 279 mp | 7\% | 796 WWHUSMG | 7.07\% |

NP 3127 MT 3~ Adaptive 438
Dimensional drawing






## NP 3127 MT 3~ Adaptive 438

Life cycle costs (LCC)

| Total lifetime | 15 | Inflation rate (rate of price increasesi) | $2 \%$ |
| :--- | ---: | :--- | ---: |
| Annual operating time | 5600 | Interest rate (for investment) | $3 \%$ |
| Energy cost per IMh | 0.00 USD |  |  |

Power input P1

Total costs


First year costs

0.00 USD Energy (1st year)
0.00 USD investment costs (1st year)
0.00 USD Installation \& commissioning (1st year)
0.00 USD Operating cost (1st year)
0.00 USD Maintenance \& repair (1st year)
0.00 USD Downtime (1st year)
0.00 USD Environmental (1st year)
$0 \%$
0.00 USD Decommissioning (1st year)
0.00

USD

Disclaimer. The calculations and the results are based on user input values and general assumptions and provide only estimated costs for the input data. Xyleminc can therefore not guarantee that the estimated savings will actually occur.

## FC-1 Long-Term Lift Station Calculations

## Wet Well Design Worksheet

## Long-Term Flamingo East Parcel - FC1



The height to which this volume will rise in the wet well is calculated by the following equation.

| $\mathrm{H}=$ Volume/(Volume/foot of the wet well) $=$ | 5.92 feet |
| :--- | :--- |
| Use an actual height of: | $\square .00$ feet |

## Check Pump Run-Time

Actual Pump Flow Rate $=$ Run-time $=$ Volume $/$ flow rate $=$

| 891 |
| ---: |
| 2.50 |
|  |
| minutes |


| Finished Grade Elevation$113.59 \mathrm{ft}$ | Top of Wet Well | 113.84 ft | Grade to Top of Wet Well 0.25 feet |
| :---: | :---: | :---: | :---: |
|  |  |  |  |
|  |  | $\xrightarrow{3}$ | Total Wet Well Depth 19.88 feet |
|  | 103.46 ft |  |  |
|  |  |  | 0.5 feet |
|  | Alarm | 102.96 ft |  |
|  |  |  | 0.5 feet |
|  | Lag Pump On | 102.46 ft |  |
|  |  |  | 0.5 feet |
|  | Lead Pump On | 101.96 ft |  |
|  |  |  | 6.00 feet |
|  | Both Pumps Off | 95.96 ft |  |
|  |  |  | 1.50 feet |
|  | Top of Grout | 94.46 ft |  |
|  |  |  | 0.50 feet |
|  | Bottom | 93.96 ft |  |

NP 3202 HT 3~ 467

## Technical specification



Abte: Picture might not cornespond to the current configuration.
General
Patented sew cleaning semi-open channel impelar, ideal lor pumping in meste water appications. Possibie to b uporided with Guide-pint for even betior clooring resistance. Modumer based desion whih high adaptation oride.

## Impeller


Installation: P.Semi permanent, Wet


| Motor |  |
| :---: | :---: |
| Motor | N3202. 185 30-24-4AA+W 60 mp Standerd |
| Stator varlant | 1 |
| Frequency | 60 Mz |
| Rated voinge | 400 V |
| Number of potes | 4 |
| Prases | $3 \times$ |
| Rated pomer | 60 hp |
| Rated curnent | 08 |
| Starting cumtent | 4254 |
| Rated speed | 1770 rpm |
| Power factior |  |
| 1/1 Load | 0.91 |
| 9/4 Lodd | 0.88 |
| 1/2 Loed | 0.82 |
| Motor efficimey |  |
| 1/1 Lowd | 91.5 \% |
| 3/4 Laed | 92.0\% |
| 1/2 Lasd | 92.5\% |

Conflguration

## Different pumps are used for FC-1 in the short-term and long-term scenarios

## Performance curve



## xylem

NP 3202 HT 3-467


Duty Analysis

xylem
NP 3202 HT 3~ 467
VFD Curve


xylem
NP 3202 HT 3~ 467


VFD Analysis


Pumps Synning
1
1
1
1
1

| Frequency | Flow | Head |
| :---: | :---: | :---: |
| COHz | g38usgam | 146 |
| 547 Hz | 768 USgpm | 120 |
| 497142 | G98USgm | 1017 |
| 448Hz | 6RUSgpm | 818 h |
| 328 Hz | 508 USgam | 647 t |


| Shati power | Flow |
| :---: | :---: |
| 44.6 hp | 838 Usgpm |
| 342 m | 768 Uspm |
| 257 mp | gr8usgmm |
| 187 p | 63 USg mm |
| 132 p | 5\% Usgmm |


| Shath powner | Hyd eff. | specific energy | NPSHTe |
| :---: | :---: | :---: | :---: |
| 4.6 tp | 69.4\% | 713 MMNS MG | 13.6 n |
| 327 ¢ | 62.4\% | 59 MMHNSMG | 11.9 n |
| $227 \%$ | 694\% | 499 WMNS MG | 10211 |
| 187 mp | 694\% | 4 T0MMUSMG | $86 \%$ |
| 132 p | 69.4\% | 332 WMNSS MG | 7.12 l |

xylem
NP 3202 HT 3~ 467
Dimensional drawing


| Weight (lbs) |  |
| :---: | :---: |
|  |  |
| Pump with cooling jaded | Dish |
| 1235 | 100 |



## NP 3202 HT 3~ 467

Life cycle costs (LCC)

| Total lifetime | 15 | Inflation rate frate of price increases) | $2 \%$ |
| :--- | ---: | :--- | ---: |
| Annual operating time | 5600 | Interest rate (for investment) | $3 \%$ |
| Energy cost per KWh | 0.00 USD |  |  |
| Power input $P 9$ |  |  |  |

Total costs
0.00 USD Energy
0.00 USD Investment costs
0.00 USD Installation \& commissioning
0.00 USD Operating cost
0.00 USD Maintenance \& repair
0.00 USD Downtime
0.00 USD Environmental
0.00 USD Decommissioning
0.00

USD

First year costs
0.00 USD Energy (1st year)
0.00 USD Investment costs (1st year)
0.00 USD Installation \& commissioning (1st year)
0.00 USD Operating cost (1st year)
0.00 USD Maintenance \& repair (1st year)
0.00 USD Downtime (1st year)
0.00 USD Environmental (1st year)
0.00 USD Decommissioning (1st year)
0.00

USD costs for the input data. Xyleminc can therefore not guarantee that the estirnated savings will actually occur.

## FC-2 Short-Term Lift Station Calculations

## Wet Well Design Worksheet

## Short-Term Flamingo East Parcel - FC2

| Diameter $=$ | 12 ft |
| :--- | ---: |
| Area $=$ | 113.10 Sq ft |
| Vol. $/ \mathrm{ft}=$ | $846.0 \mathrm{gal} / \mathrm{ft}$ |
| Peak Flow In= | 833 gpm |
| Actual Pump Rate $=$ | 856 gpm |
|  |  |
| $\mathrm{V}=(\mathrm{QT}) / 4$ |  |
| where, |  |
| $\mathrm{Q}=$ Design Flow Rate $=$ |  |
| $\mathrm{T}=$ Assumed Cycle Time $=$ | 856 |
| $\mathrm{~V}=$ Volume $=$ | gpm |
|  |  |
|  |  |

The height to which this volume will rise in the wet well is calculated by the following equation.
$H=$ Volume/(Volume/foot of the wet well) $=\square 2.53$ feet
Use an actual height of:

## Check Pump Run-Time

Actual Pump Flow Rate $=$ Run-time $=$ Volume/ flow rate $=$

| 856 |
| ---: |
| 2.50 | minutes



Technical specification


Installation: P.Sewi permanent Wot



Note: Picture might not comespond to the current configuration.


| Motor |  |
| :---: | :---: |
| Mator ${ }^{\text {\% }}$ | N3102.920 18-11-AAS-W IE3 5.5MP Standard |
| Stator variant | 1 |
| Frequancy | 00 Hz |
| Rated volage | 400 V |
| Number of poles | 4 |
| Phases | 3- |
| Rated power | 5.5 hp |
| Rated current | 6.2 A |
| Starting current | 42 A |
| Rated speed | 1800 pm |
| Pomer fisctor |  |
| 1/1 Land | 0.90 |
| 3/4 Land | 0.86 |
| 1/2 Loed | 0.75 |
| Motor fiflelency |  |
| 1/1 Lod | 91.9 \% |
| 3/4 Loed | 91.5 \% |
| 1/2 Loed | 90.1 \% |

Configuration

Different pumps are used for FC-2 in the short-term and long-term scenarios

NP 3102 LT 3~ Adaptive 422

## Performance curve

| Pump |  | Motor |
| :---: | :---: | :---: |
| Discharge Fange Diameter | $315 / 16$ inch | Motor ${ }^{\text {a }}$ |
| Suction Range Diameter | 100 mm |  |
| Impeller diameter | $61 \%$ | Stator veriant |
| Number of blades | 2 | Frequency |
|  |  | Premed voltage |
|  |  | Number of pol |
|  |  | Phases |
|  |  | Rated power |
|  |  | Rated cument |
|  |  | Starting current |
|  |  | Reated speed |


| N3102.920 18-11-4AS-WIE3 5.5hp | Powar factor |  |
| :--- | :--- | :--- |
|  | $1 / 1$ Laad | 0.90 |
| 1 | $3 / 4$ Load | 0.86 |
| 60 H | $1 / 2$ Laad | 0.75 |
| 460 V |  | Motor efliciency |
| 4 | $1 / 1$ Laad | $91.9 \%$ |
| 3 m np | $3 / 4$ Lasd | $91.5 \%$ |
| 5.5 hp | $1 / 2$ Load | $90.1 \%$ |
| 42 A |  |  |




Duty point
Flow Head
833 US g.p.m. 16.8 ft

Project
Project ID
Created by
Created on 2/14/2018

Last updete

NP 3102 LT 3~ Adaptive 422
Duty Analysis

xylem
NP 3102 LT 3~ Adaptive 422


## VFD Curve


xylem
NP 3102 LT 3～Adaptive 422
VFD Analysis


Pumps
running 1syem

| Frequency | Flow | Head | Shant powner | Flaw | Hend | Shate power | Hyd eff． | Specific energy | MPSHre |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| COHz | $80 \times$ USg．pm | 15．3n | 5.37 pp | cor USgpm | r6．3n | 5.37 \％ | 62\％ | ga7MOMUSMC | 13.6 n |
| 55 Hz | 710USg．pm | M7 7 | 4.12 p | 7045 gmm | 147 | 401p | 618\％ | Tomurusmg | 11.7 䛔 |
| $5 \mathrm{SOH}^{\text {H }}$ | 608 US 9 pm | 132月 | 300 p | 608 USgpra | 132 n | 300 tp | 65．7\％ | TOMMUSMG | 10n |
| 45 Hz | 501 US 9．pm | 118 l | 22200 | 501 USgam | 118月 | 220 | 67．4\％ | G3MMVSMG | 852 年 |
| 40 Hz | 362 US $9 . \mathrm{pm}$ | 10．6 f | 1．521p | 362 US9．pm | 106月 | 1．52\％ | 67．5\％ | $5 \times .7 \mathrm{MWMUSMG}$ | 7．1年 |

xylem
NP 3102 LT 3~ Adaptive 422
riner
Dimensional drawing




## FC-2 Long-Term Lift Station Calculations

## Wet Well Design Worksheet

Long-Term Flamingo East Parcel - FC2

| Diameter $=$ | 12 ft |
| :--- | ---: |
| Area $=$ | 113.10 Sq ft |
| Vol./ft $=$ | $846.0 \mathrm{gal} / \mathrm{ft}$ |
| Peak Flow In= | 833 gpm |
| Actual Pump Rate $=$ | 907 gpm |
|  |  |
| $\mathrm{V}=(\mathrm{QT}) / 4$ |  |
| where, |  |
| $\mathrm{Q}=$ Design Flow Rate $=$ |  |
| $\mathrm{T}=$ Assumed Cycle Time $=$ |  |
| $\mathrm{V}=$ Volume $=$ | 907 |
|  |  |
|  |  |

The height to which this volume will rise in the wet well is calculated by the following equation.
$\mathrm{H}=$ Volume $/($ Volume/foot of the wet well $)=$

| 2.7 |
| :--- |
| feet |

Use an actual height of:
3.0 feet

## Check Pump Run-Time

Actual Pump Flow Rate $=$ Run-time $=$ Volume $/$ flow rate $=$

| 907 | gpm |
| ---: | :--- |
| 2.50 |  |

Finished Grade Elevation
xylem
NP 3202 HT 3~ 468 Technical specification


Imetallation: P.Semi permanent, Wet



Different pumps are used for FC-2 in the short-term and long-term
scenarios

FIEGGT


Note: Picture might not correspond to the cument configuration.
General
Patented sell cloaning semi-cpen channel impou-r, ideel for pumping in
ueste weter epplications. Possible to be upgraded with Guide-pind for even better cloging restiblence. Modutar based dasign with high adaptation grade.

## impalier

| Impelior materiel | H |
| :---: | :---: |
| Discharge Flance Olameter | $319 / 16$ inch |
| Suction Fiange biameter | 7710 inch |
| Imperiter diameter | 316 mm |

Number of biede

| Motor |  |
| :---: | :---: |
| Motor ${ }_{\text {動 }}$ | N3202. 185 30-19-4NA+W 45mp Standard |
| Stator vartant | 1 |
| Frequency | 60 Hz |
| Rated voltage | 460 V |
| Number of poles | 4 |
| Prases | 3- |
| Rated power | 45 hp |
| Rated curnent | 52 A |
| Starting current | 308 A |
| Rated speed | 1775 rmm |
| Power factor |  |
| 1/1 Lasd | 0.89 |
| 3/4. Land | 0.85 |
| 1/2 Lasd | 0.77 |
| Motor ofliciency |  |
| $1 / 1$ Lodd | 81.0\% |
| 3/4 Lasd | 91.5 \% |
| 1/2 Lowd | 91.0\% |

Configuration
xylem
NP 3202 HT 3~ 468

## Performance curve

xylem
NP 3202 HT 3~ 468
Duty Analysis

xylem
NP 3202 HT 3~ 468
VFD Curve

xylem
NP 3202 HT 3~ 468
VFD Analysis


\& . - Bate (x)


## NP 3202 HT 3~ 468

## Life cycle costs (LCC)

| Total lifatime | 15 |
| :--- | ---: |
| Annual operading time | 5600 |
| Energy cost per iath | 0.00 us0 |

Power input P1

Total cosis

0.00

USD

First year costs

0.00

USD

| Inflation rate (rate of price increases) | $2 \%$ |
| :--- | :--- |
| Interest rate (for investment) | $3 \%$ | $3 \%$


|  | 0.00 USD Energy |
| :--- | :--- |
|  | 0.00 USD Investment costs |
| $0 \%$ | 0.00 USD Installation $\&$ commissioning |
|  | 0.00 USD Operating cost |
|  | 0.00 USD Maintenance \& repair |
| $\%$ | 0.00 USD Downtime |
| $0 \%$ | 0.00 USD Environmental |
| $0 \%$ | 0.00 USD Decommissioning |


|  | 0.00 USD Energy (1st year) |
| :--- | :--- |
|  | 0.00 USD |
| $0 \%$ | 0.00 USD Instment collation \& commissioning (1st year) |
|  | 0.00 USD Operating cost (1st year) |
|  | 0.00 USD Maintenance \& repair (1st year) |
|  | 0.00 USD Downtime (1st year) |
| $0 \%$ | 0.00 USD Environmental (1st year) |
| $0 \%$ | 0.00 USD Decommissioning (1st year) |

Disclaimer. The calculations and the results are based on user input values and general assumptions and provide only estimated costs for the input data. Xyleminc can therefore not guarantee that the estimated savings will actually occir.

## BI-N Long-Term Lift Station Calculations

## Wet Well Design Worksheet

## Long-Term Flamingo East Parcel - Bl-N



The height to which this volume will rise in the wet well is calculated by the following equation.
$H=$ Volume/(Volume/foot of the wet well) $=\quad 4.67$ feet
Use an actual height of:

## Check Pump Run-Time

Actual Pump Flow Rate $=$ Run-time $=$ Volume $/$ flow rate $=$

| 395 | gpm |
| ---: | ---: |
| 2.50 | minutes |



## xylem

NP 3153 SH 3~ 275


## Technical specification



Note: Picture minh not correspond to the cerrent configuretion.

| General <br> Petented sell cleaning semiweste witer applications. Po for ev en better clogging res adaplation grede. | rannel impeltor, ideal for pumpin obe upgraded with Guide-pind Modutar based clasion with high |
| :---: | :---: |
| Impeller |  |
| Impener materta | Herctron ${ }^{\text {m }}$ |
| Discharge Flance Olameter | 31516 mach |
| Suction Fiange Dismeler | 150 mm |
| Impertor dismeter | 167 mm |
| Number of blades | 2 |
| Motor |  |
| Motor \# | N3153.185 21-18-28B-W/ 23 hp |
| Stator varlant |  |
| Frequency | 60 Hz |
| Rated voltage | 400 V |
| Number of poles | 2 |
| Phases | 3 |
| Rated power | 23 p |
| Rated curment | 26 A |
| Starting current | 215 A |
| Rated speed | 3510 pmm |
| Pomer factor |  |
| 1/1 Loed | 0.90 |
| 3/4 Lond | 0.87 |
| 1/2 Lowd | 0.79 |
| Pump Efficioncy |  |
| 1/1 Lond | 91.0\% |
| 3/4 Land | 91.5\% |
| 1/2 Lodd | $91.5 \%$ |

NP 3153 SH 3~ 275
Performance curve


Project
xylem
NP 3153 SH 3~ 275

## Duty Analysis


xylem
NP 3153 SH 3~ 275
VFD Curve

xylem
NP 3153 SH 3- 275
VFD Analysis


| Pumps running 13ymiom | Frequency | Flow | Head | Shatt power | Flow | Head | Shat power | Hyd ent. | Specific energy | NPSHT |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | cottz | 32 usgmm | 119 | 179 mp | 32 usg mm | 199f | 17.9 np | 558\% | 727 marus | 1412 |
| 1 | $5{ }^{5} \mathrm{H}$ | 310 US gmm . | 9961 | 138 p | 300 USgm | 9066 | 138 p | 58.8 | 612 win | 123年 |
| 1 | 50 Hz | 2710 | 8031 | 103 p | 2710 Sg pm | 823n | 103 p | 588\% | 509 tan US | 10.6t |
| 1 | 45te | 29 USgm. | 6a7a | 7314 | 29 USgpm. | 66.7 l | 7391p | 588\% | 417 MHTUS | 892f |
| 1 | 40tte | $22 \mathrm{USgm}$. | 527 n | 5.319 | 22 USgmm | 527 h | 5.310 | 558\% | 338 WTHUS | 738 |

xylem
NP 3153 SH 3~ 275


40

$\mathrm{B}^{\mathrm{N}}$


## NP 3153 SH 3~ 275

## Life cycle costs (LCC)

| Total lifetime | 15 |
| :--- | ---: |
| Annual operating time | 5600 |
| Energy cost per leth | 0.00 USD |
| Pomer input P1 |  |
| Total costs |  |


| Inflation rate (rate of price increases) | $\mathbf{2 \%}$ |
| :--- | :--- |
| Interest rate (for investment) | $\mathbf{3 \%}$ |



|  | 0.00 USD Energy |
| :--- | :--- |
|  | 0.00 USD Investment costs |
| $0 \%$ | 0.00 USD Installation $\&$ commissioning |
|  | 0.00 USD Operating cost |
|  | 0.00 USD Maintenance $\&$ repair |
|  | 0.00 USD Downtime |
| 0 | 0.00 USD Emvironmental |
| $0 \%$ | 0.00 USD Deconmissioning |

0.00

USD

First year costs

0.00 USD Energy (1st yeer)
0.00 USD Investment costs (1st year)
0.00 USD Installation $\&$ commissioning (1st year)
0.00 USD Operating cost (1st year)
0.00 USD Maintenance $\&$ repair (1st year)
0.00 USD Downtime (1st year)
0.00 USD Ervironmental (1st year)
0.00 USD Decommissioning (1st yeer)
0.00

USD

Disclaimer: The calculations and the results are based on user input values and general assurptions and provide only estimated costs for the input data. Xyleminc can therefore not guarantee that the estimated savings will actually occur.

Project

| Project to | Creeted by |
| :--- | :--- |

Created on

Last update 2/21/2018

## BI-S Long-Term Lift Station Calculations

## Wet Well Design Worksheet

## Long-Term Flamingo East Parcel - Bl-S

| Diameter $=$ | 6 ft |  |  |
| :---: | :---: | :---: | :---: |
| Area $=$ | 28.27 Sq fi |  |  |
| Vol./ft = | $211.5 \mathrm{ga} / \mathrm{ff}$ |  |  |
| Peak Flow In= | 341 gpm |  |  |
| Actual Pump Rate $=$ | 343 gpm |  |  |
| $V=(Q T) / 4$ <br> where, |  |  |  |
| Q = Design Flow Rate = |  | 343 |  |
| T = Assumed Cycle Time = |  | 10 | minutes |
| V = Volume $=$ |  | 857.5 | gallons |

The height to which this volume will rise in the wet well is calculated by the following equation.

| $H=$ Volumel(Volume/foot of the wet well) $=$ | 4.05 feet |
| :--- | :--- |
| Use an actual height of: | 4.00 feet |

## Check Pump Run-Time

Actual Pump Flow Rate $=$ Run-time $=$ Volume/ flow rate $=$

343 gpm
2.50 minutes


## xylem

NP 3153 SH 3~ 276

## Technical specification



Installation: $P$-Semi permanent, Wet



Note: Picture might not correspond to the current configuration.
General
Patented sel cieaning semi-open channel impelar, ideel for pumping in
waste water appicitions. Possible to be upgraded with Guide-pin or even bettor clogging resistance. Modult besed design with high adaptation grade.

Impallar
Impelior meterial
Olachurga Fling
Suction Flange Diameter
Suclion Flanga biameter
Number of biedes
Herdtion
31816 theh
31316 mat
190 mm
150 mm
2

| Motor |  |
| :---: | :---: |
| Molor | M3153. 185 |
| Stator variant |  |
| Frequency | 60 Hz |
| Rated votage | 460 V |
| Number of poles | 2 |
| Phases | 30 |
| Rated power | 17 hp |
| Rated current | 19 A |
| Starting current | 111 A |
| Rated speed | 3900 pm |
| Power fictior |  |
| 1/1 Land | 0.94 |
| $3 / 4$ Loed | 0.92 |
| 1/2 Lasd | 0.88 |
| Pump Efficiency |  |
| 1/1 Lond | 91.0\% |
| 3/4 Lasd | 92.0\% |
| 1/2 Laxd | 92.5\% |

## Conliguration

Performance curve

| Pump |  | Motor |
| :---: | :---: | :---: |
| Discharge Fange Diameter | $315 \mathrm{H6}$ inch | Motor ${ }^{\text {\% }}$ |
| Suction Range Diameter | 150 mm | Stalor vaniant |
| Impeller diameter | 61/4 ${ }^{\text {a }}$ | Frequency |
| Number of blades | 2 | Prated voltage |
|  |  | Number of poles |
|  |  | Phases |
|  |  | Rated power |
|  |  | Rated cument |
|  |  | Starting cument |
|  |  | Ratad speed |


| N3153.185 21-18-2FB-W 17hp | Power factor |  |
| :--- | :--- | :--- |
| 1 | $1 / 1$ Load | 0.94 |
| 60 Hz | $3 / 4$ Load | 0.92 |
| 460 V | $1 / 2$ Load | 0.88 |
| 2 | Pump Elliciency |  |
| $3-$ | $1 / 1$ Load | $91.0 \%$ |
| 17 hp | $3 / 4$ Load | $92.0 \%$ |
| 19 A | $1 / 2$ Load | $92.5 \%$ |
| 141 A |  |  |



Duty point
Flow Head
341 US g.p.m. $95.6 \mathrm{ft} \quad$ No
$\square$ ? That by 2/21/2018 -
xylem
NP 3153 SH 3~ 276

## Duty Analysis


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NP 3153 SH 3~ 276


VFD Curve

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NP 3153 SH 3~ 276


VFD Analysis


| Pumps: running ISyation | Frequency | Flow | Heed | Shaft power | Flow | Heed | Shat power | Hyd off. | Spectifc energy | NPSHre |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 6012 | 33 us 9 mm | 944 | M6mp | 399 USgpm | 944n | 146 p | 583\% | sesmanus | 142A |
| 1 | S9H2 | 3t3Uspm. | $066 \pi$ | 1150 | 313 USg mm | 8067 | 11.519 | 53\% | 458 mmus | 125 |
| 1 | 99月矿 | musgrm. | 6667 | 86719 | 2 USgm | $6{ }^{6} \mathrm{n}$ | 86710 | 5 | 4 tammus | 1076 |
| 1 | M91420 | 20usgpm | ${ }_{51}$ | $639 \%$ | 26 LSgmm | 5 | $639 \%$ | 563\% | 33 MWNS | 905 |
| 1 | 395 | zousgmm. | 0.6 | 2419 | zusgpm |  | $44 \%$ | 503\% | 20 MWH |  |

xylem
NP 3153 SH 3~ 276
Fint
Dimensional drawing


## NP 3153 SH 3~ 276

## Life cycle costs (LCC)

| Total lifetime | 15 | Inflation rate (rate of price increases) | $2 \%$ |
| :--- | ---: | :--- | ---: |
| Annual operating time | 5600 | Interest rate (for investment) | $3 \%$ |
| Energy cost per liMh | 0.00 USD |  |  |
| Power input P1 |  |  |  |

Total costs


First year costs

0.00 USD Energy (1st year)
0.00 USD Investment costs (1st year)
0.00 USD Installation \& commissioning (1st year)
0.00 USD Operating cost (1st year)
0.00 USD Maintenance \& repair (1st year)
0.00 USD Downtime (1st year)
0.00 USD Environmental (1st year)
0.00 USD Decommissioning (1st year)
0.00

USD

Disclaimer. The calculations and the results are based on user input values and genera/ assumptions and provide only estimated costs for the input data. Xyleminc can therafore not guarantee that the estimated savings will actually occur.

## Appendix F. Flamingo Crossings Letter Agreement

UTILITIES DEPARTMENT
Raymond E. Hanson, P. E., Director
9150 Curry Ford Road
Orlando, Florida 32825-7600
Telephone: 407-254-9809
Fax: 407-254-9899
Email: Ray.Hanson@ocflnet

January 19, 2018
John H. Classe, Jr., District Administrator
Reedy Creek Improvement District
Post Office Box 10170
Lake Buena Vista, Florida 32830
Re: Amendment to 2012 Flamingo Crossings Letter Agreement for Water and Reclaimed Water Interconnection and Wholesale Service by and between Reedy Creek Improvement District (the "District") and Orange County Utilities (the "County") Dated December 19, 2012 (the "Letter Agreement")

Dear Mr. Classe:
Upon counter-signature of this letter below by the District, the Letter Agreement shall be amended as follows:

The introductory language of Section 3 is amended as follows (deletions are double strikethrough and insertions are bold and double underline):
3. Volume and Delivery of Potable Water and Reclaimed.
A. Potable Water. RCID shall provide up to 240,000 508,000 GPD annual average daily flow (AADF) of potable water to the County to serve its customers in the area of Flamingo Crossings Boulevard and at the point of connection as indicated in Exhibit "A."
RCID shall provide said potable water services at the following flows and minimum pressures in pounds per square inch ("psi") at the connection point:

```
170355 GPM @ 55 psi (AADF)
```

600-6PM-60-9si (Peak heurflow)
2040 2.710 GPM @ 45 psi (Maximum Daily demand plus Fire Flow)
B. Reclaimed Water. RCID shall provide reclaimed water to the County to serve its customers in the area of Flamingo Crossings Boulevard and at the point of connection as indicated in Exhibit "A" in an amount that does not exceed the volume of wastewater County delivers to RCID (as governed by the Substitute Letter Agreement for Orange Lake/Reams Road Letter Agreement for Wastewater Interconnection and Wholesale Service entered into on $6 / 21 / 11$, as amended) and in no event shall

Amendment to 2012 Flamingo Crossings Letter Agreement for Water and Reclaimed Water Interconnection and Wholesale Service by and between Reedy Creek Improvement District (the "District") and Orange County Utilities (the "County") Dated December 19, 2012 (the "Letter Agreement")
January 19. 2018
Page 2
exceed 1.0 million GPD nor cause the residual service pressure in the Flamingo Crossings area, as determined by RCID in its sole discretion, to drop below 50 psi. RCID chat providesadreclaimed-water-serviees the following minimum-prescures in pounder per equare-nch-("pe")- at the cOnnection point 600GRM-50-50

The Initial Term as set forth in Section 7 of the Letter Agreement is hereby extended to January 24, 2028.

All other terms of the Letter Agreement remain unchanged and continue in full force and effect. If you agree to these changes, please sign both copies of this letter amendment and return one execution original to my office.

Sincerely,


Raymond E. Hanson, P.E., Director Orange County Utilities Department

Signed and Agreed to:


John H. Classes, Jr. District Administrator
Reedy Creek Improvement District
Date: $\quad 1 / 24 / 18$

UTILITIES DEPARTMENT
Raymond E. Hanson, P. E., Director
9150 Curry Ford Road
Orlando, Florida 32825-7600
Telephone: 407-254-9809
Fax: 407-254-9899
Email: Ray.Hanson@ocfl.net

January 19, 2018
John H. Classe, Jr., District Administrator
Reedy Creek Improvement District
Post Office Box 10170
Lake Buena Vista, Florida 32830
Re: Amendment to 2017 Substitute Letter Agreement for Orange Lake/Reams Road Wastewater Interconnection and Wholesale Service by and between Reedy Creek Improvement District (the "District") and Orange County Utilities (the "County") Dated October 11, 2017 (the "Letter Agreement")

Dear Mr. Classe:
Upon counter-signature of this letter below by the District, the Letter Agreement shall be amended as follows:

The introductory language of Section 4.2 and subsections 4.2.1 and 4.2.2 are amended as follows (deletions are double strikethrough and insertions are bold and double underline) (subsection 4.2.3 remains unchanged):
4.2 RCID agreed to accept, treat and dispose or reuse up to $2,350,000$ $\underline{\mathbf{2}, 535,000}$ gallons per day (gpd) annual average flow (AADF) of wastewater from the combination of the Reams Road and Orange Lake connection points upon the in-service date of the new force main referred to in Sections 2.2 and 3-for-a-term-ef-fifteen-yeare, subject to other provisions of this 2017 Substitute Letter Agreement.
4.2.1 Reams Road: RCID agrees to accept, treat and dispose/reuse up to 2, 100,000 gpd AADF (and 2,917 gpm PHF) of wastewater flow at Reams Road as of the Effective Date of this 2017 Substitute Letter Agreement. Upon 30 days written notice by RCID, the capacity at the Reams Road connection can be reduced to 2,000,000 gpd AADF (and 2,778 gpm PHF).
4.2.2 Orange Lake: RCID agrees to accept, treat and dispose/reuse up to $250,000435,000 \mathrm{gpd}$ AADF (and 624900 gpm PHF) of wastewater flow at Orange Lake as of the Effective Date of this 2017 Substitute Letter Agreement. The County's delivery of wastewater to the Orange Lake connection point shall not exceed

Amendment to 2017 Substitute Letter Agreement for Orange Lake/Reams Road Wastewater Interconnection and Wholesale Service by and between Reedy Creek Improvement District (the "District") and Orange County Utilities (the "County") Dated October 11. 2017 (the "Letter Agreement ${ }^{*}$ )
January 19. 2018
Page 2
$250,000435,000 \mathrm{gpd}$ AADF and 624900 gpm PHF without prior written notification to RCID and receipt of written approval from REID.

Sections 7 and 9 of the Letter Agreement are deleted in their entirety. The Letter Agreement is effective as of the date it was signed by the last Party thereto and shall be effective until January 24, 2028 (the "Initial Term"). The Letter Agreement shall be renewed automatically for two (2) successive terms of five (5) years beyond the Initial Term unless either Party provides written notice to the other Party at least one (1) year prior to the expiration of the Initial Term or at least one (1) year prior to the expiration of the first renewal term that the Party does not intend to renew the Letter Agreement.

All other terms of the Letter Agreement remain unchanged and continue in full force and effect. If you agree to these changes, please sign both copies of this letter amendment and return one execution original to my office.

Sincerely.


Date: $\qquad$
Raymond E. Hanson, P.E., Director Orange County Utilities Department

Signed and Agreed to:


John H. Classe, Jr. District Administrator
Reedy Creek Improvement District
Date: $\quad 1 / 24 / 18$

## Appendix G. Parcel Topographic Map

Figure G-1 FC-1 Topographic Map


Figure G-2 FC-2 Topographic Map


Figure G-3 BI North and South Topographic Map


## Appendix H. Existing FC West Pump Station Pump Curve

## Engimeering Submittal \#H



Accessories


Specials: (3) Mini-Cas

## Please Confirm Voltage For This Station

## FLAMINGO CROSSING PH 1 BP 1-2-RCID





## C-3201

## 

Motor Data


Cable Data

| HP | VOLTS | MAX. LENGTH FT. | CABLE SIZE/ NOMINAL DIA. | CONDUCTORS <br> ( N ONE CABLE) | PART NUMBER |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | *:200 | 165 |  |  |  |
| 30 (4 pote) | **230 | 220 |  |  |  |
| 30 (4 poie) | 460 | 450 |  |  |  |
|  | 575 | 700 |  |  |  |
| 30 (ep pole) | **200 | 170 | $\begin{aligned} & 5 / 3-2-1-\mathrm{GC} \\ & 31.0\left(1.22^{n}\right) \end{aligned}$ | (3) 6 AWG (PWA) <br> (2) 10 AWG (CTRL) <br> (1) 8 AWG (GND) <br> (1) 10 AWG (GC) | 942109 |
|  | **230 | 230 |  |  |  |
|  | 460 | 450 |  |  |  |
|  | 575 | 700 |  |  |  |
| $35 \& 40$ | **200 | 145 |  |  |  |
|  | $* * 230$ 460 | 170 335 |  |  |  |
|  | 575 | 510 |  |  | , |
| 47 | **200 | 110 |  |  |  |
|  | ${ }^{* * 230}$ | 145 |  |  |  |
|  | 460 | 290 |  |  |  |
|  | 575 | 455 |  |  |  |

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## REQUIREMENTS

Furnish and install 3 submersible non-clog wastewater pump(s). Each pump shall be equipped with an 47 HP submersible electric motor connected for operation on 460 volts, 3 phase, 60 hertz, 4 wire service, with ___ 50 feet of submersible cable (SUBCAB) suitable forsubmersible pump applications. The power cable shali be sized according to N.E.C. and ICEA standards and also meet with P-MSHA Approval. For 230 yolt service, two power cables shall be used to share the load and thus keep power cables to a manageable size. The purnp shall be supplied with a mating cast iron 6 inch discharge connection and be capable of delivering_ 800 GPM at_113' TDH.An additional point on the same curve shall be GPM at ____ feet total head. Shut off head shall be 168 feet (minimum). Each pump shall be fitted with_25 feet of 304 SS Grlp-Eye Cables.
$\square$ The working load of the lifting system shall be $50 \%$ greater than the pump unit weight.

## PUMP DESIGN

The pump(s) shall be automatically and firmly connected to the discharge connection, guided by no less than two guide bars extending from the top of the station to the discharge connection. There shall be no need for personnel to enter the wet-well. Sealing of the pumping unit to the discharge connection shall be accomplished by a machined metal to metal watertight contact. Sealing of the discharge interface with a diaphragm, O-ring or profile gasket will not be acceptable. No portion of the pump shall bear directly on the sump floor.

## PUMP CONSTRUCTION

Majorpump componentsshallbe ofgreycastiron,ASTM A-48, Class 35B, with smooth surfaces devoid of blow holes or other irregularities. All exposed nuts or bolts shail be AlSI type 304 stainjess steel construction. All metal surfaces coming into contact with the pumpage, other then stainless steel or brass, shall be protected by a factory applied spray coating of acrylic dispersion zinc phosphate primer with a polyester resin paint finish on the exterior of the pump.

Sealing design shall incorporate metal-to-metal contact between machined surfaces. Critical mating surfaces where watertight sealing is required shall be machined and fitted with Nitrile or Viton rubber O-rings. Fittings will be the result of controlled compression of rubber O-rings in two planes and O-ring contact of four sides without the requirement of a specific torque limit.

Rectangular cross sectioned gaskets requiring specific torque limits to achieve compression shall not be considered as adequate orequal. No secondary sealing compounds, elliptical O-rings, grease or other devices shall be used.

## COOLING SYSTEM

Each unitshall be provided with an adequately designed cooling system. The water jacket shall encircle the stator housing; thus, providing heat dissipation for the motor regardless of the type of installation. Impeller back vanes shall provide the necessary circulation of the cooling liquid through the water jacket. The cooling media channels and ports shall be non-clogging by virtue of their dimensions. Provisions for external cooling and seal flushing shall also be provided. The cooling system shall provide for continuous pump operation in liquid temperature of up to $104^{\circ} \mathrm{F}$. Restrictions below this temperature are not acceptable.

## CABLE ENTRY SEAL

The cable entry seal design shall preclude specitic torque requirements to insure a watertight and submersible seal. The cable entry shall consist of a single cylindrical elastomer grommet, flanked by washers, all having a close toferance fit against the cable outside diameter and the entry inside diameter and compressed by the body containing a strain relief function, separate from the function of sealing the cable. The assembly shall provide ease of changing the cable when necessary using the same entry seal. The cable entry junction chamber and motor shall be separated by terminal board, which shall isolate the interior from foreign materlal gaining access through the puimp top. Epoxies, silicones, or other secondary sealing systems shall not be considered acceptable. $\sim$ :

## MOTOR

The pump motor shall be a NEMA B design, induction type with a squirrel cage rotor, shell type design, housed in an air filled, watertight chamber. The stator windings shall be insulated with moisture resistant Class H insulation rated for $180^{\circ} \mathrm{C}\left(356^{\circ} \mathrm{F}\right)$. The stator shall be insulated by the trickle impregnation method using Class H monomer-free polyester resin resulting in a winding fill factor of at least $95 \%$. The motor shall be inverter duty rated in accordance with NEMA MG1, Part 31. The stator shall be heat-shrink fitted into the cast iron stator housing. The use of multiple step dip and bake-type stator insulation process is not acceptable. The use of bolts, pins or other fastening
devices requiring penetration of the stator housing is not acceptable. The motor shall be designed for continuous duty handling pumped media of $40^{\circ} \mathrm{C}$ ( $104^{\circ} \mathrm{F}$ ) and capable of up to 15 evenly spaced starts per hour. The rotor bars and short circuit rings shall be made of cast aluminum. Thermal switches set to open at $125^{\circ} \mathrm{C}\left(260^{\circ} \mathrm{F}\right)$ shall be embedded in the stator end coils to monitor the temperature oieach phase winding. These thermal switches shall be used in conjunction with and supplemental to external motor overload protection and shall be connected to the control panel. The junction chamber shall be sealed off from the stator housing and shall contain a terminal board for connection of power and pilot sensor cables using threaded compression type terminals. The use of wire nuts or crimp-type connectors is not acceptable. The motor and the pump shall be produced by the same manufacturer.

The combined service factor (combined effect of voltage, frequency and specific gravity) shall be a minimum of 1.15 . The motor shall have a voltage tolerance of plus or minus $10 \%$. The motor shall be designed for operation up to $40^{\circ} \mathrm{C}\left(104^{\circ} \mathrm{F}\right)$ ambient and with a temperature rise not to exceed $80^{\circ} \mathrm{C}$. A performance chart shall be provided upon request showing curves for torque, current, powerfactor, input/ output kW and efficiency. This chart shall.also include data on starting current and torque.

The power cabla shall be sized according to the NEC and ICEA standards and shall be of sufficient length to reach the function box without the need of any splices. The outer jacket of the cable shall be oll resistant chlorinated polyethylene rubber. The motor and cable shall be capable of continuous submergence underwater withoutloss of watertight integrity to a depth of 65 feet or greater.

The motor horsepower shall be adequate so that the pump is non-overloading throughout the entire pump performance curve from shut-off through run-out.

## beARINGS

The pump shaft shall rotate on two bearings. Motor bearings shall be permanently grease lubricated. The upper bearing shail be a single rollerbearing. Thelower bearing shall be a two row angular contact bearing to compensate for axial thrust and radial forces. Single row lower bearings are not acceptable.

## MECHANICAL SEAL

Each pump shall be provided witha tandemmechanical
shaft seal system consisting of two totally independent seal assemblies. The seals shall operate in an lubricant reservoir that hydrodynamically lubricates the lapped sealfaces ata constantrate. Thelower, primaryseal unit, located between the pump and the lubricant chamber, shall contain one stationary and one positively driven rotating, corrosion resistant tungsten-carbide ring. The upper, secondary seal unit, located between the lubricant chamber and the motor housing, shall contain one stationary and one positively driven rotating, corrosion resistant tungsten~carbide seal ring. Each seal interface shall be held in contact by its own spring system. The seals shall require nelther maintenance nor adjustment nor depend on direction of rotation for sealing. For special applications, other seal face materials shall be available.

The following seal types shall not be considered acceptable nor equal to the dual independent seal specified: shaftseals without positively driven rotating members, or conventional doubla mechanical seals containing either a common single or double spring acting between the upper and lower seal taces. No system requiring a prassure differential to offset pressure and to effect sealing shall be used.

Each pump shall be provided with an lubricant chamber for the shaft sealing system. The lubricant chamber shall be designed to prevent overililing and to provide lubricant expansion capacity. The drain and inspection plug, ${ }_{1}$ with positive anti-leak seal shall be easily accessible from the outside. The seal system shail not rely upon the pumped madia for lubrication. The motor shall be able to operate dry without damage while pumping under load.

## Seal lubricanf shall be FDA Approved, nontoxic.

## PUMP SHAFI

Pump and motor shaftshall be the same unit. Thepump shaftis an extension ofthe motor shaft. Couplings shall not be acceptable. The pump shaft shall be of carbon steel ASTM A 572 and shall be completely isolated from the pumped liquid.

## IMPELLER

The impoller(s) shall be of gray cast iron, Class 35B, dynamically balanced, double shrouded non-clogging design having a long throughlet without acute turns. The impeller(s) shall be capable of handling solids, fibrous materials, heavy sludgeand other matter found in wastewater. Whenever possible, a full vaned, not vortex, impaller shall be used for maximum hydraulic

## C-3201


efficiency; thus, reducing operating costs. Impeller(s) shall be keyed to the shaft, retained with an Allen head bolt and shall be capable of passing a minimum 3.0 inch diameter solid. All impellers shall be coated with an acrylic dispersion zinc phosphate primer.

## WEAR RINGS

A wear ring system shall be used to provide efficient sealing between the volute and suction inlet of the impeller. Each pump shall be equipped with a brass, or nitrile rubber coated steel ring insert that is drive fitted to the voiute inlet.

This pump shall also have a stainless steel impeiler wear ring heat-shrink fitted onto the suction Inlet of the impeller.

## VOLUTE

Pump volute(s) shall be single-piece grey cast tron, Class 35B, non-concentric design with smooth passages large enough to pass any solids that may enter the impeiler. Minimum inlet and discharge size shall be as specified.

## PROTECTION

All stators shall incorporate thermal switches in series to monitor the temperature of each phase winding. The thermal switches shall open at $125^{\circ} \mathrm{C}\left(260^{\circ} \mathrm{F}\right)$, stop the motor and activate an alarm.

A leakage sensor shall be available as an option to detect water in the stator chamber. The Float Leakage Sensor (FLS) is a small float switch used to detect the presence of water in the stator chamber. When activated, the FLS will stop the motor and send an alarm both local and/or remote. USE OF VOLTAGE SENSITIVE SOLIDSTATE SENSORSANDTRIP TEMPERATURE ABOVE $125^{\circ} \mathrm{C}\left(260^{\circ} \mathrm{F}\right)$ SHALL NOT BE ALLOWED.

The thermal switches and FLS shall be connected to a Mini CAS (Control and Status) monitoring unit. The Mini CAS shall be designed to be mounted in any control panel.
$\square$

## MODIFIGATIONS

1. Explosion-proof Pumps ( $X$ ).
$\square$
Reier to the General Guide Specifications for additional information.

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## ITT FLYGT WARRANTY

For the period defined, ITT FLYGT offers a commercial warranty to the orlginal End Purchaser agalnst defects in workmanship and material. Warranty covers parts and labor as outlined in ADDENDUM - A.

## COVERAGE:

ITT FLYGT will pay the cost of parts and labor during the warranty perlod, provided that the product, with cable attached, is returned prepaid to an ITT FLYGT Authorized Service Facility for repairs. Coverage for parts and labor will be provided for the perlod shown In ADDENDUM - A. The warranty period will begh from date of shipment or date of a valid Startup (For permanently installed pumps only). In cases where the Start-up date is used as the beginning of the warranty on a permanently installed pump, a Start-up Report completed by an approved service technician from an ITf FLYGT Authorized Service Facllty must be received by the ITT FLYGT Area Service Manager within thirty (30) days of the initial onset of the unit placed into service. If not received, the beginning of the warranty coverage will default to the product ship date. A start-up for a permanently installed pump must occur within one (1) year from the date of shipment from IT FLYGT or warranty will automatically default to shlp date as start of warranty. (See STORAGE section). When using the start-up date as the beginning of the warranty period then a copy of the Start-up Report is required to support a Warranty Claint. Warranty on Dewatering pumps will begin with ship date.

ITT FLYGT'S sole obligation under this Warranty shall be to replace, repair or grant credit for product upon ITT FLYGT'S exclusive determination that the product does not conform to the above warranty. In the event that the product is replaced, warranty on the replacement produci will be equal to the balance remaining on the original product or ninety (90) days, which ever is greater.

## MISUSE:

This Warranty shall not apply to any product or part of product which (I) has been subjected to misuse, misapplication, accident, alteration, neglect, or physical damage (il) has been installed, operated, used or maintained in a manner and/or In an application contrary to ITT FLYGT's printed instructions for installation, operation and maintenance, including whthout limitation operation without being contiected to monitoring devices supplied with specific products for protection; or (iii) has been damaged due to a defective power supply, improper electrical protection, faulty installation or repair, ordinary wear and tear, corrosion or chemlcal attack, an act of God, an act of war or by an act of terrorism; or (iv) has been damaged resulting from the use of accessory equipment not sold by ITT:FLYGT or not approved by ITT FLYGT in connection with the product.

## WEAR PARTS:

This warranty does not cover costs for standard and/or scheduled maintenance performed, nor does it cover parts that, by vitue of their operation, require replacement through normal wear (aka: Wear Parts), unless a defect in material or workmanship can be determined by ITT FLYGT. Wear Parts are defined as eutters, Cutting Plates, Impellers, Agitators, Diffusers, Wear Rings (Stationary or Rotating), Volutes (when used in an affasive environment), oil, grease and/or any items deemed necessary to perform normal maintenance on ITT FLYGT equipment. '

## DISCLAIMERS:

(i) ITT FLYGT'S warrantles are null and void when the product is exported outside of the United States of America without the knowledge and written consent of IIT Flygt US; (ii) ITT FLYGT makes no independent warranty or representation with respect to parts or products manufactured by others and provided by ITT FLYGT (however, ITT FLYGT will extend to the Purchaser any warranty received from ITT FLYGT'S supplier of such parts or products).

## ITT FLYGT WARRANTY

## LIMITATIONS:

ITT FLYGT NEITHER ASSUMES, NOR AUTHORIZES ANY PERSON OR COMPANY TO ASSUME FOR ITT FLYGT, ANY OTHER OBLIGATION IN CONNECTION WITH THE SALE OF ITS EQUIPMENT. ANY ENLARGEMENT OR MODIFICATION OF THIS WARRANTY BY A DISTRIBUTOR, OR OTHER SELLING AGENT SHALL EECOME THE EXCLUSIVE RESPONSIbILITY OF SUCH ENTLTY.

THE FOREGOING WARRANTY IS EXCLUSIVE AND IN LIEU OF ANY AND ALL OTHER EXPRESS OR IMPLIED WARRANTIES, GUARANTEES, CONDITIONS OR TERMS OF WHATEVER NATURE RELATING TO THE PRODUCT(S), INCLUDING WITHOUT LIMITATTON ANY IMPLIED WARRANTIES OF MERCHANTABILITY.AND FITNESS FOR A PARTICULAR PURPOSE WHICH ARE HEREBY EXPRESSLY DISCLAIMED AND EXCLUDED. PURCHASER'S EXCLUSIVE REMEDY AND TT FLYGT‘S AGGREGATE LIABILITY FOR BREACH OF ANY OF THE FOREGOING WARRANTIES [S LIMITED TO REPAIRING OR REPLACING THE PRODUCT AND SHALL IN ALL CASES BE LIMITED TO THE AMOUNT PAID BY THE PURCHASER HEREUNDER. IN NO EVENT IS ITT FLYGT LIABLE FOR ANY OTHER FORM OF DAMAGES, WHETHER DIRECT, INDIRECT, LQUIDATED, INCIDENTAL, CONSEQUENTIAL, PUNITIVE, EXEMPLARY OR SPECIAL DAMAGES, INCLUDING BUT NOT LIMITED TO LOSS OF USE, LOSS OF PROFIT, LOSS OF ANTICIPATED SAVINGS OR REVENUE, LOSS OF INCOME, LOSS OF BUSINESS, LOSS OF PRODIUCTION, LOSS OF OPPORTUNITY OR LOSS OF REPUTATION.

ITT FLYGT WILL NOT BE HELD RESPONSIBLE FOR TRAVEL EXPENSES, RENTED EQUIPMENT, OUTSIDE CONTRACTOR'S FEES, EXPENSES PERFORMED BY AN UNAUTHORIZED REPAIR SHOP, UNAUTHORIZED ALTERATIONS, OR FOR PUMPS USED WITHOUT ITT FLYGT SUPPLIED CABLE OR CONTROLS UNLESS IT CAN BE PROVEN SUCH ANCILLARY EQUIFMENT IS SUITABLE FOR THE PURPOSE AND EQUAL TO ITT FLYGF CABLES OR CONTROLS THAT WOULD ORIGINALLY BE SUPPLIED WITH THE TYPE OF EQUIPMENT IN USE. REIMBURSEMENT COSTS FOR CRANES AND/OR ANY SPECIAL EQUIPMENT USED IN CONJUNCTION FOR THE REMOVAL OR REINSTALLATION OF ANY IIT FLYGT EQUIPMENT IS NOT COVERED UNDER THIS WARRANTY.

## REQUIREMENTS:

$\because$
A copy of Electrical System Schematics of the control used (induding Control's Bill of Material) could be required to support a Warranty Claim when a non Flygt control is used. In addition, a written record, hereby known as "the log", will be assoclated with each unit serial number and must be maintained by the organization having product maintenance responsibility. The log must record each preventative maintenance activty and any repalr activity during the life of the warranty or verifleation that a Flygt authorized Service Contract is in force and is available for review and/or auditing. Failure to meet these conditions could render this warrant null and void. Such logs could be required to determine warranty coverage.

## STORAGE:

$=$
Should a delay occur between shlp date and the date of start-up, mainterance as outined in ITT FLYGT's care \& Maintenance Manual must be performed by the "CONTRACTOR" and/or "OWNER" during any such period of storage. Documentation providing proof and outlinfing what maintenance was performed must be provided to ITT FLYGT or its representative within thisty (30) days of said maintenance, or the ITT FLYGT warranty could be considered vold.

## ITT

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## ITT FLYGT WARRANTY

## CONTROLS:

Warranty coverage for permanently installed controls start on date of shipment to end purchaser. This warranty does not apply to controls that have been damaged due to a defective and/or improper input power supply, improper electrical protection, accidental damage, improper or unauthorized installation and/or repair, unauthorized alteration, negligence, environmental corrosion or chemical attack, improper maintenance or storage of control, any act of God, an act of war, an act of terrorism or damage resulting from the use of accessory equipment not approved by ITT Flygt. Further, this warranty does not apply in the event an adjustment is found to correct the alleged defect.

Solid state devices will be covered for a period of one year. Electrical control panels containing controlers, PLC's, drives, soít starts, and other computerized equipment require Transient Voltage Surge Suppression (TVSS) protection in order to satisfy the requirements of this warranty. The protection equipment associated with the control must be kept in working condition during the life of the warranty. Auxllary equipment supplied with the control (alr-conditioners etc.) is limited by the respective origlnal equipment manufacturer's warranty offered. Components not supplied by Flygt are not covered by this warranty.

## rops (The Optimum Pump Station)

ITT Flygt will warrant the TOPS pre-engineered fiberglass pump station components against defects in material and workmanship for a period of one (1) year from date of start-up or eighteen (18) months from date of shipment, whichever is sooner to the original owner of the station. Warranty shafl cover the cost of labor and materials, excluding removal and reinstallation costs, required to correct any warrantable defect, FOB, Manufacturer's authorized warranty service location, ITT Flygt products contalned within a TOP5 pre-engineered fiberglass pump station will carry the standard $\Pi T$ Flygt warranty for the product and/or accessory installed in the TOPs pre-engineered flberglass pump station.

All restrictions and/or limitations as outlined and described within the context of this warianty are germane to all sections of this IT Flygt Warranty document.

## ITT Flygt US

Wational Quality Assurance - US Corporate prodqual@ill.com


## ITT

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## ITT FLYGT WARRANTY

ADDENDUM - A
WARRANTY COVERAGE BY PRODUCT


## Section 2 wnere $\because:$ Flygt Pumps $\quad$ Issued: 6/80 $\mid$ Supersedes: $2 / 88$

The laultless lunctioning of a Flygt Pumping Station will depend upon the correct selection of the pump to suit system requirements and proper installation. Agreat majority of Fiygl Electric Submersible Waslewaler Pumps are. installed in underground wei pils with Automatic Discharge Connections, Gulde Bars and Access Covers as shown in the station drawings. Wet Pits constructed of precast concreta rings offer significant savings in labor costs over pourad-in-place concrete, masonry or brlck and are universally accepted tor use in sanitary or storm sewer systams. Precast concretesections are availabla up to 120 Inch inside diameter (sometimes up to 14 inch $^{\text {In }}$ Inside diameter) throughout the U.S. and are generally manufactured in accordance with the provisions. of ASTM Speciflcation C478

Because of this, Fyyg Cofporation's official engineering docurnentation is based on stations designed in precast concrete circularman-holes. Eachindlvidual stationdrawing shows a suggested Simplex and a suggested Duplex PumpingStallon bult of precast concretesections installed between a Eottom Slab and a Top Slab the Top Slab, usually atground level, contalns the cast-in Access Cover). The contigurations and dimensions shown on these Proposed Layouls are suggested minimum requirements only, all details, including sizing of pll, typer size, location and arrangement of valves and piping, etc. are to be specilied by the Consulting Engineer andare sublect to his approval.

The following is a partlal list of useful suggestons for construction and installation. (Please always observe local regulations applicabie).

## A. Excavation:

Excavate a large enough hole to provide sutficient working room around the statlon. The oulside dlameter of the Bottom Slab should be at least one foot laiger than that of the concrele sections used.

## 日. Conneoting Plpes:

Provide connecting holes for the Inlluent Pipe, Effluent Fipe(s) and Cable Thrulats in eccordance with the Engineer's specitication. Flexible joints outside of concrete wall will reduce the danger of dislocation due to settement.

## C. Backfill:

Backill gradually and bvenly around station after concrate and Joints have hardened. Compact backill to minimize post-Installation setllement.

## D. Top S!ab with Access Cover:

Diameter of Top Slab shall be alleast two leet larger inan O.D. of ring sections. The Access Cover must be installed and properly ontented in the Top Slab.

1. See Station Drawngs for Pump Model and Access Cover location in relation to the cenlerline of the station.
2. Positloning of the Hinge Side of the Cover (See Accessorles Section).
3. The Top Slab and Access Cover must be level.
4. For Heavy Duty Covers
(See Accessories Section).

## E. Automatic Discharge Comnection:

The Automatic Discharge Connectlon must be attrached to the Bottorn Slab at the exactlocation required relative to the Access Cover.

## SUGGESTED PROCEDURES:

1. Aftach the Upper Guide Bar Bracket(s) to the Access Frame (See Accessories Sectlon). Also, the centerifine of the Bracket(s) will detemine the centerline of the installed pump(s).
2. Place thepump Discharge Connection(s) ontha Bottom Slab and line up as shown in the Accessories Section.
3. Cut to length and install the Guide Bars between the Upper Guids Bar Bracket(s) and Dlscharge Connection(s).
4. Before secuing anchor bolt nuts, check across the Discharge Connectlon(s) Outlat Flange(s) face with level and shin if necessary. Gulde Bars should be Parallel and Vertical.

## F. Internal Piping and Manifold:

Use proper gaskets, tighten bolks gradually and evenly. In deep statlons, install Dlscharge pipe Brackets to relleve Discharge Connections from overload and intermedlate Guida Bar Brackats to prevent Gulde Bars from bending.

## Installation Procedures

## Section 2. xater $\therefore \therefore$ Flygt Pumps $\quad$ Issued: 6/90 $\quad$ Supersedes: 2/88

G. Installation of Pump Unlts:

Lower Pump Units into place along guide bars. Check visually metal-to-melal conlact between Volute Flange and Discharge Connection.lf necessary, re-check and reallignDlscharge Connection(s) and GuldeBars with pumps in place.

## H. Groutling:

After properallgnment of all components, including metal-to-metal connectlon of pump flange is astablished, grout Access Cover, DischargeConnection(s) and Plpe Thrulets. Bulld up and shape slopes at bottom of the station as shown in Station Drawinge. Thls will help in preventing bulld-up of sollds at the bottom where slde walls meet the floor.

## 1. Surtace Protectlon:

An epoxy-coal tar system is suggested for all Internal surfaces, concrete or metallic, if possible, follow the recommendations in WPCF Manual of Practice No. 17 -Paints and Protecive Coatings forWastewater Treatment Facilities" or the instructions of a reputable manulacturer of protectivacoating systems, such as Carbollne, Koppers, Inettol, Perry-Austen, etc. Proper surfacepreparation and caretul application will pay off in reduced malntenance costs and longer ilfe.
J. Storage of Pump Unlts Prior to Start-Up: It is not good pracllce to store the Pump Unils in the wet pit, especially when long perfods between instalkation and start-up areantcipated. If this practice cannolbe avolded, rather thanleaving them on their Dlscharge Connections, secure them and their power cable at some point above any anticlpated liquid level. Pay special aitention to unprotected open cable ends; seal them off and make sure that they are not submerged or exposed to molsture. Penetration of moisture thru the cable may cause breakdown of the insulation, arcing at the pump terminal board, destructlon of the Junction Chamber and serious damage to the pump. It in doubt, before start-up, recheck the cable, Cable Entry and Junction Chamber following instructions in the Maintenance Manual under "Electrical Checks'. If possible, connect Pumps power cables to Control Panel and duringlonger periods untll the official starl-up, start and run the unlts manually for 30 seconds atleast once every two weaks. (see "Storage" in this section.)

## Storage

## 

Each Flygt pump leaves the factory properly assembled and prepaired to perform even after a reasonable idle time in storage. However, as prolonged idle time can be detrimental to any rotating machinery, the procedures outting below should be followed in order to insure that the equipment is in top conditton to oparate when flnally installed. Whenever possibie, store pumping units in a dry environment free of extremie temperatures and strong direet sunlight.

## NEW pumps:

## Storage 6 to 12 montha:

In general, rotaling machiney left Idis for exlended perlods of time, tends to establish a "set" position due to inaction of themovingparts. Some of these areasmaybe damaged (especially seals) from the sudden fast breakaway of start-up after a prolonged Idle lime. To insure that all rotaing parts are free for final installation and start-up, it is good practice to rolate the impeller by hand onee a month. Itls also good practice to relieve the tension on the cable entry sealling grommet by backling off the cable entry compression scraws slightly. If this is done, Itis most important that a clear note be attached as a reminder to: Re-Tighten Cable Entry Compression Screws Before installation.

Storage 12 to 24 months:
In addition to the above, apply a protective spray coating of silicone or rust inhibiting oil to the impeller and inside of the volute by spraying in through the volute outtet and up through the volute inlet. Also coat the volute outlet flange face.

## USED pumps:

Before storing a used pump for an extended period of time, the unlt should be dismantled, checked for any defects, repatred where necessary and reassembled. At reassembly, follow Instructions in the Service Manual, especlally regarding seal assemblles. Protect the impeller and volute as mentloned in the paragraph above.

In all casos, it is good practice to check all external bolts, nuts and screws for tightnese betore final Insiallation after extended storage.

## CONTROLS:

It is most imporkant to make sure that Electrical Controls, when subjectedto extended storage, be stored in aprotected dry environment. free from any carrosive almosphere. Molsture in any form, inciuding condensation, can cause serious corrosion problams to the contactpoinl surfaces as wall as terminal connections.

Even though all terminal connectlons have been made tight on initlal assembly at the factory, they maynotremain $100 \%$ tight over an extended storage perlod due to the compressibllity of the copper wire and possible movement due lo variations in ambient temperature. The problem will vary in degree depending on wire size and whether the terminal connectlon is of solld or stranded vire. To insure proper operation, recheck all terminal connection screws for lightness prior to placing the control on line.

## Explosionmproof Pumps



Exploslon-proof Pumps for Hazardous Locations
ITTFygiÉlectric Submersible Exploslon-proof Wastewater Pumps are examined; tested, and approved by Factory Mutual Research (FM) as Explosion-proof. They conform to the latest editlon of the National Electrical Code (NEC), Articles $500,501,502$, and 503 requirements as explosion proof and suitable for use in Class I, Division 1, Groups $C$ and $D_{1}$, and dust ignition proof and suilable for use in Class III, Division 1; Groups E and G hazardous locatlons, and suitable for use InClass lil, Division 1 hazardous locations. FM approval also meets OSHA (Occupational Safety and Health Administration) requirements.

## Deffitition of Hazardous Locations by NEC

Class i locations are those in which flammable gases or vapors are or may be present in the air in quantitles sufficient to produce explosion or ignitable mixtures.

Class I, Division 1 location is a location: (1) in which ignitable concentrations of tlammable gases or vapors exist under normal operating condilions; or (2) in which. ignitable concentralions of such gases or vapors may exlst frequently because of repalr or maintenance operations or because of leakage; or (3) in which breakdown or faulty operatlon of equipment or processes mightreleáse ignitable concentrations of flammable gases or vapors, and might also cause simultaneous fallure of electric equipment.

Class Il locations are those that are hazardous because of the presence of combustible dust.

Class $\mathrm{II}_{\mathrm{I}}$ Division 1 location is a location: (1) in which combustible dust is in the air under normal operating condilions in quantities sufficient to produce explosive or ignitable mixturss; or (2) where mechanlcel failure or abnormal operation of machinery or equipment might cause such explosive or ignitable mixtures to be produced, and might also provide a source of ignition through .simutaneous fallure of electric equipment, operation of protection devices, or from other causes; or (3) In which combustlble dusts of an eiectrically conductive natura may be present.

Class ill focations are those that ara hazerdous because of the presence of easily ignllable fibers or flyings but not likely in alr suspension in quanitles sufficient to produce Ignifable mixtures.

Class Ill, Division 1 location la one in which aasily ignitable flibers or materlals producing combustlble flyings are handled, manufactured, or used.

## Speclal Features

The construction of an Explosion Prool pump is similar in most respects to the standard wastewater pump, but differs in the following details:

1. Hydrostallcally pressure tested high strength, cast iron housings are designed to wilhstand an Internal explosion and have long tight flame paths to reduce extt temperature of any exploding. gases to a value below the Ignitton temperature of the surrounding environment.
2. All pumps have required pllot thermal sensors embedded in stator windings, to guerantee that the pump surface temperature never exceeds safe limits, avoiding possible environmental lgnition.
3. Externally mounted leakage sensors may not be used unless explosion proof or intrinsically safe (consult factory for detalls).
4. Special approved power cables required: Flygt SUBCAB.
5. All pumps, except $3075(X), 3085(X), 3102(X)$ and $3127(x)$, have a special stator inspection plug. The $3075(X), 3085(X), 3102(X)$ and $3127(X)$ stator housings are inspected for leakage through the cable entry. Here, penetration of oll from the oif chamber below, or water from the functionchamber above canbedetected.
6. ITT Flygt controls supplied with these pumps incorporate the following required circuits:

## A. Motorpilot themalsensors (connectionls approval mandatory).

B. Intrinsically safe relays for ENM-10 level sensors (or equal) - usage is mandatory.

CAUTION: All controls, used wilh these pumps but not supplied by ITT Flygt, must be designed according to the latest applloable standards. See Tab Section 11 for addltional detalls and requirements.

## Environmental Limits ${ }^{\text {' }}$

The maxlmum temperature of exposed (external) pump surfaces is sell controlled by the motor pllot thermal switches. Meximum allowed amblent (environmental) temperature is. $115^{\circ} \mathrm{F}\left(46^{\circ} \mathrm{C}\right)$.

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## Explosion-proof Pumps

CAUTION: To maintain APPAOVAL, the pump cannot be altered without Factory Mutual permission and service mustbedone by an Explosion-proof CERTIFIED repairman. For training and certiflication detaifs, consult faclory.

## Appilcation of Explosion-proof Pumps

These purnpe may be used in sewage wet wells that are classilled as Class I, Division 1, Groups C and D hazardous locations (gases and vapors). Thay can also be used in applications that are clessified as Class II, Division $\dagger_{1}$ Groups $E$ and $G$ hazardous locatlons (lyplifed by grain or coal storage); also, Class III, Dkislon 1 locatlons (libers and flyings).

Other areas, which may be classified hazardous under normal condiflons and where the use of Explosion-prool pumps for handling contaminated wastewater is required are: reflneries,"petrochemical industry locations, tank farms, gas utilityvaults, etc, alwaystakinginto consideration that these pumps are not designed or approved as process pumps dellberately and protractedly handling high concentrations of hazardous liquids, e.g.: gasoline, etc.

## Limitations

1. CP/CS, DP/DS and FP/FS $3085(X)$ does not oplionally have aterminal board as does the standard version.
2. $\mathrm{CP} / \mathrm{CT} / \mathrm{CS}$ and $\mathrm{HP} / \mathrm{HS} 3201(\mathrm{X})$ for 230 volf service requires two (2) 6/3-2-1 power cables.
3. Nons of these Explosion-proof pumps is available in the Warm Liquid (WL) Variant.

Division 2; All Classes: For Class I or II locations; a Olvision 2 designation means that the ignitable or combustble materlals will not normally be present in hazardous concentrations exoept by accident or malunctions of containing or protective systems. In Class III locations, Division 1 and 2 are almost the same (check NEC Arilcle 503).
Equipment approved as sultable for use In Division 1 locetions is autornatically sultable for use in Dlysion 2 looations. However, if the. Authority Having Jurlsdiction has dellillely defined the area as Division 2, standard submersible pumps (motors) may be used so long as they do not oontaln any open (non-hermetically seated) ignition sources (See NEC Article 501-8 and 502-8) and use motor pllot thermal switches to limit surface temperatures. Standard ITT Flygt submersible pumps meet these requirements.

## Classification

A sewage wet well (or any other wastewater collection locatlon) is not automatically a hazardous focation. The neture and classitication of any locatlon musl be determined and Indicated by whoever is considered to be the Authortty Having Jurisdiction.
This Authority is not always easily determined. Care and dillgence must be exercised to make sure, once a preliminary Identification has been made، that there is not some other superseding Authority.

Depending on the type and geographical position of the "locatlon'", the Authority may range the gamut from a federal agency to state, reglonal, local agencies or the consulting or plant engineer. Often the best source of information is the state Administrattive Code or a state agency such as a Department of Environmenial Protection (DEP), Environmental Protection Agency (EPA), Department of Healih, etc.

## Approval Fequirements (NEC/Factory Mutual)

Class 1, Division 1: sultabla equipment must be explosion proof. It must also contaln pllot motor thermal sensors (which must be connected in the motor control).

Class II, Division 1: suitable equipment'must be "dust ignition proof" and use motor pilot sufface temperature llmiling thermal switches as in Class I.

Class ill, Division 1: suitable equipment need only bs totaliy enclosed, non ventilated.

Curtent Approvals for hazardous location pumps previously noted are by FM (Factory Mutual Research). FM Is officlally.llsted by OSHA (Occupational Safety \& Health Administralion), In the Fideral Register as a Nationally recognized testing laboratory (NRTL). It is in all regards equivalent to UL (Underwriters Laboratory).

Restrictions: The listed ( $X$ ) pumps are not approved for "process pumping" where high concentrations of liquids (other than wastewater) are handled for process work, Iransfer, or recovery, The acceptable usage is for handiling waslewater (contaminated water, sewage, etc.) for the purposes of treatmenf, transfer, storage, or dilsposal.

No accessory equipment may be atlached to an approved pump unless it is specifically approved for the location or "intrinsically sata" (See NEC 500-2 for Intrinsic Safe requirements).

## Explosion-proof Pumps

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WARNING: All NEC and local code requirements must be scrupulously observed when making an installation. Be certain thal giands and conduits where pump(s) or control whing/cable passes irom a hazardous location (wet pit, etc.) to electrical service, controls, or nonclasslifed area are sultably sealed agalnst passage of gases or liquids.

Aggressive Liquids: Depending on temperalure, pH , concentration, and their intrinslo reaclivity, certain contaminant chemicals (acids, alkalles, soivents; etc.) may have a delerlorating elfect on the equipment and pose a safely hazard to the Installation. Be careful to fully examine these circumstances with the end user or his representative and consult with ITT Flygt.

A number of alternative configurations or approaches are available which may make the equipment sultable in the presence of these materials: alternate elastomers, cable sheathing, special cable entries, elc.

Accessorles: Non-sparking bronze "Safe-Slide ${ }^{(1)}$ " installation/removal gulde accessories are available for all approved pumps. While not required by the Approval Authority they may be desired by local authoritles and do provide an extra margin of safety for particularly hazardous classified locations.

Cable: Flexlble cords or cables used in hazardoús locations must be of the NEC type "extra-hard usage" and be specifically approved/lested for the approved equipment (motor/pumps) whlch they will be used with. Nounapproved substlutions mavibe made withoutloss of oficial approval Cables supplied by $\Pi$ Flygt and used with ITT Flygt electric submersible pumps are FM tested and approved for the hazardous locations listed for the pumps in the beginning of this Explosion-proof pumps section.

Toprotect against the damaging and unsate effects of very aggressive contaminants (fiquids, dissolved solids) in the wastewater, special cable entries are avallable which will allow pipe or slainless steel flex hose sheathing to be attached to protect the cable.

Special Exceptions for Hazardous Locations: It is possible in some circumstances to use standard pumps in what would normally be declared as hazardous locations. These approaches are supportad by varlous codes but may not be used if speciflcally disallowed by an Authorty Having Jurlsdiction.

Guaranteed Pump Submersion (GPS): Ifthe equipment Is so controlled that the llquld levalnever falls below a point 4 - 6 inches above the topmost point of the pump, then
standard non-approved pumps may be used. This is because the volume below a liquidsurfacels not consldered hazardous.

The maans for guaranteeing that a pump will always remain submergad during operation vary from one pari of the couniry io another. Consult IT Flygt for appropilate configurations.

Declassification: An examination of local/state administrative codes, NEC Chapter 5 ; and NFPA Standard 70 C and 496, shows that a hazardous location may be reduced in claseffleatlon from Division 1 to Divislon 2 or even to a nonhazardous condition through the use of suitable air purging and use of monitoring safaguards. This would then allow the instailation of standard pumping equipment.

This is a common practice in many parts of the country when the installation makes it practical. The approach has additlonal benelifs: purging not only removes any explosivel flammable gases, but also removes smothering or poisonous gases thus improving the personnel safety aspects of the location.

## Mine Safety and Health Administration (MSHA)

Equlpment approved by MSHA (Permissible-sultabie for use in gassy mines) may not be used In any hazardous location covered by the NEC categones (Class i, II, or III) without written permission of the Authority Having Jurisdicilon.

Nor may equipment approved//lsted by FM or UL be used in a gassy mine withoul the written approval of MSHA.

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## ITT Fiygt Grip Eye System

The nomal method of lowering and ralsing a CP pump in and out of a lift stallon is by use of a chain or cable attached to the pump. The length of the chain or cable is dependent on the depth of the station. The average length would probathy be between 18 to 20 ft . and in certain cases may be much longer. In many cases, depeniding on the lifling device (usually a holst), the operator may have to take a second or third bite on the pump chaln in order to lift the pump clear of the station.

An added acceassory to the ITT Ffygt lirie is the pattented ITT Flygt Grip-Eye System which consists of 33 it of nifton line; a short tength of high tensile strength gavanieed chain and a forged "Grip-Eye" of wrought alloy steel.

The operation of this positive recovery system is as follows:
1.. Connect the small eye of the "grip eye to the end of the:hoist cable.
2. Slip the end of the ryton line throught the large eye of the:grip-aye: The nyton … $\therefore$ line simply acts as a guide for the grip-eye on its way down to the short length of the pump lifting chain.
3. While keeping the myon line (guide fine) taut, proceed totower the grip-eye .- until it is.well positioned over the pump lifting chain.
4. Release the tension on the nyon guide line. The lifting chain will now take a position to become engaged in the grip-eye.
5. Gradually take up tension on the hoist cable and the grip-eye will make a positive grip on the pump lifting chain. Continue hoisting until the pump is clear of the station.

Caution: The Giap-Eyes may only be used with the corresponding special IT Fyg̀t Chain Sling Units.

Grip-Eyes are not warrantied if other chains are used.
$\therefore$. . Refer to the following:pages for pumpimodels:andicorrect assembly.

fig. 1
(Slandand) The eno ding of the Ctwin Slivy is slipped overtwe pumpliftingtuandle


FIG. 2
(Customet to surpply exití shackel A snacste can be strackle) A snacide can be
used in conpuncion mith the used in connericion with the
standaldangstrouldoriam. er choose nod to rernowe and replace pump harntle.


FIG. 3
(Slancard) This type (Slancara) This type conces inith a strackle as
partonthe CTwinSting for conrecting to pump fitting hanctle.

## Povers

FASTENERS
SPECIFICATION \& DESIGN MANUAL
AC100 Plus'

## AC'100 Plus ${ }^{\text {™ }}$ Epoxy Aerflate Atherive Anchoring System

## PRODIIGIDESEDPIDM

The AC100 Pius epoxy arcyiate adhesive system is a ivio-component, structural adhesive whith is packaged in engineered plastic carijidges. II is used with either a manual, pneumath or power-operated Injection tool and proportionally miked through a statit-element mixing nozze. ALI00 pius has been vigorously tested to meet or exceed required stendards as on anchoring adheslve. This all-weather adhesive can be used effectively in temperatures as low as $-4^{\circ} \mathrm{F}\left(-2 D^{\circ} \mathrm{C}\right)$
ACIOD plus is deslgned for use in anthoring threaded tods, bolts, reinfordng bars, and smodth dowels into concete and masony base meterials. The system can alloo be used to anchor into hollow masonry maierials using rod and rebar whth screen tubes. The AC100 Pius athesive is a $100 \%$ solids, low odor, molsiture linsenstive formulation which does not contain volatile organic compounds (VO['s) and is fies of styrene and solvents.


- Heavy duriy anchoring such as rebar, threaded anchor rods, and threaded bolts in solid concrete, grout filled black, stone, etc
- Used in wet enviranmenis, a wide range of temperatures and vehenever solvert or shyrene fumes are not acceptable
- Anchoring with screen tubes in hollow block ar brich
- Repalt and setroftt projectis


## 

- Lsted and approved to resist dead loads, live loads, and short-tern loads such as those resulting from wind or earthquake
- Superior dispensing speed and fast cure even in low temperatures
- Allweather material is ideal for coll and moderate envinonment applicaticns
- $100 \%$ sollds, syrrene free, anchoring mortar; juith no VOC's
- Avallable in five cartidge slzes to match project and application
- Non-flarnmable, does not contain hazardous methy-methacrylate like other "acyliti" fommulas
- Virtually odorless for indioor and outdors applications
- Meets the requirements of ASTM CBB), Types I ard N, Grade 3, Class A, B and C
- Optimal for use In diamond cored hoies
- suitable for dyy, damp or water-filled holes
- Mieets current building code and DOT repulrements
- High load capactities in concreie and masony
- Excellent chemical resistance
- Independenty tested and quatified so ASTM E1512 and AC5B Criteria, Induding crep resistance, freeze-thaw cyclling and simulated selismbecyind conditions

International Code Council, Evaluation Senice (ICC-E5) ESR-1 636
aty oi Los Angeles (COLA) Research Report LARR-25579
Miaml-Dade County Notlce oi Acceptance (NOA) O4-0820,02
Meets ASTM C8Bi and AA5HTO M235
Various North American Deparments of Transportation (DOi) - See whw.powers,com
(TH11
C5I Divisions: 03151-Concrete Anchoring, 04081-Masonry Anchorage and 05090-Metal
Fastenings, Adhesse anchoring s.siem shail be AC100 Plus es supplled by Powers Fasteners,
inc., New Rocheile, NY.


## sEETION CONTENTS Page No.

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Derlgn Criteria ............................ 15
Ordering Information $\qquad$

Actoo Plus Coaxlad Cartridge


AC100 Plus Dual Cartridge

## PACKAGING

Coaxlal Cartrldge
5.5 fi. oz. (160 mil or 9.9 (n3)

10 in. ox ( 295 ml or $i 8,0$ in in )
Dual (Sida-by-Side) Cartridge
8 解 oz ( 235 ml or 14.4 lm )
12 A or ( $355 \mathrm{mfor} 21.6 \mathrm{fn}^{3}$ )
30 fl. ox. ( 590 तl or $54.8 \mathrm{~m}^{3}$ )
ANCHOR SIZE RANGE (TYP.)
$3 / 8^{2}$ bo $1-9 / 14^{2}$ diameter rod Na. 3 to No. 11 relnforing bat $3 / 8^{\circ}$ to $1-1 / 4^{\circ}$ smooth dowe bar $1 / 2^{\circ}$ to $3 / 4^{*}$ intermally threared inseris

## SUITABLE RASE MATERIALS

## Normal-weight Concate

Structural Ughtwelght Concrete
Gravted Concerte Masonfy
Hollow CMU
Brick Majony
Stone

## 

Physical Properties of Adheslue

| Shetillife | 18 monthrs from date of manufacture |
| :---: | :---: |
| Storage Condlions | $14^{\circ} \mathrm{F}\left(-10^{\circ} \mathrm{C}\right)$ to $\mathrm{B} \mathrm{S}^{\mathrm{P}} \mathrm{F}\left(30^{\circ} \mathrm{C}\right)$ |
| Injedion Temperature | -4F $\left(-20^{\circ} \mathrm{C}\right.$ ) or preater |
| Color | $\begin{aligned} & \text { Component A (Resla) -White } \\ & \text { Componem } \mathrm{B} \text { (Hardener) - Blad } \end{aligned}$ |
| Mixing ratio | 10:1 by wolume |
| Consistency | Unilorm, nan-sag gray morisi |
| Shone Hardness (A5710 D2240) | 90 |
| Compressive 5trangh (ASTh D 695) | 10, 100 p5l, 7 days |
| Tensill Strength (ASTMD 63B) | 2,100 psi |
| $\begin{aligned} & \text { Fexuial Strength } \\ & \langle\text { ASiMD } 790\} \end{aligned}$ | 3,670 ps |
| $\begin{aligned} & \text { Sant Shear Strength } \\ & \text { \|ASThD732\} } \end{aligned}$ | 4,590 pri |
| Water ábsorpilon (ASTMD 570) | Lers than $1 \%$ ( $0.119 \%$ ) |
| $\begin{aligned} & \text { Bond Syength } \\ & \text { (ASTMC } \operatorname{BED}) \end{aligned}$ | $\begin{aligned} & \text { 1,380 ps, } 2 \text { Day Cure } \\ & 1,760 \text { ps), } 2 \text { Day Cure } \end{aligned}$ |
| $\begin{aligned} & \text { Shinlage } \\ & \text { (ASTMD 2566) } \end{aligned}$ | 0.004 info |
| $\begin{aligned} & \text { Hest Deflecion } \\ & \text { (ASTh D 648) } \end{aligned}$ | 1760 ${ }^{\text {a }}$ ( $\left.0^{\circ} \mathrm{C}\right)$ |

Setting Times

| Bare Material Temperatare | Maximum Gel TImer | $\begin{aligned} & \text { Minlmum } \\ & \text { Curing Tme } \end{aligned}$ |
| :---: | :---: | :---: |
| -4P $\left(-20^{\circ} \mathrm{C}\right)$ | 12 halus | 72 hous |
| $505[-15 . \square$ | 9 helis | 2't hous |
| 145 (-10 $\left.0^{\circ} \mathrm{C}\right)$ | 4 hours | 12 hours |
| $23^{\circ} 5\left(-5^{\circ} \mathrm{C}\right)$ | 2 hours | 5 houst |
| $32^{\circ} \times 10^{\circ} \mathrm{C}$ | 40 minules | 4 hous |
| $41^{\circ} \mathrm{F}\left(5^{\circ} \mathrm{C}\right)$ | 20 minules | 2 bours |
| $50^{\circ} \mathrm{F}\left(10^{\circ} \mathrm{C}\right)$ | 15 mhuter |  |
| $59^{\circ} \mathrm{F}\left(15^{\circ} \mathrm{C}\right]$ | 10 minutes | 45 minutes |
| $68^{5} 5\left(20^{2} \mathrm{C}\right)$ | 7 minules | 30 minut |
| $85^{4} \mathrm{~F}$ (30 ${ }^{\circ} \mathrm{C}$ | 4 minutes | 25 minutes |
| $104^{\circ} \mathrm{F}\left(40^{\circ} \mathrm{C}\right)$ | 2 minules | 20 minates |

2. The gel tine is the meximum ine duting whith the athetive can be tispensed Eelore it
 ale dre.



## mstallation Specifications

| Propaty |  | Rod Dlamater ${ }^{\text {d ( }} \mathrm{In}$ ) |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 3/8 | $1 / 2$ | 5/8 | 3/4 | 718 | 1 | $11 / 4$ |
| Asom $=$ Naminal ajea of hreaded rod (inchi) |  | 0.71305 | 0, 1963 | 0.3068 | 0,4418 | 0.6013 | 0.7854 | 1.2272 |
| Ae $=$ Yensile styess araa of roid incin') |  | 0.0775 | 0.1419 | 0.2260 | 0.3345 | 0.4617 | 0.6057 | 0,9691 |
| $\mathrm{dsm}_{\text {a }}=$ Nominal bit dameter (lnch) |  | 7116 | 915 | 1V16 | 13/16 | 15/15 bt) | 11/16 | 1378 |
| $\begin{aligned} & T_{\text {mx }}=\begin{array}{c} \text { Nax fightering } \\ \text { torgue range (fi-Hss) } \end{array} \end{aligned}$ |  | 5-6 | 10.12 | 20.22 | 35.40 | 55-68 | 75.85 | 135-950 |
|  | $h_{1} \geq 8 \mathrm{~d}$ | 15.17 | 30-35 | 50-62 | 100-110 | 148-955 | 210.230 | 400-450 |


| Propetty | Relnforeing Bar Slzes, $d^{\prime}$ |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | No, 3 | No. 4 | $\mathrm{No,5}$ | No. 5 | N0. 7 | HaB | No. 9 | Hal 10 | 140.11 |
| $\mathrm{d}^{\circ}=$ Nomlal bas damater (inch) | 318 | $1 / 2$ | 518 | 3/4 | -718 | 1 | $11 / 8$ | 1114 | 1318 |
| $\mathrm{d}_{4}=$ Elilective anchol dimmeter (ind) | 0.375 | 0.500 | 0.625 | 0.750 | 0.875 | 1.000 | 1.128 | 9.270 | 1,410 |
| As = Nominal zea of felinforing bas [inch') | 0.110 | 0.200 | 0.310 | 0.440 | -0,600 | 0.790 | 1,000 | 1.270 | 1,560 |
|  | 7196 | $9 / 96$ | 11/16 | 718 | -1 | $11 / 8$ | $11 / 4$ | $11 / 2$ | 15/8 |



Nomencliature
$d=$ Diamale of $100 \mathrm{D} \times$ rebar
dor $=$ Dimeler of dind bir
$h=825$ malemal thidness The minhmur value of $h$ should be 1.5 h $_{\text {, }}$
$h,=$ Mintrum erbeoinent deph
$1=$ Oysall lengh of rod or refan

- F Feture ind ${ }^{2}$ ness

Trau = Masmum thhtening trique (Oity passbe allay full ore)

## Flygt Monitoring Devices ENM-10 Liquid Level Sensors

## n-… <br> Conirols : $\quad$ :

The simplest possible method for levei coniroll A mechanical micro switch in a plastic casing, freely suspended at the desired height from its own cable. When the ilquid level reaches the regulator, the casing will tilt and the mechanical switch will close or break the clrcuit, thereby starting or stopping a pump or actuating an alarm device. No wear, no maintenancel Use in sewage pumping stations, for ground water and drainage pumping - in fact, for most level control applications - the ENM-10 is the ideal solution.

The regulator casing is made of polypropylene and the cable is sheathed with a special PVC compound. The plastic components are welded and screwed together adhesive is never used. Impurities and deposits will noi adhere to the smooth casing.

This level regulator is available in different versions, depending upon the medium in which it is to be used. The standard model can be obtained with 20, 43 or 66 feet of cable for liquids with specilic gravities between 0.95 and 1.10. For other specific gravities, the regulator is only
 available with 66 feet of cabie. The regulator can withstand ternperatures of $32^{\circ} \mathrm{F}$ to $140^{\circ} \mathrm{F}$.


When the level drops, the mioro switch is activaled....

pumping slops and the level begins to rise....


When it reacbes the highest pergaissible point, the second regulator reacts....

and pumping resumes.

| Specific Gravity of Liquid | Cable Lenglh | $\begin{gathered} \text { ENM-10 } \\ \text { Part Number } \end{gathered}$ | ENM-10 <br> Sensor Specificatlons |  |
| :---: | :---: | :---: | :---: | :---: |
| $0.65-0.80$ | $66^{\prime}$ | 5828827 | Min. oper lemp. | $32^{\circ} \mathrm{F}\left(0^{\circ} \mathrm{C}\right)$ |
| $0.80-0.95$ | $66^{\prime}$ | 5828828 | Max. oper. temp. | $140^{\circ} \mathrm{F}\left(60^{\circ} \mathrm{C}\right)$ |
| 0.95-1.10 | $20^{\prime}$ | 5828829 | Max. applied vollage | 250VAC/30VDC |
| $\rightarrow 0.95-1.10$ | $43^{\prime}$ | 5828830 | Max, amperagé - | 16A @ 250VAC |
| 0.95-1.10 | 66' | 5828831 | Resistive load. | 16A@ 250VAC |
| 1.05-1.20 | $66^{\prime}$ | 5828832 |  | 5A@30VDC |
| 1.20-1.30 | $66^{\prime}$ | 5828833 | Induclive load- | 4A@ 260VAC |
| 1.40 | $66^{\prime}$ | 5828834 | Max. angular displacement | $60^{\circ}$ |
| 1.50 | $66^{1}$ | 5828835 | Opelatting point - rising Operaling poinl - descending |  |

## 

PART NO. 14-407129


## Features:

- Plug in replacement for existing MiniCAS / FUS unit
- $120 \mathrm{VAC}, 24 \mathrm{VAC}$, or 24 VDC powered.
- Durable plastic enclosure bith flange for mounting on door of pump control enclosure
- Highly visible red LEDs for indication of Leakage and Temperature alarms
- Green LED for indication power is applied
- Temperature alarm reset mode select switch, for selection of Manual or Aalo reset modes
- Tenperature alarm reset push-button on front of cinit
- Inpul power transient protected
- Sensor input circuit transiene protected
- Sensor input cixcuit short cireuit protected
- Noise Filter on Sensor Tpput
- Sensor circuit steply voltage regulated to 12 VDC
- Detailed connection diagram on side of unit

ITT FLYGT OORPORATION
35 Nutmeg Drive
Trunbull, Connectiout 066i1
Phone (203) 380-4700
FAK (203) 380-4705


PART NO. 14407129


Speciffcations:

- Input Power: $120 \mathrm{VAC} \pm 10 \%, 7.0 \mathrm{VA}$ max $24 \mathrm{VAC}=10 \%, 3.5 \mathrm{VA}$ max $24 \mathrm{VDC} \pm 10 \%, 125 \mathrm{~mA}$ max
- Input Power Transient Protection: Metal Oxide Vaxistor
- Sensor Input Transient Protection: Metal Oxide Varistor
- Relay Contact Rating: 10A © 250 VAC
- Relay Confact Material: Silver Cadnuuna Oxide (AgCdO)
- Ambient Operating Temperature: $-20^{\circ} \mathrm{C}$ to $+65^{\circ} \mathrm{C}\left(-4^{\circ} \mathrm{F}\right.$ to $\left.+149^{\circ} \mathrm{F}\right)$
- Agency Approvals: OL 508, CAN/CSA (Pending)
- Alam fodicatom: Super Bright Red LED
- Power On lindicator: Green LED
- Enclosure: Blue Lexan (141R) or Noryl (PX9406)
- Faceplate Oycrtay: Silver Lexan with Black Text
- Side Label: Sitver Lexan with Black 'Text
- Sensor Citcuit Supply Voltage: Regulated 12 VDC $\pm 10 \%$,
- Weight 11.75 oz ( 334 grams)
- Temperature Alam Tip Point: Sensor Current $\leq 3 \mathrm{~mA} \pm 5 \%$
- Leak Alarm Trip Point: Sensor Cument $\geq 22 \mathrm{~mA} 5 \%$


## ITT FLYGT <br> CONTROL PANEL STORAGE RECOMMENDATIONS

The following procedure is recommended when a control panel is stored over extended periods of time.

- Since the moisture content of the air can be extremely high, it is recommended that the controls be stored in a controlled atmosphere.
- The cọrosion inhibitor should be replaced yearly
- Ensure the enclosure door is tightened.




## Appendix I. Gravity Districts - REMOVED

## Appendix J. Approved Landuse Plan



## FLAMINGO CROSSINGS PD

LUP-16-04-147<br>FLAMINGO CROSSINGS BLVD \& WESTERN WAY<br>ORANGE COUNTY, FL







| PERMITED USES |  |  |  |
| :---: | :---: | :---: | :---: |
| 1. Thoses uses Permitited within the $\mathrm{C}-1$ (Retrai Commerciat) Distict, and the following Special Exopption Use colleges and universties | RECREATION | as Requeste |  |
|  | Active and Pamsive recreational amentifies will be provided at a ratio of 2.5 Acrea per 1,000 residents, in accortanca with Section $38-1253$ (b) of the Orange County Code of Ordinances. Locatlons and types of amenities to se provided at time of Development Plis. |  |  |
|  |  |  |  |
|  |  | tunded my (150) imarsutemert |  |
| Ptogramp Partictpantus for the durstion of their iniernitips of 3 -15 month is, and and on-stre operator's control, any remanining units may be lessed for a Wall Cheney Wortd Oporating Perricipant businessoes (Employees'). Residential unitis for Paricipants and Employeses shall be separated by buildng. There shall be no co-mingling of Paricipants and Employees within the same multh-amily buiding The developor (ar Disney) shall resport on an annual basis the number of buddings leased to Employess dunng the previocus yeerf, or partion of a previous year, and the number of buildings antacoitad to bo needed for Employeet in tho up-coming years (it is understood that during a given year, the occupancy forecast might be revised and the number of buildings deployed for the Employees may vary) | traffic data |  entrunded by paiking. The perking arees wil be nagregited fom the buiding arees to <br>  |  |
|  | Thip Gemernton Culculution |  |  |
|  | Lensuse If cose Size |  | аппи\% |
|  |  | 2 A wiver fom, Orange County Coce Sedion 38-1254(1) bo mindila minium PD <br>  <br>  | - omil - come |
|  |  |  | somer mecmer |
|  | ${ }_{4}^{4.808}$ |  |  |
|  | 2001200 |  | 3 - |
|  | 2,05 |  | \%expelv |
|  |  |  |  |
|  |  |  |  |
|  |  |  |  |
|  | HOOL DEM |  |  |
| DEVELOPMENT STANDARDS | The applicant is working with the Orenge County School Diatrici to deterrines the public school impact of the proposed residentiel program. |  |  |
|  |  |  |  |
|  |  |  |  |
|  | 1. Whater <br> Waber, genitary semer, and reclaimed waber कervices shair be provided by Orange County Unilities. A detailod fiow analysia will be submided during the Master Utility Pian review procest | 4. A wever fom Orange County Code Secion $38-1259$ (d) 0 alow a mamum biding <br>  storidefority (H0) liont |  |
| Menimum Lot Size: 6,000 SQ FT |  |  |  |
|  | 2. Devolopment within the preject ehel maintein minimum fre flows in accordance with Orange County requirements. |  <br>  |  |
| Commercial: $\quad{ }_{30}^{50}$ Foed within 100 Feoot of Residenvial | 3. Other servictas, such as cable and electrical wifl be provided in accondante whth the approprime territarial agreementa. |  | 8 |
| Minimum Builing Soblecks: | STORMWATER MANAGEMENT |  trines |  |
|  | The Reedy Creek Improvernent District (RCID) has constructed and mantans a Mester Stomwater System for the entire RCID district, which includes the Flemingo Crossinge PD. Design and Maintanance of stormwater feailities will be in accordance with the interiocal Agreement between Orange County and RCID, excecuted on —— 2017. |  |  |
| Croseings Blvel: From Westom Way: |  |  | - |
|  |  | en whim axommodating mej jexign. |  |
|  |  |  |  |
| Minimum Residenial Lixng Anembu: 500 Sort | Slommoater pond loctations will be delermined at PSP and the frial dezign with be submitted during the construction plan approval proceses. | etheot botween intemal lot ines witin the develponert in liou of prowing a landectape buller a rinimum of senel (7) fisel in widh | - |
| - See Whivera section for rotanted waviver. <br> -* Maximum buibiling helght doess not indude arctinectural deeipn features that may extend beyond the top of the roof ine. | A totai of 32.76 atrea of Class I wedands is located an the West Parcel. The wettand arees within the West Parcel are subjeci to the following environmental permitt: |  |  |
|  |  |  |  |
| PHASING |  on Nowember is, 2015. <br>  modifad on Octiober 19, 2015. |  |  |
| The propact will be conssucted in multple phases. Each phase will contain edequate infrastructura to stand on its own. |  | 7. A wever from Orenge County Code Section 24-E(a)(3) to allow a Type C, opequetrd <br>  <br>  |  |
| OPEN SPACE \& IMPERVIOUS AREA (OVERALL PDI | Note: An Orange Counly Conservation Area Determination (CAD) for the subject watiand area muat be epproved prior to PSP of DP spproyil. |  |  |
| Minimum Open Spaca Reavired: |  | 寿 |  |
|  | PARNME <br> Tha multi-imily cominarcied uses will meet the pariding requitromentis of Orange County Land Dovelopment Code Articla Xl. On-atreet pariding locatied within intemal rights-or-way may be unad to meat the parting requitremants of reaidential andror commercial users. |  |  |
| Individual tracts or phases may not meot the open space criteria, but the overal PD will meet the open uppace requirement. A meater Oper Spaca chan shat dee provisted on the firx PSP. With Updates added on esth subsequent PSP of CP |  |  |  |
|  |  |  |  |
|  |  | bo | amor Lemen |
| Moxinum Impervious Areatat Cowerege:Mulin-Finily: $30 \%$ (Buildinge onty) <br> Commercial: $70 \%(3.0$ FAR) | sigmage <br> Signege with the PD shat meet the roquremente of Orange County Land dovelopment Code Aricte II |  |  |
|  |  |  | 6 of 6 |

## Appendix K. Wholesale Meter Specification

## ULTRA MAG

## U|tratiet: <br> Ultra Mag And SIGMAL CONVERTER



## DESCRIPTION

MODELS UM06 AND UM08 FLANGED TUBE Wetre Mice meters are manufactured to the highest standard available for magmeters. They incorporate microprocessor technologyto offer verylow flows and broad range ability. The flanged end tube design permits use in a wide range of applications with up to 300 PSI working pressure. Flanged ends are:

- Steel AWWA Class "D" flat face flanges (150 PSI) for UM06
- Steel AWWA Class "F" raised face flanges ( 300 PSI) for UM08 (2", 3", and $\geq 14^{\prime \prime}$ )
- Steel ANSI 300 lb . Raised Face Flanges for UM08 ( $4^{\prime \prime}$ - $12^{\prime \prime}$ )

The fabricated tube is stainless steel with steel or stainless steel flanges and is lined with UltraLiner ${ }^{\text {TM }}$, an NSF approved, fusion bonded epoxy material.

INSTALLATION is made similar to placing a short length of flanged end pipe in the line. The meter can be installed vertically, horizontally, or inclined on suction or discharge lines. The meter must have a full pipe of liquid for proper operation. Fluid must be grounded to the downstream flange of the sensor either via internal grounding electrodes (4-12") or using McCrometer 316 SS Grounding Rings. For best performance, grounding rings are recommended for all sizes. Any 90 or 45 degree elbows, valves, partially opened valves, etc. should not be placed closer than one pipe diameters upstream and zero pipe diameters downstream. All blending and chemical injection should be done early enough so the flow media is thoroughly mixed prior to entering the measurement area.

SIGNALCONVERTER:The signal converteristhe reporting, input and output control device for the sensor. The converter allows the measurements, functional programming, control of the sensor and data recording to be communicated through the display and inputs/outputs. The microprocessor-based signal converter has a curve-fitting algorithm to improve accuracy, dual 4-20mA analog outputs, an optional RS485 communication port, an 8 line graphical backlit LCD display with 3-key touch programming, anda rugged enclosure that meets IP67.In addition to a menu-driven self-diagnostic test mode, the converter continually monitors the microprocessor's functionality. The converter will output rate of flow and total volume. The converter also comes standard with password protection and many more features.

ISOLATED POWER AND SIGNAL: The power and signal between the converter and sensor are isolated and placed in separate cables giving superior resistance to electrical signal noise compared to single cable designs. An added benefit from the dual cable design is a maximum cable length of up to 500 ft .

## OPTIONAL:

DC powered converter ( $10-35 \mathrm{VDC}, 21 \mathrm{~W}$ )
Meter mounted converter
Extended warranty
Hastelloy ${ }^{6}$ electrodes
ANSI or DIN flanges
Quick Connect cable fittings
Special lay lengths, including ISO standard lay lengths
Converter sun shield
Modbus Protocol RS485 converter; HART ${ }^{*}$ Converter; Profibus Converter (No Dual 4-20mA on HART \& Profibus); Smart Output" (Sensus or Itron compatible); Panel mount converter (Not CSA approved); Battery or battery-solar powered converter (Not CSA approved, $\pm 1 \%$ accuracy)

# MODEL UM06 AND UM08 <br> Ultran Mag' electromagnetic flow meter <br> 150 PSI FLANGED TUBE METER, SIZES 2" thru 48" 300 PSI FLANGED TUBE METER, SIZES 2" thru 48" 

## SPECIFICATIONS

WARRANTY: 2 Years
ACCURACY TESTS: 5-point wet flow calibration of every complete flow tube with its signal converter. If desired, the tests can be witnessed by the customer. The McCrometer test facilities are traceable to the National Institute of Standards \& Technology. Uncertainty relative to flow is $\pm 0.15 \%$
ACCURACY: Plus or minus $0.5 \%$ of actual flow (battery powered is $\pm 1 \%$ of flow)
IMPORTANT NOTICE ON FLOW METER ACCURACY: The flow meter, the cable and the electronics are factory calibrated for accuracy as a single unit. Changing the cable length with the Splice Kit changes the accuracy of the meter and invalidates the calibration certificate.

REPEATABILITY: $\pm 0.05 \%$ or $\pm .0008 \mathrm{ft} / \mathrm{s}( \pm 0.25 \mathrm{~mm} / \mathrm{s})$, whichever is greater
HEAD LOSS: None. No obstruction in line and no moving parts
PRESSURE RANGE: 150 PSI maximum working pressure (UM06); 300 PSI maximum working pressure (UMO8)
TEMPERATURE RANGE: Sensor Operating: - 10 to $77^{\circ} \mathrm{C}$ ( 14 to $140^{\circ} \mathrm{F}$ ) Sensor Storage: - 15 to $77^{\circ} \mathrm{C}\left(5\right.$ to $140^{\circ} \mathrm{F}$ ) Electronics: Operating and storage temperature:
$-4^{\circ}$ to $140^{\circ} \mathrm{F}\left(-20^{\circ}\right.$ to $\left.60^{\circ} \mathrm{C}\right)$
VELOCITY RANGE: 2 to 32 FPS
BI-DIRECTIONAL FLOW: Forward and reverse flow indication and forward, reverse, net totalization are standard with all meters
CONDUCTIVITY: $5 \mu \mathrm{~s} / \mathrm{cm}$
LINER: UltraLiner NSF approved, fusion bonded epoxy
ELECTRODES: Type 316 stainless steel, others optional
POWER SUPPLY: AC: $100-240 \mathrm{VAC} / 45-66 \mathrm{~Hz}$ (20W/25VA), DC: $10-35 \mathrm{VDC}(21 \mathrm{~W})$, battery (four lithium D cell batteries), five-year estimated life, solar ( 5 W panel). $A C, D C$, battery, or battery \& solar must be specified at time of ordering.
OUTPUTS: Dual 4 -20mA Outputs (Not available for Profibus, HART, or battery converters): Galvanically isolated and fully programmable for zero and full scale ( $0-22 \mathrm{~mA}$ ).
Four separate digital programmable outputs: open collector transistor usable for pulse, frequency, or alarm settings.

- Volumetric Pulse - Hardware Alarm
- Flow Rate (Frequency) . Empty Pipe
- Directional Indication - Range Indication
- High/Low Flow Alarms

SENSOR CABLE:
Standard: $25^{\prime}$ McCrometer supplied submersible cable with each remote mount unit.
Qptional: Up to 500 feet, or 50 feet max for battery powered.
Quick connect: Available in standard cable lengths: $25^{\prime \prime}, 50^{\prime}, 75^{\prime}, 100^{\prime}, 125^{\prime}$,
150', 175,200 , and $500^{\prime}$. Custom cable lengths at additional cost.
CONVERTER/SENSOR SEPARATION: $\leq 500$ feet; for longer lengths consult factory
EMPTY PIPE SENSING: Zero return when electrodes are uncovered
ALARMS: Programmable alarm outputs
DIGITAL TOTALIZER: Cubic Meter; Cubic Centimeter; Milliliter; Liter; Cubic Decimeter; Decaliter; Hectoliter; Cubic Inches; US Gallons; Imperial Gallons; Cubic Feet; Kilo Cubic Feet; Standard Barrel; Oil Barrel; US Kilogallon; Ten Thousands of Gallons; Imperial Kilogallon; Acre Feet; Megagallon; Imperial Megagalion; Hundred Cubic Feet, Megaliters
RATINGS: Metering Tube: NEMA 6P/IP68 with remote converter; submersible with a standard quick-connect cable to 6 ft . and optional strain relief at 30 ft . Die cast aluminum converter: IP67; Panel mount converter: IP65

## CERTIFICATIONS:

- CE Certified (Converter only)
- Listed by CSA to 61010-1: Certified by C5A to UL 61010-1 and C5A C22.2 No.61010-1-04
- ISO 9001:2015 certified quality management system



## ULTra Mac

## MODEL UM06 AND UM08

## ELECTROMAGNETIC FLOW METER



Converter Dimensions


4" to 12" Models Body Style
14+" Models Body Style

| Pipe Size <br> (Nominal) | Meter Pipe ID | $\begin{array}{\|c} \text { Flow Ranges GPM } \\ \text { Standard } \\ .2 \text { to } 32 \text { FPS } \\ \text { Min - Max } \end{array}$ | DIMENSIONS (Lay Lengths) |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | A* |  | B | C |  | D | E |  |  |
|  |  |  | UM06 | UM08 |  | UM06 | UM08 |  |  | UM06 | UM08 |
| $2^{\text {n }}$ | 2.117 | 2-340 | 11.00 | 11.00 | 6.70 | 6.00 | 6.50 | 7.90 | 9.26 | 93 | 107 |
| $3{ }^{\prime \prime}$ | 3.220 | 5-730 | 13.40 | 13.40 | 6.70 | 7.50 | 8.25 | 9.40 | 10.01 | 97 | 111 |
| 4" | 3.720 | 8-1,140 | Rec | d |  |  | 10.00 | n/a | 8.06 | 78 | 108 |
| $6^{\prime \prime}$ | 5.692 | 19-2,6\%30 | Recla |  | r | \% | 12.50 | n/a | 9.06 | 82 | 138 |
| $8{ }^{\prime \prime}$ | 7.692 | 33-4,, 70 | 16.10 | 17.25 | n/a | 13.50 | 15.00 | n/a | 10.06 | 115 | 195 |
| $10^{\prime \prime}$ | 9.682 | 52-7670 | 18.50 | 18.50 | n/a | 16.00 | 17.50 | n/a | 10.46 | 144 | 247 |
| $12^{\prime \prime}$ | 11.682 | 74-11,180 | 19.70 | 19.70 | n/a | 19.00 | 20.50 | n/a | 12.31 | 193 | 342 |
| $14^{\prime \prime}$ | 13.440 | 90-16,070 | 21.70 | 22.75 | 12.00 | 21.00 | 23.00 | 20.30 | 15.46 | 321 | 476 |
| $16^{\prime \prime}$ | 15.440 | 118-20,900 | 23.60 | 25.25 | 14.20 | 23.50 | 25.50 | 21.10 | 16.21 | 390 | 645 |
| 18 | 17.44 U | IT 150-20,480 | 23.60 | 25.25 | 14.20 | 25.00 | 28.00 | 21.10 | 17.21 | 446 | 750 |
| $20 "$ | 19.440 | 185-32,720 | 25.60 | 28.25 | 16.20 | 27.50 | 30.50 | 24.80 | 18.26 | 588 | 874 |
| $24^{\prime \prime}$ | 23.440 | 270. 47,180 | 30.70 | 35.75 | 21.70 | 32.00 | 36.00 | 29.60 | 20.11 | 769 | 1,568 |
| $30^{\prime \prime}$ | 29.190 | 420-78,62 16 | Potabl | Water | 6.50 | 38.75 | 43.00 | 35.90 | 23.26 | 1,261 | 2,317 |
| 36 " | 35.190 | 610-105,93 Me |  |  | 8.8.20 | 46.00 | 50.00 | 42.70 | 26.66 | 1,696 | 2,915 |
| $42^{\prime \prime}$ | 41.190 | 830-144,370 | 48.05 | ** | 32.10 | 52.75 | ** | 48.35 | 29.99 | ** | ** |
| $48^{\prime \prime}$ | 47.190 | 1,080-188,430 | 50.00 | ** | 36.00 | 59.50 | ** | 54.00 | 33.31 | ** | ** |

[^2]
## Appendix L. RCID HGL Approval Letter

Machlus, Kimberly A


## The wastewater intertonnect HGL is as foilows:

- Wastewater interconnect HGL - The propased manhole invert elevation out side of LS-91 is 103 feet

Please confirm RCID approval of the HGL information used in the short term models.
Thank you,

Kimberly Krutski Machlus
Project Director
Engineering, Design, and Project Management
$\square, 407.8064132 \square 814.360 .4982$
지
482 South Keller Road Orlando. FL 32810





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## Atkins

482 South Keller Road
Orlando, FL 32810-6101

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[^0]:    Paul E Partlow, PE
    Senıor Engıneer

[^1]:    Walt Disney Wortd West District Water, Wastewater and Reclaimed Water Master Utility Plan | Version 6.0| September 2018 Page $\mid 44$

[^2]:    * Laying lengths for meters with ANSI Class 150 Flanges are equal to UM08 laying lengths
    ** Consult factory

